## MEMORANDUM

TO:<br>Cobra Realty Trust<br>c/o Messrs. Daniel Prouty and<br>Michael Ciesla<br>P.O. Box 1039<br>Charlton City, MA 01508

FROM: Mr. Jeffrey S. Dirk, P.E.*, PTOE, FITE DD<br>Managing Partner and<br>Mr. Andrew J. Arseneault<br>Senior Transportation Engineer<br>Vanasse \& Associates, Inc.<br>35 New England Business Center Drive<br>Suite 140<br>Andover, MA 01810-1066<br>(978) 269-6830<br>jdirk@rdva.com<br>*Professional Engineer in CT, MA, ME, NH, RI and VA

DATE: April 28, 2023 RE: 9650
SUBJECT: Transportation Impact Assessment
Proposed Commercial Building - 150 Charlton Road (Route 20)
Sturbridge, Massachusetts

Vanasse \& Associates, Inc. (VAI) has conducted a Transportation Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed construction of a commercial building to be located at 150 Charlton Road (Route 20) in Sturbridge, Massachusetts (hereafter referred to as the "Project"). This study evaluates the following specific areas as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; and identifies and analyzes existing traffic conditions and future traffic conditions, both with and without the Project, along Route 20 and at the intersection of Route 20 at the Project site driveway.

Based on this assessment, we have concluded the following with respect to the Project:

1. Using trip-generation statistics published by the Institute of Transportation Engineers (ITE), ${ }^{1}$ the Project is expected to generate approximately 326 vehicle trips on an average weekday (two-way volume over the operational day of the Project), with 32 vehicle trips expected during the weekday morning peak-hour and 32 vehicle trips expected during the weekday evening peak-hour;
2. All movements exiting the Project site were shown to operate at a level-of-service (LOS) C during both the weekday morning and evening peak-hours with residual vehicle queues of up to one (1) vehicle, which can be contained within the Project site without impeding access or circulation, or the movement of vehicles along Route 20;
3. No apparent safety deficiencies were noted with respect to the motor vehicle crash history along Route 20 in the vicinity of the Project site; and
4. Lines of sight to and from the Project site driveway intersection with Route 20 were found to exceed the recommended minimum distance for safe operation.
[^0]In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with the implementation of the recommendations defined herein.

The following details our assessment of the Project.

## PROJECT DESCRIPTION

The Project will entail the construction of an $8,000 \pm$ square foot (sf) commercial building to be located at 150 Charlton Road (Route 20) in Sturbridge, Massachusetts, that is anticipated to be occupied by a designer/manufacturer of prototype, laser-powered diagnostic and medical devices. The Project site encompasses approximately $6.18 \pm$ acres of undeveloped land that is bounded by areas of open and wooded space and a solar farm to the north and west; Route 20 to the south; and commercial properties to the east.


Imagery ©2023 Google

Access to the Project site will be provided by way of a new driveway that will intersect the northwest side of Route 20 approximately 650 feet southwest of the Center at Hobbs Brook driveway. On-site parking will be provided for 16 vehicles, which exceeds the requirements of Part 4, Article XVI, Section 300-16.11, Parking spaces required, of the Zoning Bylaw of the Town of Sturbridge. ${ }^{2}$

[^1]
## STUDY METHODOLOGY

This study was prepared in consultation with the Massachusetts Department of Transportation (MassDOT) and the Town of Sturbridge; was performed in accordance with MassDOT's Transportation Impact Assessment (TIA) Guidelines and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports; and was conducted in three distinct stages.

The first stage involved an assessment of existing conditions in the study area and included an inventory of roadway geometrics; pedestrian and bicycle facilities; on-street parking; public transportation services; observations of traffic flow; and collection of pedestrian, bicycle, and vehicle counts.

In the second stage of the study, future traffic conditions were projected and analyzed. Specific travel demand forecasts for the Project were assessed along with future traffic demands due to expected traffic growth independent of the Project. A seven-year time horizon was selected for analyses consistent with MassDOT guidelines. The analysis conducted in stage two identifies existing or projected future capacity, safety, and access issues, as these areas relate to the transportation infrastructure.

The third stage of the study presents and evaluates measures to address deficiencies in the transportation infrastructure, if any, identified in stage two of the study.

## EXISTING CONDITIONS

A comprehensive field inventory of existing conditions within the study area was conducted in February and March 2023. This inventory included the collection of traffic-volume data and vehicle travel speed measurements, as well as a review of existing pedestrian and bicycle accommodations, public transportation services, and motor vehicle crash data. The following summarizes existing conditions within the study area.

## Roadway

## Route 20

Route 20 is a four lane, urban principal arterial roadway under MassDOT jurisdiction that traverses the study area in a general northeast-southwest direction. Route 20 provides two (2) 11 to 12-foot wide travel lanes that are separated by a painted median with 2 to 3 -foot wide marked shoulders. The posted speed limit within the study area is 50 miles per hour (mph). Sidewalks and formal bicycle facilities are not provided along Route 20 and Route 20 was not found to provide sufficient width to accommodate bicycle travel in a shared traveled-way condition (i.e., bicyclists and motor vehicles sharing the traveled-way). ${ }^{3}$ Illumination is provided by way of street-lights mounted on wood poles. Land use along Route 20 within the study area consists of the Project site, commercial properties and areas of open and wooded space.

[^2]
## Existing Traffic Volumes

In order to determine existing traffic-volume demands and flow patterns within the study area, automatic traffic recorder (ATR) counts were conducted on Route 20 in the vicinity of the Project site on March $8^{\text {th }}$ through $9^{\text {th }}, 2023$ (Wednesday through Thursday, inclusive) in order to record weekday traffic over an extended period.

In order to evaluate the potential for seasonal fluctuation of traffic volumes within the study area, MassDOT weekday seasonal factors for Urban Group 3 roadways (principal arterials, the functional classification of Route 20) were reviewed. ${ }^{4}$ Based on a review of this data, it was determined that traffic volumes for the month of March are approximately 2.0 percent above average-month conditions. As such, no adjustment was made to the raw traffic count data as the data is representative of traffic volume conditions that are higher than those under average-month conditions.

Based on updated guidance from MassDOT, ${ }^{5}$ adjustments to account for the impact on traffic volumes and trip patterns resulting from the COVID-19 pandemic for traffic counts taken on or after March 1, 2022 are not recommended in areas where the adjacent land uses are not predominantly office properties. As the study area roadway and intersections serve a diverse range of land uses (residential, retail, office and industrial), further adjustment of the traffic-volume data was not required.

Route 20 in the vicinity of the Project site was found to accommodate approximately 21,010 vehicles per day on an average weekday (two-way, 24 -hour volume), with approximately 1,477 vehicles per hour (vph) during the weekday morning peak-hour (8:00-9:00 AM) and 1,839 vph during the weekday evening peak-hour (4:15-5:15 PM). The 2023 Existing weekday morning and evening peak-hour traffic volumes are graphically depicted on Figure 1.

## Spot Speed Measurements

Vehicle travel speed measurements were performed on Route 20 in the vicinity of the Project site in conjunction with the ATR counts. Table 1 summarizes the vehicle travel speed measurements.

Table 1 VEHICLE TRAVEL SPEED MEASUREMENTS

|  | Route 20 |  |
| :--- | :---: | :---: |
|  | Northeastbound |  |
| Mean Travel Speed (mph) | 44 | 44 |
| $85^{\text {th }}$ Percentile Speed (mph) | 50 | 52 |
| Posted Speed (mph) | 50 | 50 |
| mph $=$ miles per hour. |  |  |

[^3]

## WEEKDAY EVENING PEAK HOUR (4:15-5:15 PM)



As can be seen in Table 1, the mean vehicle travel speed along Route 20 in the vicinity of the Project site was found to be 44 mph in both the northeast and southwestbound directions. The measured $85^{\text {th }}$ percentile vehicle travel speed, or the speed at which 85 percent of the observed vehicles traveled at or below, was found to be 50 mph eastbound and 52 mph westbound, which is generally consistent with the posted speed limit ( 50 mph ). The $85^{\text {th }}$ percentile speed is used as the basis of engineering design and in the evaluation of sight distances, and is often used in establishing posted speed limits.

## Public Transportation Services

The Town of Sturbridge is a member of the Worcester Regional Transit Authority (WRTA); however, regularly scheduled, fixed-route public transportation services are not currently provided within the Town. The WRTA does provide fixed-route bus service to the Towns of Charlton and Southbridge by way of bus Route 29, which provides service along Route 169 and Route 20 to the east of the Project site, with continued service to Union Station in downtown Worcester, where connections can be made to the Massachusetts Bay Transportation Authority (MBTA) commuter rail system (Worcester Line) and to other WRTA bus routes.

## Motor Vehicle Crash Data

Motor vehicle crash information for the study area intersection was provided by the MassDOT Highway Division Safety Management/Traffic Operations Unit for the most recent five-year period available (2016 through 2020, inclusive) in order to examine motor vehicle crash trends occurring within the study area. Based on a review of this data, no (0) motor vehicle crashes were reported to have occurred in the immediate vicinity of the Project site over the five-year review period. In addition, a review of the MassDOT statewide High Crash Location List indicated that there are no Highway Safety Improvement Program (HSIP) eligible high crash locations in the vicinity of the Project site.

## Based on a review of the MassDOT motor vehicle crash data, no discernible safety deficiencies were apparent at the intersection.

## FUTURE CONDITIONS

Traffic volumes in the study area were projected to the year 2030, which reflects a seven-year planning horizon consistent with MassDOT guidelines. Independent of the Project, traffic volumes on the roadway network in the year 2030 under No-Build conditions include all existing traffic and new traffic resulting from background traffic growth. Anticipated Project-generated traffic volumes superimposed upon the 2030 No-Build traffic volumes reflect 2030 Build traffic-volume conditions with the Project.

## Future Traffic Growth

Future traffic growth is a function of the expected land development in the immediate area and the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed estimates an annual percentage increase in traffic growth and applies that percentage to all traffic volumes under study. The drawback to such a procedure is that some turning volumes may actually grow at either a higher or a lower rate at particular intersections.

An alternative procedure identifies the location and type of planned development, estimates the traffic to be generated, and assigns it to the area roadway network. This procedure produces a more realistic estimate of growth for local traffic; however, potential population growth and development external to the study area would not be accounted for in the resulting traffic projections.

To provide a conservative analysis framework, both procedures were used, the salient components of which are described below.

## Specific Development by Others

The Town of Sturbridge Planning Department and the Town of Charlton Planning Board were contacted in order to determine if there were any projects planned within the study area that would have an impact on future traffic volumes at the study intersections. Based on these discussions, the following projects were identified for inclusion in this assessment.
> Proposed Travel Center and Electric Vehicle Discovery Center, 195, 197, 201, and 201A Charlton Road (Route 20), Sturbridge, Massachusetts (EEA No. 16389). This proposed project will entail the construction of a travel center with an associated fueling facility and an electrical vehicle discovery center to be located at 195, 197, 201, and 201A Charlton Road (Route 20). As proposed, the travel center will include the following components: a $8,437 \pm$ sf building that will contain a convenience store, coffee shop with drive-through window, a restaurant, an ice cream parlor and a common seating area; a 10-pump (20 vehicle fueling position (vfp)) fueling facility; a 4-pump diesel fueling facility; and 10 electric vehicle (EV) charging stations. The electric vehicle discovery center will consist of a separate $16,640 \pm$ sf building that will contain an $8,866 \pm$ sf electrical vehicle discovery conference center, $4,482 \pm$ sf office space and a 120 -seat sit-down restaurant. The conference center component of the discovery center will be used by electric vehicle manufacturers to display their vehicles and allow visitors to interact with manufacturer representatives and salespeople.
> Proposed Coffee Shop and Urgent Care Facility, 212, 216 and 226 Charlton Road (Route 20), Sturbridge, Massachusetts. This project will entail the construction of a $2,298 \pm$ sf coffee shop with drive-through window and a $5,148 \pm$ sf urgent care facility to be located at 212, 216 and 226 Charlton Road (Route 20).
> Tree House Brewing Expansion, 129 Sturbridge Road Charlton, Massachusetts (EEA No. 15900). This project consists of the expansion of the existing Tree House Brewing brewery located at 129 Sturbridge Road from $43,000 \pm$ sf to $67,718 \pm$ sf, of which $43,000 \pm$ sf will continue as the brewery operation, $20,710 \pm$ sf will be devoted to retail sales and $4,008 \pm$ sf will consist of a timber frame pavilion to provide additional outdoor space for customers. In addition, a 7,600 $\pm$ sf outdoor "beer garden" area will also be constructed between the main building and the pavilion. This project is not expected to result in an increase in traffic during the weekday commuter peakhours that would exceed the general background traffic growth rate.
> Auto Storage and Towing Facility, 299 Sturbridge Road, Charlton, Massachusetts. This project is currently under construction at 299 Sturbridge Road and is not expected to result in an increase in traffic within the study area that would exceed the general background traffic growth rate.
> Proposed Warehouse Development, 241 Sturbridge Road, Charlton, Massachusetts (EEA No. 16211). This project entails the construction of a $1,200,000 \pm$ sf warehouse to be located at 241 Sturbridge Road.
> Proposed Sortation Warehouse, 53 Sturbridge Road, Charlton, Massachusetts (EEA No. 16386). This proposed project will entail the construction of a $2.85 \pm$ million square foot (sf) sortation warehouse to be located at 53 Sturbridge Road.
> Proposed Gas Station and Convenience Market, 16 Sturbridge Road, Charlton, Massachusetts. This project will entail the construction of a 4-pump ( 8 vfp ) fueling facility with an associated $3,000 \pm$ sf convenience market located at 16 Sturbridge Road and is not expected to result in an increase in traffic within the study area that would exceed the general background traffic growth rate.

Traffic volumes associated with the aforementioned specific development projects by others were obtained from the traffic study prepared in support of the project or were developed by using trip-generation information available from the Institute of Transportation Engineers (ITE) ${ }^{6}$ for the appropriate land use, and were assigned onto the study area roadway network based on existing traffic patterns where no other information was available. No other developments were identified at this time that are expected to result in an increase in traffic within the study area beyond the general background traffic growth rate.

## General Background Traffic Growth

Traffic-volume data compiled by MassDOT from permanent count stations located in Sturbridge were reviewed in order to determine general traffic growth trends in the area. This data indicates that traffic volumes have fluctuated over the 10-year period between 2009 and 2019, with an average traffic growth rate of 0.48 percent. In order to provide a prudent planning condition for the Project, a 1.0 percent per year compounded annual background traffic growth rate was used in order to account for future traffic growth and presently unforeseen development within the study area.

## Roadway Improvement Projects

MassDOT and the Town of Sturbridge were consulted in order to determine if there were any planned future roadway improvement projects expected to be complete by 2030. Based on these discussions, no roadway improvement projects aside from routine maintenance activities were identified to be planned in the vicinity of the Project site at this time.

To the east of the Project site, the proponent of the sortation warehouse that is to be located at 53 Sturbridge Road in Charlton will upgrade and replace the traffic signal controller and associated hardware and appurtenances necessary to implement adaptive traffic control systems (ATCS) technologies at the Route 20/The Center at Hobbs Brook intersection. To the west of the Project site, the proponent of the Travel Center and Electric Vehicle Discovery Center that is to be located at 195, 197, 201, and 201A Charlton Road (Route 20) in Sturbridge will be installing either a traffic signal that will include ATCS technologies or modern roundabout at the primary driveway to the travel center and will constrict geometric improvements along Route 20. These improvements will be complete within the horizon year of this assessment (2030) and are expected to result in an overall improvement in the flow of traffic along the Route 20 corridor with consideration of the increase in traffic that is expected from approved and planned future development along the corridor.

## No-Build Traffic Volumes

The 2030 No-Build condition peak-hour traffic-volumes were developed by applying the 1.0 percent per year compounded annual background traffic growth rate to the 2023 Existing peak-hour traffic volumes and then adding the traffic volumes associated with the identified specific development projects by others. The resulting 2030 No-Build weekday morning and evening peak-hour traffic volumes are shown on Figure 2.

[^4]

## WEEKDAY EVENING PEAK HOUR (4:15-5:15 PM)



## Project-Generated Traffic

As proposed, the Project will entail the construction of an $8,000 \pm$ sf commercial building that is anticipated to be occupied by a designer/manufacturer of prototype, laser-powered diagnostic and medical devices. In order to develop the traffic characteristics of the Project, trip-generation statistics published by the ITE ${ }^{7}$ for a similar land use as that proposed was used. ITE Land Use Code (LUC) 760, Research and Development Center, was used to develop the traffic characteristics for the Project, the results of which are summarized in Table 2.

Table 2
TRIP GENERATION SUMMARY

| Time Period | Vehicle Trips ${ }^{\text {a }}$ |  |  |
| :---: | :---: | :---: | :---: |
|  | Entering | Exiting | Total |
| Average Weekday: | 163 | 163 | 326 |
| Weekday Morning Peak-Hour: | 26 | 6 | 32 |
| Weekday Evening Peak-Hour: | 5 | 27 | 32 |

${ }^{\text {a Based on ITE LUC 760, Research and Development Center. }}$

## Project-Generated Traffic-Volume Summary

As can be seen in Table 2, the Project is expected to generate approximately 326 vehicle trips on an average weekday (two-way volume over the operational day of the Project, or 163 vehicles entering and 163 exiting), with 32 vehicle trips ( 26 vehicles entering and 6 exiting) expected during the weekday morning peak-hour and 32 vehicle trips ( 5 vehicles entering and 27 exiting) expected during the weekday evening peak-hour.

## Trip Distribution and Assignment

The directional distribution of generated trips to and from the Project site was determined based on a review of U.S. Census Journey-to-Work data for the Town of Sturbridge and then refined based on a review of existing traffic patterns within the study area. The general trip distribution for the Project is graphically depicted on Figure 3, with the additional traffic that is expected to be generated by the Project assigned on the study area roadway network as shown on Figure 4.

## Build Traffic Volumes

The 2030 Build condition traffic volumes consist of the 2030 No-Build traffic volumes with the addition of the traffic expected to be generated by the Project. The 2030 Build weekday morning and evening peak-hour traffic volumes are graphically depicted on Figure 5.

[^5]Legend:
XX Entering Trips
(XX) Exiting Trips


## Legend:

XX Entering Trips
(XX) Exiting Trips




## WEEKDAY EVENING PEAK HOUR (4:15-5:15 PM)



## TRAFFIC OPERATIONS ANALYSIS

In order to assess the potential impact of the Project on the roadway network, a detailed traffic operations analysis (motorist delays, vehicle queuing, and level-of-service) was performed at the intersection of Route 20 at the Project site driveway. Capacity analyses provide an indication of how well transportation facilities serve the traffic demands placed upon them, with vehicle queue analyses providing a secondary measure of the operational characteristics of an intersection or section of roadway under study.

In brief, six levels of service are defined for each type of facility. They are given letter designations ranging from A to F, with LOS "A" representing the best operating conditions and LOS "F" representing congested or constrained operations. An LOS of " E " is representative of a transportation facility that is operating at its design capacity with an LOS of "D" generally defined as the limit of "acceptable" traffic operations. Since the level-of-service of a traffic facility is a function of the flows placed upon it, such a facility may operate at a wide range of levels of service depending on the time of day, day of week, or period of the year. The Synchro® intersection capacity analysis software, which is based on the analysis methodologies and procedures presented in the Highway Capacity Manual, $6^{\text {th }}$ Edition (HCM) ${ }^{8}$ for unsignalized intersections, was used to complete the level-of-service and vehicle queue analyses.

## Analysis Results

Level-of-service and vehicle queue analyses were conducted for the study intersection under 2030 Build conditions. The results of the intersection capacity and vehicle queue analyses are summarized in Table 3, with the detailed analysis results attached. The following is a summary of the analysis results. For context, we note that an LOS of "D" or better is generally defined as "acceptable" operating conditions.

Table 3
UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

| Unsignalized Intersection/ Peak-Hour/Movement | 2030 Build |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Demand | Delay | LOS | Queue $95^{\text {th }}$ |
| Route 20 at the Project Site Driveway |  |  |  |  |
| Weekday Morning: |  |  |  |  |
| Project site driveway SEB LT/RT | 6 | 19.8 | C | 0 |
| Route 20 NEB LT/TH | 889 | 0.8 | A | 0 |
| Route 20 SWB TH/RT | 1,084 | 0.0 | A | 0 |
| Weekday Evening: |  |  |  |  |
| Project site driveway SEB LT/RT | 27 | 23.6 | C | 1 |
| Route 20 NEB LT/TH | 1,075 | 0.1 | A | 0 |
| Route 20 SWB TH/RT | 1,191 | 0.0 | A | 0 |

${ }^{a}$ Demand in vehicles per hour.
${ }^{\text {b }}$ Average control delay per vehicle (in seconds).
${ }^{\text {c }}$ Level-of-Service.
${ }^{\text {d }}$ Queue length in vehicles.
NEB = northeastbound; SWB = southwestbound; SEB = southeastbound
LT = left-turning movements; $\mathrm{TH}=$ through movements; RT = right-turning movements.

[^6]
## Route 20 at the Project Site Driveway

All movements exiting the Project site driveway are predicted to operate at LOS C during both the weekday morning and evening peak-hours, with a predicted vehicle queue of up to one (1) vehicle. All movements along Route 20 approaching the driveway were shown to operate at LOS A during both the weekday morning and evening peak-hours with negligible vehicle queuing predicted.

## SIGHT DISTANCE EVALUATION

Sight distance measurements were performed at the Project site driveway intersection with Route 20 in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO) ${ }^{9}$ requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an on-coming vehicle and safely complete a turning or crossing maneuver with on-coming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 4 presents the measured SSD and ISD at the subject intersection.

Table 4
SIGHT DISTANCE MEASUREMENTS ${ }^{\text {a }}$

| Intersection/Sight Distance Measurement | Feet |  |  |
| :---: | :---: | :---: | :---: |
|  | Required <br> Minimum (SSD) | Desirable (ISD) ${ }^{\text {b }}$ | Measured |
| Route 20 at the Project Site Driveway |  |  |  |
| Stopping Sight Distance: |  |  |  |
| Route 20 approaching from the northeast | 455 | -- | 740 |
| Route 20 approaching from the southwest | 455 | -- | 1,000+ |
| Intersection Sight Distance: |  |  |  |
| Looking to the northeast from the Project site driveway | 455 | 500 | 480 |
| Looking to the southwest from the Project site driveway | 455 | 615 | 1,000+ |

${ }^{\text {a }}$ Recommended minimum values obtained from A Policy on Geometric Design of Highways and Streets, $7^{\text {th }}$ Edition; American Association of State Highway and Transportation Officials (AASHTO); 2018; and based on a 52 mph approach speed along Route 20.
${ }^{\text {b }}$ Values shown are the intersection sight distance for a vehicle turning right or left exiting a roadway under STOP control such that motorists approaching the intersection on the major street should not need to adjust their travel speed to less than 70 percent of their initial approach speed. The critical gap for left-turn movements exiting the Project site driveway was increased by 0.5 seconds in order to account for the time to cross the additional travel lane along Route 20.

As can be seen in Table 4, the available lines of sight at the Project site driveway intersection with Route 20 were found to exceed the recommended minimum sight distance for the driveway to function in a safe (SSD) manner based on a 52 mph approach speed along Route 20 , which is slightly above the posted speed limit ( 50 mph ) and consistent with the higher measured $85^{\text {th }}$ percentile vehicle travel speed approaching the driveway ( $50 / 52 \mathrm{mph}$ ).

[^7]

## SUMMARY

VAI has completed a detailed assessment of the potential impacts on the transportation infrastructure associated with the proposed construction of a commercial building to be located at 150 Charlton Road (Route 20) in Sturbridge, Massachusetts. The following specific areas have been evaluated as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; under existing and future conditions, both with and without the Project. Based on this assessment, we have concluded the following with respect to the Project:

1. Using trip-generation statistics published by the ITE $^{10}$ the Project is expected to generate approximately 326 vehicle trips on an average weekday (two-way volume over the operational day of the Project), with 32 vehicle trips expected during the weekday morning peak-hour and 32 vehicle trips expected during the weekday evening peak-hour;
2. All movements exiting the Project site were shown to operate at LOS C during both the weekday morning and evening peak-hours with residual vehicle queues of up to one (1) vehicle, which can be contained within the Project site without impeding access or circulation, or the movement of vehicles along Route 20;
3. No apparent safety deficiencies were noted with respect to the motor vehicle crash history along Route 20 in the vicinity of the Project site; and
4. Lines of sight to and from the Project site driveway intersection with Route 20 were found to exceed the recommended minimum distance for safe operation.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with the implementation of the recommendations that follow.

## RECOMMENDATIONS

A detailed transportation improvement program has been developed that is designed to provide safe and efficient access to the Project sites and address any deficiencies identified as a part of this assessment. The following improvements have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

## Project Access

Access to the Project site will be provided by way of a new driveway that will intersect the northwest side of Route 20 approximately 650 feet southwest of the Center at Hobbs Brook driveway. The following recommendations are offered with respect to the design and operation of the Project site access and internal circulation, many of which are reflected on the Site Plans:
> The Project site driveway should be a minimum of 24 feet in width and designed to accommodate the turning and maneuvering requirements of service and delivery vehicles and the largest anticipated responding emergency vehicle as defined by the Sturbridge Fire Department.

[^8]> Where perpendicular parking is proposed, the drive aisle behind the parking should be a minimum of 23 feet ( 24 feet is proposed) in order to facilitate parking maneuvers.
> One-way drives within the Project-site are a minimum of 20 -feet in width. "One Way" and "Do Not Enter" signs should be installed where one-way traffic is to be conveyed and at locations where the one-way drive transitions to two-way operation.
> Vehicles exiting the Project sites should be placed under STOP-sign control with a marked STOP-line provided (shown on the Site Plan)
> All signs and pavement markings to be installed within the Project sites should conform to the applicable standards of the Manual on Uniform Traffic Control Devices (MUTCD). ${ }^{11}$
> Pedestrian walkways are proposed within the Project site that should include ADA-compliant wheelchair ramps at all pedestrian crossings to link the parking field to the proposed building.
> Signs, landscaping and other features that are to be installed as a part of the Project within the intersection sight triangle areas of the Project site driveway should be designed and maintained so as not to restrict lines of sight.
> Snow accumulation (windrows) within the sight triangle areas of the Project site driveways should be promptly removed where such accumulations would impede sight lines.

With implementation of the aforementioned recommendations, safe and efficient access will be provided to the Project site and the Project can be accommodated within the confines of the existing and improved transportation system.
cc: File

[^9]

## ATTACHMENTS

PROJECT SITE PLAN<br>AUTOMATIC TRAFFIC RECORDER COUNT DATA<br>SEASONAL ADJUSTMENT DATA<br>VEHICLE TRAVEL SPEED DATA<br>TRANSIT SCHEDULES<br>MASSDOT HIGH CRASH LOCATION MAPPING<br>GENERAL BACKGROUND TRAFFIC GROWTH<br>BACKGROUND DEVELOPMENT TRAFFIC-VOLUME NETWORKS<br>PROPOSED TRIP-GENERATION CALCULATIONS<br>US CENSUS JOURNEY-TO-WORK DATA<br>SIGHT DISTANCE CALCULATIONS<br>CAPACITY ANALYSIS WORKSHEETS

PROJECT SITE PLAN


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9.) If THE PROPOSED ROADWAY AREAS ARE NOT PAVED IMMEDATEL SHALL EE PLACED TO PROTECT THE INTEGRITT OF THE STRUCTURES
10.) THE LOCATION OF UNDERGROUND UTILTIES AND STRUCTUVES ARE
BASED ON FIELD AND RECORD INEORMATION. THE ENGINER DOES NOT GUARANTEE THEIR ACCURACY OR THAT ALL UTILTIES AND SUBSURFACE
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12.) THE CONTRACTOR SHALL PROVIDE FOR ALL TRAFFIC CONTROL
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SHALL be separate horlontaly yY minimum or ten feer. 15.) TREE WORK MUST RE COMPLETED BY COMPANEE HOLDING CURRENT


IST OF DRAWINGS
SHEET-1
SHET-2
SHEET-3
SHET-4
SHET-5
SHEET-6
SHET-7
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SHETT-9
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CONSTRUCTION DETAIL SHEET\# CONSTRUCTION DETAIL SHEET\#2
CONSTRUCTION DETAIL SHEET\#3


APPLICANT

## Cobra realty trust MICHAEL CIESLA $\otimes$

 14 HARVARD STREE
## OUNERS:

 14 AARVARD
WORCESTER, MA
DEED Book 9104 PG. 301
ASSESSORS PARCEL iD:
208-02612-15

## DEFINITIVE SITE PLAN at 150 CHARLTON ROAD (ROUTE 20) STURBRIDGE, MA



beng a manority

2nem


## Accurate Counts

978-664-2565

Location: Route 20
96500001
Location: North of Gifford Road

| 3/8/2023 | NB |  | Hour Totals |  | SB |  | Hour Totals |  | Combined Totals |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Morning | Afternoon | Morning | Afternon | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon |
| 12:00 | 26 | 164 |  |  | 14 | 191 |  |  |  |  |
| 12:15 | 21 | 144 |  |  | 13 | 171 |  |  |  |  |
| 12:30 | 16 | 183 |  |  | 6 | 143 |  |  |  |  |
| 12:45 | 7 | 162 | 70 | 653 | 10 | 176 | 43 | 681 | 113 | 1334 |
| 1:00 | 10 | 171 |  |  | 6 | 195 |  |  |  |  |
| 1:15 | 11 | 179 |  |  | 11 | 149 |  |  |  |  |
| 1:30 | 7 | 156 |  |  | 6 | 177 |  |  |  |  |
| 1:45 | 10 | 164 | 38 | 670 | 11 | 171 | 34 | 692 | 72 | 1362 |
| 2:00 | 8 | 177 |  |  | 8 | 169 |  |  |  |  |
| 2:15 | 10 | 181 |  |  | 7 | 219 |  |  |  |  |
| 2:30 | 6 | 171 |  |  | 5 | 217 |  |  |  |  |
| 2:45 | 9 | 172 | 33 | 701 | 6 | 206 | 26 | 811 | 59 | 1512 |
| 3:00 | 12 | 203 |  |  | 7 | 209 |  |  |  |  |
| 3:15 | 9 | 183 |  |  | 11 | 176 |  |  |  |  |
| 3:30 | 7 | 223 |  |  | 12 | 200 |  |  |  |  |
| 3:45 | 13 | 190 | 41 | 799 | 9 | 235 | 39 | 820 | 80 | 1619 |
| 4:00 | 14 | 221 |  |  | 15 | 245 |  |  |  |  |
| 4:15 | 23 | 213 |  |  | 7 | 233 |  |  |  |  |
| 4:30 | 27 | 211 |  |  | 26 | 266 |  |  |  |  |
| 4:45 | 35 | 208 | 99 | 853 | 30 | 248 | 78 | 992 | 177 | 1845 |
| 5:00 | 28 | 217 |  |  | 44 | 232 |  |  |  |  |
| 5:15 | 44 | 192 |  |  | 47 | 218 |  |  |  |  |
| 5:30 | 57 | 185 |  |  | 56 | 207 |  |  |  |  |
| 5:45 | 76 | 184 | 205 | 778 | 63 | 202 | 210 | 859 | 415 | 1637 |
| 6:00 | 103 | 143 |  |  | 62 | 177 |  |  |  |  |
| 6:15 | 116 | 126 |  |  | 84 | 173 |  |  |  |  |
| 6:30 | 129 | 120 |  |  | 109 | 124 |  |  |  |  |
| 6:45 | 157 | 119 | 505 | 508 | 127 | 149 | 382 | 623 | 887 | 1131 |
| 7:00 | 120 | 94 |  |  | 143 | 124 |  |  |  |  |
| 7:15 | 131 | 81 |  |  | 132 | 106 |  |  |  |  |
| 7:30 | 153 | 81 |  |  | 140 | 111 |  |  |  |  |
| 7:45 | 159 | 76 | 563 | 332 | 150 | 116 | 565 | 457 | 1128 | 789 |
| 8:00 | 127 | 78 |  |  | 128 | 97 |  |  |  |  |
| 8:15 | 124 | 68 |  |  | 121 | 78 |  |  |  |  |
| 8:30 | 167 | 55 |  |  | 125 | 72 |  |  |  |  |
| 8:45 | 163 | 51 | 581 | 252 | 149 | 63 | 523 | 310 | 1104 | 562 |
| 9:00 | 134 | 72 |  |  | 136 | 41 |  |  |  |  |
| 9:15 | 125 | 50 |  |  | 120 | 63 |  |  |  |  |
| 9:30 | 152 | 40 |  |  | 155 | 52 |  |  |  |  |
| 9:45 | 148 | 37 | 559 | 199 | 154 | 41 | 565 | 197 | 1124 | 396 |
| 10:00 | 126 | 37 |  |  | 127 | 54 |  |  |  |  |
| 10:15 | 138 | 38 |  |  | 144 | 32 |  |  |  |  |
| 10:30 | 153 | 18 |  |  | 170 | 29 |  |  |  |  |
| 10:45 | 172 | 29 | 589 | 122 | 142 | 15 | 583 | 130 | 1172 | 252 |
| 11:00 | 137 | 8 |  |  | 151 | 26 |  |  |  |  |
| 11:15 | 161 | 23 |  |  | 156 | 21 |  |  |  |  |
| 11:30 | 156 | 18 |  |  | 156 | 22 |  |  |  |  |
| 11:45 | 164 | 8 | 618 | 57 | 176 | 8 | 639 | 77 | 1257 | 134 |
| Total | 3901 | 5924 |  |  | 3687 | 6649 |  |  | 7588 | 12573 |
| Percent | 39.7\% | 60.3\% |  |  | 35.7\% | 64.3\% |  |  | 37.6\% | 62.4\% |

Accurate Counts
978-664-2565

Location: Route 20
96500001
Location: North of Gifford Road
City/State: Sturbridge, MA

| 3/9/2023 | NB |  | Hour Totals |  | SB |  | Hour Totals |  | Combined Totals |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | Morning | Afternoon | Morning | Afternon | Morning | Afternoon | Morning | Afternoon | Morning | Afternoon |
| 12:00 | 11 | 165 |  |  | 15 | 158 |  |  |  |  |
| 12:15 | 4 | 175 |  |  | 13 | 185 |  |  |  |  |
| 12:30 | 9 | 140 |  |  | 6 | 182 |  |  |  |  |
| 12:45 | 6 | 176 | 30 | 656 | 6 | 176 | 40 | 701 | 70 | 1357 |
| 1:00 | 5 | 151 |  |  | 15 | 177 |  |  |  |  |
| 1:15 | 5 | 166 |  |  | 14 | 191 |  |  |  |  |
| 1:30 | 6 | 183 |  |  | 9 | 174 |  |  |  |  |
| 1:45 | 9 | 163 | 25 | 663 | 5 | 201 | 43 | 743 | 68 | 1406 |
| 2:00 | 9 | 185 |  |  | 9 | 204 |  |  |  |  |
| 2:15 | 3 | 169 |  |  | 7 | 209 |  |  |  |  |
| 2:30 | 13 | 165 |  |  | 4 | 186 |  |  |  |  |
| 2:45 | 8 | 160 | 33 | 679 | 8 | 196 | 28 | 795 | 61 | 1474 |
| 3:00 | 7 | 212 |  |  | 12 | 203 |  |  |  |  |
| 3:15 | 11 | 189 |  |  | 10 | 212 |  |  |  |  |
| 3:30 | 13 | 222 |  |  | 7 | 262 |  |  |  |  |
| 3:45 | 2 | 228 | 33 | 851 | 8 | 226 | 37 | 903 | 70 | 1754 |
| 4:00 | 17 | 206 |  |  | 16 | 229 |  |  |  |  |
| 4:15 | 20 | 215 |  |  | 6 | 262 |  |  |  |  |
| 4:30 | 20 | 230 |  |  | 26 | 228 |  |  |  |  |
| 4:45 | 38 | 202 | 95 | 853 | 30 | 245 | 78 | 964 | 173 | 1817 |
| 5:00 | 34 | 220 |  |  | 46 | 244 |  |  |  |  |
| 5:15 | 48 | 191 |  |  | 51 | 212 |  |  |  |  |
| 5:30 | 62 | 194 |  |  | 49 | 216 |  |  |  |  |
| 5:45 | 81 | 184 | 225 | 789 | 68 | 200 | 214 | 872 | 439 | 1661 |
| 6:00 | 95 | 149 |  |  | 94 | 164 |  |  |  |  |
| 6:15 | 105 | 134 |  |  | 94 | 168 |  |  |  |  |
| 6:30 | 134 | 138 |  |  | 121 | 158 |  |  |  |  |
| 6:45 | 142 | 104 | 476 | 525 | 168 | 138 | 477 | 628 | 953 | 1153 |
| 7:00 | 126 | 99 |  |  | 212 | 134 |  |  |  |  |
| 7:15 | 122 | 105 |  |  | 241 | 104 |  |  |  |  |
| 7:30 | 135 | 84 |  |  | 285 | 110 |  |  |  |  |
| 7:45 | 156 | 102 | 539 | 390 | 291 | 103 | 1029 | 451 | 1568 | 841 |
| 8:00 | 154 | 72 |  |  | 252 | 99 |  |  |  |  |
| 8:15 | 146 | 72 |  |  | 315 | 99 |  |  |  |  |
| 8:30 | 159 | 62 |  |  | 342 | 86 |  |  |  |  |
| 8:45 | 140 | 44 | 599 | 250 | 339 | 55 | 1248 | 339 | 1847 | 589 |
| 9:00 | 135 | 62 |  |  | 207 | 49 |  |  |  |  |
| 9:15 | 121 | 52 |  |  | 163 | 57 |  |  |  |  |
| 9:30 | 141 | 49 |  |  | 146 | 49 |  |  |  |  |
| 9:45 | 155 | 41 | 552 | 204 | 152 | 50 | 668 | 205 | 1220 | 409 |
| 10:00 | 151 | 36 |  |  | 134 | 50 |  |  |  |  |
| 10:15 | 145 | 36 |  |  | 154 | 37 |  |  |  |  |
| 10:30 | 135 | 13 |  |  | 136 | 34 |  |  |  |  |
| 10:45 | 169 | 23 | 600 | 108 | 154 | 23 | 578 | 144 | 1178 | 252 |
| 11:00 | 144 | 25 |  |  | 175 | 21 |  |  |  |  |
| 11:15 | 173 | 17 |  |  | 190 | 22 |  |  |  |  |
| 11:30 | 164 | 15 |  |  | 188 | 14 |  |  |  |  |
| 11:45 | 157 | 12 | 638 | 69 | 164 | 12 | 717 | 69 | 1355 | 138 |
| Total | 3845 | 6037 |  |  | 5157 | 6814 |  |  | 9002 | 12851 |
| Percent | 38.9\% | 61.1\% |  |  | 43.1\% | 56.9\% |  |  | 41.2\% | 58.8\% |
| Grand Total | 7746 | 11961 |  |  | 8844 | 13463 |  |  | 16590 | 25424 |
| Percent | 39.3\% | 60.7\% |  |  | 39.6\% | 60.4\% |  |  | 39.5\% | 60.5\% |
| ADT |  | ADT: 21,007 |  | DT: 21,007 |  |  |  |  |  |  |

ـ


| Day | 0 | 0 | 20161 |  | 217 |  | 0 | 0 0 |  | 20962 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AM Peak |  |  | 11:00 | 11:00 | 11:00 | 8:00 |  |  |  | 11:00 | 8:00 |
| Volume |  |  | 618 | 639 | 638 | 1248 |  |  |  | 628 | 886 |
| PM Peak |  |  | 4:00 | 4:00 | 4:00 | 4:00 |  |  |  | 4:00 | 4:00 |
| Volume |  |  | 853 | 992 | 853 | 964 |  |  |  | 853 | 978 |
| Comb Total | 0 | 0 | 2016 |  | 217 |  | 0 | 0 | 0 |  |  |
| ADT | ADT: 21,007 | AADT: 21,007 |  |  |  |  |  |  |  |  |  |

SEASONAL ADJUSTMENT DATA

Massachusetts Highway Department
Statewide Traffic Data Collection
2019 Weekday Seasonal Factors

| Factor Group | JAN | FEB | MAR | APR | MAY | JUN | JUL | AUG | SEP | OCT | NOV | DEC | Axle Factor |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| R1 | 1.22 | 1.14 | 1.12 | 1.06 | 1.00 | 0.96 | 0.87 | 0.85 | 0.96 | 0.99 | 1.04 | 1.12 | 0.85 |
| R2 | 0.95 | 0.96 | 0.98 | 0.97 | 0.97 | 0.93 | 0.97 | 0.94 | 0.96 | 0.90 | 0.92 | 0.93 | 0.96 |
| R3 | 1.15 | 1.06 | 1.07 | 1.00 | 0.89 | 0.88 | 0.89 | 0.89 | 0.95 | 0.92 | 1.02 | 1.01 | 0.97 |
| R4-R7 | 1.09 | 1.09 | 1.11 | 1.02 | 0.96 | 0.92 | 0.89 | 0.89 | 0.99 | 0.98 | 1.09 | 1.13 | 0.98 |
| U1-Boston | 1.03 | 1.01 | 0.98 | 0.94 | 0.94 | 0.92 | 0.95 | 0.93 | 0.94 | 0.94 | 0.97 | 1.04 | 0.96 |
| U1-Essex | 1.09 | 1.06 | 1.03 | 0.99 | 0.94 | 0.90 | 0.88 | 0.86 | 0.93 | 0.94 | 0.99 | 1.06 | 0.93 |
| U1-Southeast | 1.06 | 1.05 | 1.01 | 0.97 | 0.95 | 0.93 | 0.93 | 0.90 | 0.94 | 0.94 | 0.98 | 1.04 | 0.98 |
| U1-West | 1.19 | 1.14 | 1.09 | 0.95 | 0.92 | 0.89 | 0.89 | 0.86 | 0.91 | 0.95 | 0.97 | 1.07 | 0.84 |
| U1-Worcester | 1.02 | 1.04 | 0.97 | 0.94 | 0.93 | 0.91 | 0.95 | 0.91 | 0.93 | 0.92 | 0.95 | 1.10 | 0.88 |
| U2 | 1.01 | 1.00 | 0.94 | 0.93 | 0.91 | 0.89 | 0.93 | 0.90 | 0.90 | 0.91 | 0.94 | 1.02 | 0.99 |
| U3 | 1.06 | 1.03 | 0.98 | 0.94 | 0.93 | 0.91 | 0.95 | 0.91 | 0.92 | 0.93 | 0.97 | 1.00 | 0.98 |
| U4-U7 | 1.01 | 1.00 | 0.95 | 0.92 | 0.88 | 0.86 | 0.92 | 0.91 | 0.92 | 0.94 | 0.99 | 1.04 | 0.99 |
| Rec - East | 1.04 | 1.16 | 1.12 | 0.98 | 0.92 | 0.88 | 0.77 | 0.81 | 0.94 | 1.02 | 1.08 | 1.12 | 0.99 |
| Rec - West | 1.30 | 1.23 | 1.32 | 1.18 | 0.95 | 0.82 | 0.70 | 0.69 | 0.97 | 0.96 | 1.16 | 1.15 | 0.98 |

Round off:
$0-999=10$
$>1000=100$
$\mathrm{U}=\mathrm{Urban}$
R = Rural

1 - Interstate
2 - Freeway and Expressway
3 - Other Principal Arterial
4 - Minor Arterial
5 - Major Collector
6 - Minor Collector
7 - Local Road and Street

Recreational - East Group - Cape Cod (all towns) including the town of Plymouth south of Route 3A (stations
7014,7079,7080,7090,7091,7092,7093,7094,7095,7096,7097,7108 and 7178), Martha's Vineyard and Nantucket.
Recreational - West Group - Continuous Stations 2 and 189 including stations
$1066,1067,1083,1084,1085,1086,1087,1088,1089,1090,1091,1092,1093,1094,1095,1096,1097,1098,1099,1100,1101,1102,1103,1104,1105,1106,1107,1108,1113,1114$,
$1116,2196,2197$ and 2198.

## Accurate Counts

978-664-2565

Location : Route 20
96500001
Location : North of Gifford Road
City/State: Sturbridge, MA
Direction: NB

| $\begin{array}{r} \hline 3 / 8 / 2023 \\ \text { Time } \end{array}$ | $\begin{aligned} & 0-15 \\ & \mathrm{MPH} \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & >70 \\ & \mathrm{MPH} \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 0 | 3 | 4 | 12 | 20 | 17 | 13 | 0 | 0 | 0 | 1 | 70 |
| 1:00 | 0 | 1 | 0 | 2 | 5 | 9 | 12 | 4 | 4 | 1 | 0 | 0 | 0 | 38 |
| 2:00 | 0 | 0 | 0 | 0 | 4 | 5 | 8 | 10 | 4 | 2 | 0 | 0 | 0 | 33 |
| 3:00 | 0 | 0 | 1 | 0 | 3 | 6 | 12 | 13 | 1 | 4 | 1 | 0 | 0 | 41 |
| 4:00 | 0 | 0 | 0 | 1 | 2 | 23 | 23 | 32 | 9 | 5 | 3 | 1 | 0 | 99 |
| 5:00 | 0 | 1 | 0 | 1 | 8 | 26 | 47 | 71 | 27 | 21 | 3 | 0 | 0 | 205 |
| 6:00 | 0 | 2 | 0 | 1 | 13 | 50 | 125 | 173 | 96 | 39 | 6 | 0 | 0 | 505 |
| 7:00 | 0 | 5 | 2 | 6 | 10 | 65 | 173 | 164 | 95 | 34 | 9 | 0 | 0 | 563 |
| 8:00 | 0 | 3 | 7 | 5 | 27 | 114 | 157 | 173 | 78 | 15 | 2 | 0 | 0 | 581 |
| 9:00 | 2 | 2 | 4 | 10 | 35 | 128 | 173 | 139 | 52 | 8 | 2 | 3 | 1 | 559 |
| 10:00 | 2 | 2 | 4 | 11 | 75 | 140 | 184 | 131 | 34 | 6 | 0 | 0 | 0 | 589 |
| 11:00 | 0 | 6 | 1 | 15 | 54 | 145 | 237 | 119 | 36 | 5 | 0 | 0 | 0 | 618 |
| 12:00 PM | 2 | 6 | 6 | 7 | 63 | 136 | 205 | 143 | 66 | 17 | 0 | 2 | 0 | 653 |
| 1:00 | 2 | 3 | 3 | 9 | 52 | 156 | 230 | 143 | 59 | 11 | 1 | 1 | 0 | 670 |
| 2:00 | 2 | 7 | 2 | 7 | 38 | 169 | 216 | 183 | 66 | 9 | 2 | 0 | 0 | 701 |
| 3:00 | 3 | 4 | 7 | 9 | 30 | 119 | 247 | 247 | 100 | 32 | 1 | 0 | 0 | 799 |
| 4:00 | 0 | 8 | 12 | 14 | 34 | 167 | 276 | 223 | 89 | 25 | 5 | 0 | 0 | 853 |
| 5:00 | 4 | 6 | 3 | 1 | 29 | 158 | 258 | 218 | 83 | 14 | 4 | 0 | 0 | 778 |
| 6:00 | 2 | 2 | 3 | 10 | 30 | 110 | 186 | 126 | 33 | 5 | 1 | 0 | 0 | 508 |
| 7:00 | 1 | 2 | 2 | 0 | 11 | 61 | 108 | 95 | 42 | 10 | 0 | 0 | 0 | 332 |
| 8:00 | 0 | 1 | 0 | 2 | 9 | 58 | 80 | 69 | 26 | 6 | 1 | 0 | 0 | 252 |
| 9:00 | 0 | 1 | 0 | 4 | 8 | 40 | 63 | 53 | 26 | 3 | 1 | 0 | 0 | 199 |
| 10:00 | 0 | 0 | 0 | 0 | 7 | 20 | 37 | 33 | 18 | 5 | 2 | 0 | 0 | 122 |
| 11:00 | 0 | 0 | 0 | 0 | 5 | 6 | 13 | 24 | 9 | 0 | 0 | 0 | 0 | 57 |
| Total | 20 | 62 | 57 | 118 | 556 | 1923 | 3090 | 2603 | 1066 | 277 | 44 | 7 | 2 | 9825 |
| Percentile |  |  |  | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
| Speed |  |  |  | 37 | 43 | 50 | 53 |  |  |  |  |  |  |  |
| Mean Speed (Average) |  |  |  | 43.4 |  |  |  |  |  |  |  |  |  |  |
| 10 MPH Pace Speed |  |  |  | 40-49 |  |  |  |  |  |  |  |  |  |  |
| Number in Pace |  |  |  | 5679 |  |  |  |  |  |  |  |  |  |  |
| Percent in Pace |  |  |  | 57.8\% |  |  |  |  |  |  |  |  |  |  |
| Number > 45 MPH |  |  |  | 3999 |  |  |  |  |  |  |  |  |  |  |
| Percent > 45 MPH |  |  |  | 40.7\% |  |  |  |  |  |  |  |  |  |  |

Location : Route 20
96500001
Location : North of Gifford Road
City/State: Sturbridge, MA
Direction: NB

| $\begin{array}{r} \hline \text { 3/9/2023 } \\ \text { Time } \end{array}$ | $\begin{aligned} & 0-15 \\ & \mathrm{MPH} \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \\ \hline \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & >70 \\ & \mathrm{MPH} \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 0 | 0 | 3 | 6 | 6 | 9 | 6 | 0 | 0 | 0 | 0 | 30 |
| 1:00 | 0 | 0 | 0 | 1 | 1 | 5 | 7 | 6 | 4 | 1 | 0 | 0 | 0 | 25 |
| 2:00 | 0 | 0 | 0 | 0 | 1 | 5 | 15 | 8 | 4 | 0 | 0 | 0 | 0 | 33 |
| 3:00 | 0 | 0 | 0 | 0 | 0 | 4 | 12 | 13 | 4 | 0 | 0 | 0 | 0 | 33 |
| 4:00 | 0 | 0 | 0 | 0 | 1 | 8 | 31 | 33 | 16 | 4 | 2 | 0 | 0 | 95 |
| 5:00 | 0 | 1 | 0 | 0 | 6 | 12 | 52 | 78 | 49 | 23 | 1 | 3 | 0 | 225 |
| 6:00 | 1 | 2 | 0 | 5 | 12 | 47 | 121 | 148 | 93 | 33 | 14 | 0 | 0 | 476 |
| 7:00 | 0 | 7 | 2 | 2 | 10 | 50 | 152 | 168 | 95 | 41 | 7 | 5 | 0 | 539 |
| 8:00 | 1 | 8 | 4 | 3 | 43 | 97 | 183 | 149 | 76 | 28 | 6 | 1 | 0 | 599 |
| 9:00 | 0 | 3 | 5 | 5 | 37 | 100 | 189 | 145 | 49 | 11 | 7 | 0 | 1 | 552 |
| 10:00 | 5 | 5 | 9 | 13 | 57 | 167 | 158 | 125 | 52 | 6 | 3 | 0 | 0 | 600 |
| 11:00 | 1 | 5 | 5 | 3 | 53 | 172 | 201 | 139 | 52 | 6 | 1 | 0 | 0 | 638 |
| 12:00 PM | 2 | 2 | 4 | 5 | 24 | 149 | 204 | 182 | 65 | 18 | 0 | 1 | 0 | 656 |
| 1:00 | 3 | 12 | 3 | 11 | 37 | 140 | 215 | 151 | 65 | 23 | 2 | 1 | 0 | 663 |
| 2:00 | 1 | 7 | 1 | 7 | 38 | 143 | 216 | 172 | 72 | 19 | 3 | 0 | 0 | 679 |
| 3:00 | 5 | 8 | 4 | 7 | 48 | 190 | 250 | 214 | 84 | 34 | 5 | 2 | 0 | 851 |
| 4:00 | 2 | 5 | 2 | 10 | 35 | 147 | 301 | 221 | 101 | 19 | 10 | 0 | 0 | 853 |
| 5:00 | 2 | 2 | 5 | 8 | 35 | 193 | 241 | 178 | 106 | 14 | 5 | 0 | 0 | 789 |
| 6:00 | 0 | 9 | 4 | 6 | 32 | 126 | 172 | 120 | 49 | 7 | 0 | 0 | 0 | 525 |
| 7:00 | 1 | 2 | 1 | 1 | 25 | 87 | 129 | 100 | 31 | 11 | 2 | 0 | 0 | 390 |
| 8:00 | 1 | 2 | 1 | 2 | 9 | 38 | 83 | 72 | 37 | 2 | 3 | 0 | 0 | 250 |
| 9:00 | 0 | 1 | 0 | 2 | 8 | 34 | 71 | 55 | 26 | 4 | 3 | 0 | 0 | 204 |
| 10:00 | 0 | 1 | 0 | 0 | 7 | 16 | 28 | 32 | 16 | 3 | 4 | 1 | 0 | 108 |
| 11:00 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 11 | 4 | 0 | 2 | 0 | 0 | 25 |
| Total | 25 | 82 | 50 | 91 | 522 | 1936 | 3045 | 2529 | 1156 | 307 | 80 | 14 | 1 | 9838 |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | $37$ | 43 | 50 | 54 |  |  |  |  |  |  |  |
|  |  | n Speed | (Average) | $43.6$ |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | 40-49 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | r in Pace | 5562 |  |  |  |  |  |  |  |  |  |  |
|  |  | Perce | t in Pace | $56.5 \%$ |  |  |  |  |  |  |  |  |  |  |
|  |  | Number | $45 \mathrm{MPH}$ | $4087$ |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent > | 45 MPH | 41.5\% |  |  |  |  |  |  |  |  |  |  |
| Grand Total | 45 | 144 | 107 | 209 | 1078 | 3859 | 6135 | 5132 | 2222 | 584 | 124 | 21 | 3 | 19663 |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 37 | 43 | 50 | 54 |  |  |  |  |  |  |  |
|  |  | n Speed | Average) | 43.5 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | $40-49$ |  |  |  |  |  |  |  |  |  |  |
|  |  | Numb | r in Pace | $11242$ |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 57.2\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number > | 45 MPH | 8086 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent $>$ | 45 MPH | 41.1\% |  |  |  |  |  |  |  |  |  |  |

## Accurate Counts

978-664-2565

Location : Route 20
96500001
Location : North of Gifford Road
City/State: Sturbridge, MA
Direction: SB

| $\begin{array}{r} 3 / 8 / 2023 \\ \text { Time } \\ \hline \end{array}$ | $\begin{aligned} & 0-15 \\ & \mathrm{MPH} \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \\ \hline \end{gathered}$ | $\begin{gathered} \hline>25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >30- \\ 35 \mathrm{MPH} \\ \hline \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & >70 \\ & \mathrm{MPH} \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 0 | 1 | 3 | 6 | 7 | 12 | 9 | 1 | 2 | 1 | 1 | 43 |
| 1:00 | 0 | 0 | 0 | 1 | 0 | 4 | 6 | 13 | 6 | 3 | 0 | 0 | 1 | 34 |
| 2:00 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 9 | 5 | 5 | 2 | 0 | 0 | 26 |
| 3:00 | 0 | 0 | 0 | 0 | 0 | 3 | 8 | 18 | 8 | 2 | 0 | 0 | 0 | 39 |
| 4:00 | 0 | 0 | 1 | 0 | 2 | 4 | 13 | 31 | 20 | 7 | 0 | 0 | 0 | 78 |
| 5:00 | 0 | 0 | 1 | 0 | 2 | 9 | 31 | 74 | 65 | 23 | 4 | 1 | 0 | 210 |
| 6:00 | 0 | 0 | 2 | 8 | 11 | 20 | 43 | 140 | 96 | 40 | 18 | 4 | 0 | 382 |
| 7:00 | 1 | 0 | 2 | 10 | 13 | 46 | 104 | 158 | 147 | 60 | 21 | 2 | 1 | 565 |
| 8:00 | 0 | 0 | 3 | 19 | 28 | 57 | 128 | 125 | 106 | 40 | 9 | 7 | 1 | 523 |
| 9:00 | 0 | 3 | 10 | 19 | 23 | 70 | 149 | 153 | 102 | 33 | 3 | 0 | 0 | 565 |
| 10:00 | 0 | 3 | 6 | 23 | 28 | 96 | 161 | 150 | 87 | 23 | 6 | 0 | 0 | 583 |
| 11:00 | 0 | 4 | 12 | 15 | 60 | 138 | 195 | 125 | 72 | 16 | 1 | 1 | 0 | 639 |
| 12:00 PM | 1 | 2 | 9 | 24 | 53 | 142 | 190 | 193 | 55 | 8 | 4 | 0 | 0 | 681 |
| 1:00 | 0 | 2 | 7 | 25 | 59 | 125 | 203 | 163 | 76 | 28 | 4 | 0 | 0 | 692 |
| 2:00 | 1 | 9 | 7 | 19 | 41 | 145 | 267 | 177 | 104 | 26 | 14 | 1 | 0 | 811 |
| 3:00 | 1 | 7 | 14 | 44 | 61 | 104 | 224 | 194 | 123 | 40 | 5 | 1 | 2 | 820 |
| 4:00 | 2 | 9 | 10 | 26 | 46 | 106 | 268 | 296 | 159 | 48 | 21 | 1 | 0 | 992 |
| 5:00 | 1 | 1 | 10 | 25 | 49 | 105 | 250 | 258 | 129 | 24 | 5 | 2 | 0 | 859 |
| 6:00 | 0 | 1 | 4 | 21 | 35 | 80 | 161 | 191 | 108 | 18 | 4 | 0 | 0 | 623 |
| 7:00 | 0 | 2 | 2 | 13 | 15 | 46 | 106 | 150 | 87 | 31 | 4 | 1 | 0 | 457 |
| 8:00 | 0 | 0 | 1 | 0 | 8 | 35 | 92 | 102 | 52 | 16 | 4 | 0 | 0 | 310 |
| 9:00 | 2 | 0 | 0 | 3 | 1 | 22 | 50 | 67 | 36 | 11 | 3 | 1 | 1 | 197 |
| 10:00 | 0 | 1 | 0 | 0 | 3 | 18 | 37 | 39 | 20 | 7 | 2 | 2 | 1 | 130 |
| 11:00 | 0 | 0 | 1 | 0 | 0 | 5 | 24 | 22 | 13 | 8 | 2 | 2 | 0 | 77 |
| Total | 9 | 44 | 102 | 296 | 541 | 1386 | 2722 | 2860 | 1685 | 518 | 138 | 27 | 8 | 10336 |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
| Speed |  |  |  | 37 | 45 | 52 | 56 |  |  |  |  |  |  |  |
| Mean Speed (Average) |  |  |  | 44.8 |  |  |  |  |  |  |  |  |  |  |
| 10 MPH Pace Speed |  |  |  | 40-49 |  |  |  |  |  |  |  |  |  |  |
| Number in Pace |  |  |  | 5553 |  |  |  |  |  |  |  |  |  |  |
| Percent in Pace |  |  |  | 53.7\% |  |  |  |  |  |  |  |  |  |  |
| Number > 45 MPH |  |  |  | 5236 |  |  |  |  |  |  |  |  |  |  |
| Percent > 45 MPH |  |  |  | 50.7\% |  |  |  |  |  |  |  |  |  |  |

Location : Route 20
96500001
Location : North of Gifford Road
City/State: Sturbridge, MA
Direction: SB

| $\begin{array}{r} \hline 3 / 9 / 2023 \\ \text { Time } \end{array}$ | $\begin{aligned} & 0-15 \\ & \mathrm{MPH} \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & >70 \\ & \mathrm{MPH} \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 0 | 2 | 6 | 6 | 10 | 9 | 5 | 0 | 2 | 0 | 0 | 40 |
| 1:00 | 0 | 0 | 0 | 0 | 1 | 2 | 16 | 15 | 9 | 0 | 0 | 0 | 0 | 43 |
| 2:00 | 0 | 0 | 0 | 0 | 0 | 1 | 6 | 8 | 9 | 4 | 0 | 0 | 0 | 28 |
| 3:00 | 0 | 1 | 1 | 1 | 0 | 2 | 8 | 11 | 10 | 3 | 0 | 0 | 0 | 37 |
| 4:00 | 0 | 0 | 0 | 2 | 4 | 1 | 12 | 27 | 25 | 5 | 2 | 0 | 0 | 78 |
| 5:00 | 0 | 0 | 0 | 0 | 1 | 6 | 36 | 56 | 66 | 33 | 14 | 1 | 1 | 214 |
| 6:00 | 0 | 0 | 1 | 5 | 17 | 30 | 59 | 137 | 108 | 84 | 32 | 4 | 0 | 477 |
| 7:00 | 0 | 5 | 8 | 26 | 53 | 90 | 222 | 280 | 231 | 96 | 12 | 6 | 0 | 1029 |
| 8:00 | 8 | 19 | 62 | 108 | 167 | 216 | 275 | 239 | 138 | 12 | 4 | 0 | 0 | 1248 |
| 9:00 | 1 | 3 | 10 | 25 | 47 | 77 | 211 | 165 | 85 | 37 | 6 | 1 | 0 | 668 |
| 10:00 | 4 | 2 | 9 | 17 | 39 | 73 | 143 | 151 | 101 | 31 | 6 | 1 | 1 | 578 |
| 11:00 | 0 | 3 | 2 | 9 | 56 | 113 | 253 | 181 | 77 | 17 | 6 | 0 | 0 | 717 |
| 12:00 PM | 0 | 1 | 5 | 14 | 35 | 122 | 207 | 176 | 97 | 41 | 3 | 0 | 0 | 701 |
| 1:00 | 2 | 2 | 11 | 24 | 37 | 119 | 202 | 225 | 94 | 23 | 3 | 0 | 1 | 743 |
| 2:00 | 1 | 3 | 5 | 31 | 76 | 106 | 209 | 200 | 126 | 35 | 1 | 2 | 0 | 795 |
| 3:00 | 1 | 9 | 10 | 37 | 67 | 151 | 250 | 240 | 97 | 33 | 8 | 0 | 0 | 903 |
| 4:00 | 3 | 7 | 20 | 46 | 56 | 133 | 281 | 242 | 129 | 41 | 3 | 2 | 1 | 964 |
| 5:00 | 0 | 5 | 20 | 27 | 49 | 134 | 260 | 255 | 96 | 24 | 2 | 0 | 0 | 872 |
| 6:00 | 0 | 1 | 1 | 15 | 30 | 98 | 178 | 182 | 90 | 25 | 6 | 2 | 0 | 628 |
| 7:00 | 2 | 0 | 4 | 6 | 23 | 51 | 120 | 139 | 67 | 29 | 6 | 3 | 1 | 451 |
| 8:00 | 1 | 2 | 1 | 7 | 23 | 28 | 82 | 121 | 44 | 22 | 5 | 3 | 0 | 339 |
| 9:00 | 0 | 1 | 0 | 3 | 5 | 35 | 50 | 58 | 41 | 11 | 1 | 0 | 0 | 205 |
| 10:00 | 0 | 1 | 0 | 1 | 1 | 10 | 47 | 40 | 29 | 11 | 3 | 0 | 1 | 144 |
| 11:00 | 0 | 0 | 0 | 0 | 0 | 3 | 2 | 6 | 8 | 2 | 0 | 0 | 0 | 21 |
| Total | 23 | 65 | 170 | 406 | 793 | 1607 | 3139 | 3163 | 1782 | 619 | 125 | 25 | 6 | 11923 |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 36 | 45 | 52 | 56 |  |  |  |  |  |  |  |
|  |  | n Speed | Average) | 44.1 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pa | ce Speed | 40-49 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | $r$ in Pace | 6271 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 52.6\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number > | $45 \mathrm{MPH}$ | 5720 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent $>$ | 45 MPH | 48.0\% |  |  |  |  |  |  |  |  |  |  |
| Grand Total | 32 | 109 | 272 | 702 | 1334 | 2993 | 5861 | 6023 | 3467 | 1137 | 263 | 52 | 14 | 22259 |
|  |  |  | Percentile | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
|  |  |  | Speed | 37 | 45 | 52 | 56 |  |  |  |  |  |  |  |
|  |  | n Speed | Average) | 44.4 |  |  |  |  |  |  |  |  |  |  |
|  |  | MPH Pac | Ce Speed | 40-49 |  |  |  |  |  |  |  |  |  |  |
|  |  | Numbe | r in Pace | 11823 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percen | t in Pace | 53.1\% |  |  |  |  |  |  |  |  |  |  |
|  |  | Number $>$ | 45 MPH | 10957 |  |  |  |  |  |  |  |  |  |  |
|  |  | Percent $>$ | 45 MPH | 49.2\% |  |  |  |  |  |  |  |  |  |  |

## Accurate Counts

978-664-2565

Location : Route 20
96500001
Location : North of Gifford Road
City/State: Sturbridge, MA
Direction: Combined

| $\begin{array}{r} \hline 3 / 8 / 2023 \\ \text { Time } \end{array}$ | $\begin{aligned} & 0-15 \\ & \mathrm{MPH} \end{aligned}$ | $\begin{gathered} >15- \\ 20 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >20- \\ 25 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >25- \\ 30 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >30- \\ 35 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >35- \\ 40 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >40- \\ 45 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >45- \\ 50 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >50- \\ 55 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >55- \\ 60 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >60- \\ 65 \mathrm{MPH} \end{gathered}$ | $\begin{gathered} >65- \\ 70 \mathrm{MPH} \end{gathered}$ | $\begin{aligned} & >70 \\ & \mathrm{MPH} \end{aligned}$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 AM | 0 | 0 | 0 | 4 | 7 | 18 | 27 | 29 | 22 | 1 | 2 | 1 | 2 | 113 |
| 1:00 | 0 | 1 | 0 | 3 | 5 | 13 | 18 | 17 | 10 | 4 | 0 | 0 | 1 | 72 |
| 2:00 | 0 | 0 | 0 | 0 | 4 | 5 | 13 | 19 | 9 | 7 | 2 | 0 | 0 | 59 |
| 3:00 | 0 | 0 | 1 | 0 | 3 | 9 | 20 | 31 | 9 | 6 | 1 | 0 | 0 | 80 |
| 4:00 | 0 | 0 | 1 | 1 | 4 | 27 | 36 | 63 | 29 | 12 | 3 | 1 | 0 | 177 |
| 5:00 | 0 | 1 | 1 | 1 | 10 | 35 | 78 | 145 | 92 | 44 | 7 | 1 | 0 | 415 |
| 6:00 | 0 | 2 | 2 | 9 | 24 | 70 | 168 | 313 | 192 | 79 | 24 | 4 | 0 | 887 |
| 7:00 | 1 | 5 | 4 | 16 | 23 | 111 | 277 | 322 | 242 | 94 | 30 | 2 | 1 | 1128 |
| 8:00 | 0 | 3 | 10 | 24 | 55 | 171 | 285 | 298 | 184 | 55 | 11 | 7 | 1 | 1104 |
| 9:00 | 2 | 5 | 14 | 29 | 58 | 198 | 322 | 292 | 154 | 41 | 5 | 3 | 1 | 1124 |
| 10:00 | 2 | 5 | 10 | 34 | 103 | 236 | 345 | 281 | 121 | 29 | 6 | 0 | 0 | 1172 |
| 11:00 | 0 | 10 | 13 | 30 | 114 | 283 | 432 | 244 | 108 | 21 | 1 | 1 | 0 | 1257 |
| 12:00 PM | 3 | 8 | 15 | 31 | 116 | 278 | 395 | 336 | 121 | 25 | 4 | 2 | 0 | 1334 |
| 1:00 | 2 | 5 | 10 | 34 | 111 | 281 | 433 | 306 | 135 | 39 | 5 | 1 | 0 | 1362 |
| 2:00 | 3 | 16 | 9 | 26 | 79 | 314 | 483 | 360 | 170 | 35 | 16 | 1 | 0 | 1512 |
| 3:00 | 4 | 11 | 21 | 53 | 91 | 223 | 471 | 441 | 223 | 72 | 6 | 1 | 2 | 1619 |
| 4:00 | 2 | 17 | 22 | 40 | 80 | 273 | 544 | 519 | 248 | 73 | 26 | 1 | 0 | 1845 |
| 5:00 | 5 | 7 | 13 | 26 | 78 | 263 | 508 | 476 | 212 | 38 | 9 | 2 | 0 | 1637 |
| 6:00 | 2 | 3 | 7 | 31 | 65 | 190 | 347 | 317 | 141 | 23 | 5 | 0 | 0 | 1131 |
| 7:00 | 1 | 4 | 4 | 13 | 26 | 107 | 214 | 245 | 129 | 41 | 4 | 1 | 0 | 789 |
| 8:00 | 0 | 1 | 1 | 2 | 17 | 93 | 172 | 171 | 78 | 22 | 5 | 0 | 0 | 562 |
| 9:00 | 2 | 1 | 0 | 7 | 9 | 62 | 113 | 120 | 62 | 14 | 4 | 1 | 1 | 396 |
| 10:00 | 0 | 1 | 0 | 0 | 10 | 38 | 74 | 72 | 38 | 12 | 4 | 2 | 1 | 252 |
| 11:00 | 0 | 0 | 1 | 0 | 5 | 11 | 37 | 46 | 22 | 8 | 2 | 2 | 0 | 134 |
| Total | 29 | 106 | 159 | 414 | 1097 | 3309 | 5812 | 5463 | 2751 | 795 | 182 | 34 | 10 | 20161 |
| Percentile |  |  |  | 15th | 50th | 85th | 95th |  |  |  |  |  |  |  |
| Speed |  |  |  | 37 | 44 | 51 | 55 |  |  |  |  |  |  |  |
| Mean Speed (Average) |  |  |  | 44.1 |  |  |  |  |  |  |  |  |  |  |
| 10 MPH Pace Speed |  |  |  | 40-49 |  |  |  |  |  |  |  |  |  |  |
| Number in Pace |  |  |  | 11232 |  |  |  |  |  |  |  |  |  |  |
| Percent in Pace |  |  |  | 55.7\% |  |  |  |  |  |  |  |  |  |  |
| Number > 45 MPH |  |  |  | 9236 |  |  |  |  |  |  |  |  |  |  |
| Percent > 45 MPH |  |  |  | 45.8\% |  |  |  |  |  |  |  |  |  |  |

City/State: Sturbridge, MA
Direction: Combined


## Welome aboard the WRTA!

This route timetable shows the times of departure at major stops along the route and contains route maps and other tant information. Additional information be can obtained by calling the WRTA Information
at (508) 791-WRTA (9782)
or visit our website at www.TheRTA.com.


ACCESSIBILITY: All WRTA buses are wheelchair accessible and feature bicycle racks for two bicycles. For TTY service cal Massachusetts Relay TTY (800) 439-2370. For information accommodalion 2 .
option PROPER IDENTIFICATION: One of the following valid iden
tification cards must be shown to the driver each time you board SENIOR WRTA Senior I.D. card DISABLED. . . Statewide Access Pass / WRTA ADA Photo I.D. MEDICARE . . . . . . . . . . . . . . Medicare card with Photo I.D. HOLIDAY SERVICE: Saturday* Service is provided on Martin Luther King, Jr. Day, Presidents' Day, Patriots' Day, Columbus Day, and the day after Thanksgiving.
Weekday Service is provided on Veterans' Day
Routes $29,33,42$ and community shuttles operate on a weekday schedule on
these holidays. Routes 19 and 30 operate on a modified Saturday schedule on these holidays.
NO SERVICE ON: New Years Day; Memorial Day; Independence Day; Labor Day; Thanksgiving Day; Christmas Day

Please...NO Smoking, Eating, Drinking or Music **The Federal Transit Administration permits transit systems to set a minimum age limit for children riding without a parent or guardian. The
WRTA has set this age limit at Nine (9) years old. In order to ensure compliance with this age limit, operators may question a child seeking to
board a bus who appears, in the operator's opinion, to be Eight (8) years board a bus who appears, in the operator's opinion, to be Eight (8) years
old or younger. If an operator is not satisfied with a child's answer, the old or younger. II an operator is not satistied with a child's answer, tue
operator may call for assistance from a WRTA supervisor and/or public safety personnel. This policy applies to Paratransit Service as well.

## OUTBOUND = سITM <br> WEEKDAYS

See the map for matching timepoint locations

| 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| BUS | BUS |  |  | BUS | BUS | S |
| Sta |  |  | Rt 20 | Optical | Southbri |  |
| Hub | Mall | Rt 20 | Rt | Southbridge | Library | Plaza |
| 535a | 550a | 558a | 610a | 620a | 624a | 629a |
| 735a | 750a | 758a | 810a | 820a | 824a | 829a |
| 935a | 950a | 958a | 1010a | 1015a | 1019a | 1029a |
| 1135a | 1150a | 1158a | 1210p | 1215p | 1219p | 1229p |
| 135p | 150p | 158p | 210p | 215p | 219p | 229p |
| 335p | 350p | 358p | 410p | 415p | 419p | 429p |
| 535p | 550p | 558p | 610p | 615p | 619p | 629p |

THIS ROUTE DOES NOT PICK UP OR DROP OFF ON ROUTE 20 *(SEE MAP INSERT)

## SATURDAYS

See the map for matching timepoint locations

| 1 | 2 | 3 | 4 | 5 | 6 | 7 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { BUS } \\ & \text { STARTS } \end{aligned}$ | $\begin{gathered} \text { BUS } \\ \text { LEAVES } \end{gathered}$ | $\begin{gathered} \text { BUS } \\ \text { LEAVES } \end{gathered}$ | $\begin{gathered} \text { BUS } \\ \text { LEAVES } \end{gathered}$ | BUS Leaves | $\begin{aligned} & \text { BUS } \\ & \text { LEAVES } \end{aligned}$ | $\begin{aligned} & \text { BUS } \\ & \text { ENDS } \end{aligned}$ |
| Union Station | Auburn | Rt 12 \& | Rt 20 \& | Optical Dr | Southbridge | Big Bunny |
| Hub | Mall | Rt 20 | Rt 169 | Southbridge | Library | Plaza |
| 800a | 815a | 823a | 835a | 845a | 849a | 854a |
| 1000a | 1015a | 1023a | 1035a | 1045a | 1049a | 1054a |
| 1200p | 1215p | 1223p | 1235p | 1245p | 1249p | 1254p |
| 200p | 215p | 223p | 235p | 245p | 249p | 254p |
| 400p | 415p | 423p | 435p | 445p | 449p | 454p |
| 600p | 615p | 623p | 635p | 645p | 649p | 654p |

## INBOUND

$=\frac{\ln \sqrt{n} \mid}{0}$

## WEEKDAYS

See the map for matching timepoint locations

| 7 | 4 | 3 | 2 | 1 |
| :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { BUS } \\ & \text { LEAVES } \end{aligned}$ | $\begin{gathered} \text { BUS } \\ \text { LEAVES } \end{gathered}$ | $\begin{aligned} & \text { BUS } \\ & \text { LEAVES } \end{aligned}$ | $\begin{gathered} \text { BUS } \\ \text { LEAVES } \end{gathered}$ | $\begin{aligned} & \hline \text { BUS } \\ & \text { ENDS } \end{aligned}$ |
| Big Bunny | Rt 20 \& | Rt 12 \& | Auburn | Union Statio |
| Plaza | Rt 169 | Rt20 | Mall | Hub |
| 633a | 643a | 705a | 715a | 735a |
| 833a | 843a | 905a | 915a | 935a |
| 1033a | 1043a | 1105a | 1115a | 1135a |
| 1233p | 1243p | 105p | 115p | 135p |
| 233p | 243p | 305p | 315p | 335p |
| 433p | 443p | 505p | 515p | 535p |
| 633p | 643p | 705p | 715p | 735p |

THIS ROUTE DOES NOT PICK UP OR DROP OFF ON ROUTE 20 *(SEE MAP INSERT)

## SATURDAYS

See the map for matching timepoint locations

| 6 | 4 | 3 | 2 | 1 |
| :---: | :---: | :---: | :---: | :---: |
|  | BUS LEAVES Rt 20 \& Rt 169 | $\begin{aligned} & \text { BUS } \\ & \text { LEAVES } \\ & \text { Rt 12 \& } \\ & \text { Rit } 20 \end{aligned}$ | BUS LEAVES <br> Auburn <br> Mall |  |
| 858a | 908a | 930a | 940a | 1000a |
| 1058a | 1108a | 1130a | 1140a | 1200p |
| 1258p | 108p | 130p | 140p | 200p |
| 258p | 308p | 330p | 340p | 400p |
| 458p | 508p | 530p | 540p | 600 p |
| 658p | 708p | 730p | 740p | 800p |

## Route 29

## UNION STATION HUB -

 SOUTHBRIDGE - CHARLTON
## Effective Date: January 25, 2020

## Worcester Regional Transit Authority



## Serving:

Auburn Mall
Optical Drive - Southbridge
Union Station Hub
Southbridge Library
Big Bunny Plaza

Translation
English: If this information is needed in another language, please visit www.therta.com
and use the Google Translate feature.
Portuguse: Se esta informacão é necessíria em outro idioma, por favor visite
wwwwtherta. come use o Googile Translate.
Spanish: Si necesita esta
utilice Googge Translate. French: Si vous désirez ces renseignements dans une autre langue, prière de vous server
de Google Translate qui se trouve al 1 'adresse se suivante: www.thera. com. Polish: Jeśli t a informacia jest potrrzebna w innmm jezzku, proszę odwiedzic
www.therta. oom i korzystać $z$ Googe Translate funkci|.

 Swahili: Kama unanhitai habari hii katika ny
panaandikwa Google rranslate" hapa juu.
Note: French, Spanish, Polish and Portuguese translations were created by human


For Transit Information Call 508-791-9782 or visit www.therta.com


## MassDOT Top Crash Locations



## General Background Traffic Growth - Daily Traffic Volumes

| CITYıTOWN | ROUTE/STREET | Location | 2009 | 2010 | 2011 | 2012 | 2013 | 2014 | 2015 | 2016 | 2017 | 2018 | 2019 | Annual <br> Growth Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Sturbridge | Hall Road | South of Route 20 |  |  |  |  |  | 4,359 | 4,459 | 4,735 | 4,815 | 4,829 | 4,810 | 2.15\% |
| Sturbridge | Podunk Pike | North of Route 20 |  |  |  |  |  | 7,438 | 7,520 | 7,618 | 6,233 | 6,320 | 6,345 | -3.89\% |
| Sturbridge | Main Street | Under Interstate 84 |  |  |  |  |  | 12,925 | 13,067 | 13,237 | 13,383 | 13,570 | 13,624 | 1.11\% |
| Sturbridge | Interstate 84 | North of Route 20 | 62,300 | 61,600 | 62,934 | 53,795 | 51,213 | 54,862 | 57,166 | 62,036 | 63,153 | 63,785 | 64,423 | 0.66\% |
| Sturbridge | Interstate 84 | South of Route 20 | 54,652 | 55,400 | 53,645 | 51,486 | 52,177 | 52,522 | 55,467 | 55,862 | 56,868 | 57,169 | 57,566 | 0.68\% |
| Sturbridge | New Boston Road | North of Route 20 |  |  |  |  |  | 3,204 | 3,278 | 3,481 | 3,540 | 3,551 | 3,537 | 2.16\% |



## WEEKDAY EVENING PEAK HOUR (4:15-5:15 PM)




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Trip Generation Manual，11th Ed v
760 SEARCH BY LAND USE COD

LAND USE GROUP：
（700－799）Office v

LAND USE：
760 －Research and Development Center $v$

LAND USE SUBCATEGORY：
All Sites $v$
SETTING／LOCATION：
General Urban／Suburban
INDEPENDENT VARIABLE（IV）：$\quad$ v

1000 Sq．Ft．GF $\checkmark$

TME PERIOD：
Weekday
RIP TYPE：
Vehicle
NER IV VALUE TO CALCULATE TRIPS： $8 \quad$ Calculate

## Data Plot and Equation



DATA STATISTICS

## Land Use：

Research and Development Center（760）Click for Description and Data Plots
Independent Variable：
1000 Sq．Ft．GFA
Time Period：
Weekday
Setting／Location：
General Urban／Suburban
Trip Type：
Vehicle
Number of Studies：
22
Avg． 1000 Sq．Ft．GFA：
179
Average Rate：
11.08

Range of Rates：
3．48－24．95
Standard Deviation：
4.45

Fitted Curve Equation：
$\mathrm{T}=9.70(\mathrm{X})+247.71$
$\mathrm{R}^{2}$ ：
0.89

Directional Distribution
$50 \%$ entering， $50 \%$ exiting
Calculated Trip Ends：
Average Rate： 89 （Total）， 44 （Entry）， 45 （Exit） Fitted Curve： 325 （Total）， 163 （Entry）， 162 （Exit）

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| DATA source: |
| :--- |
| Trip Generation Manual, 11th Ed |
| SEARCH BY LAND USE CODE: |
| LAND USE GRoup: |
| (700-799) Office |
| LAND USE: |
| 760 - Research and Development Center |
| LAND USE SUBCATEGORY: |
| All Sites |
| SETTING/LOCATION: |
| General Urban/Suburban |
| INDEPENDENT VARIABLE (IV): |
| 1000 Sq. Ft. GFA |
| TIME PERIOD: |
| Weekday, Peak Hour of Adjacent Street Traffic, $\sim$ v |
| TRIP TYPE: |
| Vehicle |

ENTER IV VALUE TO CALCULATE TRIPS:


Data Plot and Equation
DATA STATISTICS

## Land Use:

Research and Development Center (760) Click for Description and Data Plots
Independent Variable:
1000 Sq. Ft. GFA
Time Period:
Weekday
Peak Hour of Adjacent Street Traffic
One Hour Between 7 and 9 a.m.
Setting/Location:
General Urban/Suburban
Trip Type:
Vehicle
Number of Studies:
39
Avg. 1000 Sq. Ft. GFA:
173
Average Rate:
1.03

Range of Rates:
0.17-3.73

Standard Deviation:
0.65

Fitted Curve Equation:
$\mathrm{T}=0.89(\mathrm{X})+24.54$
$\mathrm{R}^{2}$ :
0.70

Directional Distribution
$82 \%$ entering, $18 \%$ exiting
Calculated Trip Ends:
Average Rate: 8 (Total), 7 (Entry), 1 (Exit) Fitted Curve: 32 (Total), 26 (Entry), 6 (Exit)

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| LAND USE： |
| 760 －Research and Development Center |
| LAND USE SUBCATEGORY： |
| All Sites |
| SETTING／LOCATION： |
| General Urban／Suburban |
| INDEPENDENT VARIABLE（IV）： |
| 1000 Sq．Ft．GFA |
| TIME PERIOD： |
| Weekday，Peak Hour of Adjacent Street Traffic，$\sim$ v |
| TRIP TYPE： |
| Vehicle |

ENTER IV VALUE TO CALCULATE TRIPS：


Data Plot and Equation
DATA STATISTICS

## Land Use：

Research and Development Center（760）Click for Description and Data Plots
Independent Variable：
1000 Sq．Ft．GFA
Time Period：
Weekday
Peak Hour of Adjacent Street Traffic
One Hour Between 4 and 6 p．m．
Setting／Location：
General Urban／Suburba
Trip Type：
Vehicle
Number of Studies：
39
Avg． 1000 Sq．Ft．GFA：
173
Average Rate：
0.98

Range of Rates：
0．13－4．13
Standard Deviation：
0.64

Fitted Curve Equation：
$\mathrm{T}=0.84(\mathrm{X})+25.08$
$\mathrm{R}^{2}$ ：
0.70

Directional Distribution
$16 \%$ entering， $84 \%$ exiting
Calculated Trip Ends：
Average Rate： 8 （Total）， 1 （Entry）， 7 （Exit） Fitted Curve： 32 （Total）， 5 （Entry）， 27 （Exit）

## ${ }^{++}$Add－ons to do more

O）Ty Otiss Pro


## Sight Distance Calculations - Route 20

## Stopping Sight Distance:

Equation:
$S S D=1.47 \times V \times t+1.075 \times \frac{V^{2}}{a}$
Variables:
$V=52 \mathrm{mph}$
$t=2.5 s(S S D)$
$a=11.2 \mathrm{ft} / \mathrm{s}$
$S S D=1.47 \times 52 \times 2.5+1.075 \times \frac{52^{2}}{11.2}=450.6 \approx 455$
Intersection Sight Distance: looking to the northeast (turning right from stop):
Equation:
$I S D=1.47 \times V \times t$

Variables:
$V=52 \mathrm{mph}$
$t=6.5 s$ (ISD, right turns for a passenger car)
$I S D=1.47 \times 52 \times 6.5=496.9 \approx 500$
Intersection Sight Distance: looking to the southwest (turning left from stop):
Equation:
$I S D=1.47 \times V \times t$

Variables:
$V=52 \mathrm{mph}$
$t=8.0 s$ (ISD, left turns for a passenger car crossing an additional turning lane)
$I S D=1.47 \times 52 \times 8.0=611.5 \approx \mathbf{6 1 5}$



| Intersection |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 0.3 |  |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |  |
| Lane Configurations | * |  |  | ¢ $\uparrow$ | 性 |  |  |
| Traffic Vol, veh/h | 4 | 23 | 4 | 1071 | 1190 | 1 |  |
| Future Vol, veh/h | 4 | 23 | 4 | 1071 | 1190 | 1 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control | Stop | Stop | Free | Free | Free | Free |  |
| RT Channelized | - | None | - | None |  | None |  |
| Storage Length | 0 | - | - | - | - | - |  |
| Veh in Median Storage, \# | \# 0 | - | - | 0 | 0 | - |  |
| Grade, \% | 0 | - | - | 0 | 0 | - |  |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |  |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 4 | 25 | 4 | 1164 | 1293 | 1 |  |




[^0]:    ${ }^{1}$ Trip Generation, $11^{\text {th }}$ Edition; Institute of Transportation Engineers; Washington, DC; 2021.

[^1]:    ${ }^{2}$ The Bylaw requires a minimum of 1 parking space per two (2) employees (eight (8) employees are envisioned) for a manufacturing and production use. This would require that a minimum of four (4) parking spaces be provided to support the Project.

[^2]:    ${ }^{3}$ A minimum combined travel lane and paved shoulder width of 14-feet is required to support bicycle travel in a shared traveled-way condition.

[^3]:    ${ }^{4}$ MassDOT Statewide Traffic Data Collection; 2019 Weekday Seasonal Factors, Group U4-7.
    ${ }^{5}$ Traffic and Safety Engineering 25\% Design Submission Guidelines; MassDOT; Revised March 31, 2022.

[^4]:    ${ }^{6}$ Ibid 1.

[^5]:    ${ }^{7}$ Ibid 1.

[^6]:    ${ }^{8}$ Highway Capacity Manual, $6{ }^{\text {th }}$ Edition, Transportation Research Board; Washington, DC; 2016.

[^7]:    ${ }^{9}$ A Policy on Geometric Design of Highway and Streets, $7{ }^{\text {th }}$ Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2018.

[^8]:    ${ }^{10}$ Ibid 1.

[^9]:    ${ }^{11}$ Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.

