## MEMORANDUM

TO: Mr. Joseph Trefethen<br>Senior Team Operations Manager<br>Sail Energy<br>210 Commerce Way, Suite 210<br>Portsmouth, NH 03801

DATE: July 22, $2020 \quad$ RE: 8665

SUBJECT: Transportation Impact Assessment<br>Proposed Propane Storage Facility - 59 Technology Park Road Sturbridge, Massachusetts

Vanasse \& Associates, Inc. (VAI) has conducted a Transportation Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed addition of a propane storage facility to the existing Pioneer Oil \& Propane facility located at 59 Technology Park Road in Sturbridge, Massachusetts (hereafter referred to as the "Project"). This study evaluates the following specific areas as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; and identifies and analyzes existing traffic conditions and future traffic conditions, both with and without the Project, along Charlton Road (Route 20) and Picker Road, and at the intersections of Route 20 at Picker Road and Fiske Hill Road and Picker Road at Technology Park Road.

The Project is not expected to result in a material increase in traffic over existing conditions given that propane delivery services are currently provided at the Project site. Any increase in traffic would be associated with the bulk delivery of propane to fill the storage tanks, which is expected to occur one (1) to two (2) times per week. As such, we have concluded that the Project will not result in a material increase in motorist delays or vehicle queueing over existing conditions and that the transportation infrastructure affords sufficient capacity to accommodate the Project in a safe and efficient manner with consideration of the specific recommendations presented herein.

The following details our assessment of the Project.

## PROJECT DESCRIPTION

The Project will entail the addition of a propane storage facility to the existing Pioneer Oil \& Propane facility located at 59 Technology Park Road in Sturbridge, Massachusetts. Pioneer Oil \& Propane currently delivers oil and propane from the Project site which includes above-ground storage tanks for fuel oil; however, the propane is obtained from an off-site vendor. The Project site is bounded by commercial properties and areas of open and wooded space to the north and west; Technology Park Road and a
commercial property to the south; and Technology Park Road to the east. Figure 1 depicts the Project site location in relation to the existing roadway network. Access to the Project site is an will continue to be provided by way of two (2) existing driveways that intersect the west side of Technology Park Road approximately 600 and 900 feet northeast of Picker Road, respectively.

## STUDY METHODOLOGY

This study was prepared in consultation with the Massachusetts Department of Transportation (MassDOT) and the Town of Sturbridge; was performed in accordance with MassDOT's Transportation Impact Assessment (TIA) Guidelines and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports; and was conducted in three distinct stages.

The first stage involved an assessment of existing conditions in the study area and included an inventory of roadway geometrics; pedestrian and bicycle facilities; on-street parking; public transportation services; observations of traffic flow; and collection of pedestrian, bicycle and vehicle counts.

In the second stage of the study, future traffic conditions were projected and analyzed. Specific travel demand forecasts for the Project were assessed along with future traffic demands due to expected traffic growth independent of the Project. A seven-year time horizon was selected for analyses consistent with MassDOT's Transportation Impact Assessment (TIA) Guidelines. The analysis conducted in stage two identifies existing or projected future capacity, safety, and access issues, as these areas relate to the transportation infrastructure.

The third stage of the study presents and evaluates measures to address deficiencies in the transportation infrastructure, if any, identified in stage two of the study.

## EXISTING CONDITIONS

In order to establish the existing conditions context of the Project with respect to the transportation infrastructure, a comprehensive field inventory of existing conditions was conducted in July 2020. The field investigation consisted of an inventory of existing roadway geometrics; traffic volumes; public transportation services; and pedestrian and bicycle facilities; as well as posted speed limits and land use information within the study area. The study area that was assessed for the Project consisted of Route 20, Picker Road and Technology Park Road, and at the intersections of Route 20 at Picker Road and Fiske Hill Road and Picker Road at Technology Park Road. The following summarizes existing conditions within the study area.

## Roadways

## Charlton Road (Route 20)

$>$ Four-lane urban principal arterial roadway under MassDOT jurisdiction
$>$ Traverses the study area in a general east-west direction parallel to and south of Interstate 90 (I-90, a.k.a. the Massachusetts Turnpike) and provides access to I-395 to the east of the Project site (Exit 6) and to I-84 to the west (Exit 3)
$>$ Provides two (2) 12-foot wide travel lanes per direction in the vicinity of the Project site that are separated by a double-yellow centerline, painted median or raised median, with variable width marked shoulders

$>$ Sidewalks are not provided within the study area
$>$ Illumination is provided intermittently by way of street lights mounted on wood poles
$>$ Posted speed limit is 50 miles per hour (mph) in the vicinity of the Project site
$>$ Land use within the study area consists of commercial properties and areas of open and wooded space

Picker Road
$>$ Two-lane local roadway under Town jurisdiction
$>$ Traverses a general north-south direction ending at the driveway to 8 Picker Road approximately 1,200 feet north of Route 20
$>$ Consists of a 24-foot wide paved roadway that accommodates two-way travel with no pavement markings provided
$>$ A posted speed limit is not provided and, as such, the statutory or "prima facie" speed limit is $30 \mathrm{mph}^{1}$
$>$ Sidewalks are not provided
$>$ Land use within the study area consists of the Project site, commercial properties, and areas of open and wooded space

## Technology Park Road

$>$ Two-lane local roadway under Town jurisdiction
$>$ Traverses a general northeast-southwest direction ending in a cul-de-sac approximately 1,400 feet northeast of Picker Road
$>$ Consists of a 22-24-foot wide paved roadway that accommodates two-way travel with no pavement markings provided
$>$ A posted speed limit is not provided and, as such, the statutory or "prima facie" speed limit is 30 mph
$>$ Sidewalks are not provided
$>$ Land use within the study area consists of the Project site, commercial properties, and areas of open and wooded space

## Intersections

Table 1 and Figure 2 summarize lane use, traffic control, and pedestrian and bicycle accommodations at the study area intersections as observed in July 2020.

[^0]Table 1 STUDY AREA INTERSECTION DESCRIPTION

| Intersection | Traffic Control Type ${ }^{\text {a }}$ | No. of Travel Lanes Provided | Shoulder Provided? (Yes/No/Width) | Pedestrian Accommodations? (Yes/No/Description) | Bicycle Accommodations? (Yes/No/Description) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Rte. 20/ <br> Fiske Hill Rd./ Picker Rd. | S | 2 general purpose lanes on Rte. 20 EB; 1 left-turn lane, 1 through lane and 1 through/right lane on Rte. 20 WB; 1 through/right-turn lane on Fiske Hill Rd.; 1 general purpose lane on Picker Rd. Left-turns from Fiske Hill Rd. are prohibited | Yes -1 to 8 feet on Rte. 20 | No | Yes - shared traveledway ${ }^{\text {b }}$ |
| Picker Rd./ <br> Technology <br> Park Rd. | $\mathrm{S}^{\text {c }}$ | 1 general purpose lane on all approaches | No | No | No |

${ }^{\mathrm{a}} \mathrm{TS}=$ traffic signal control; $\mathrm{S}=$ STOP-sign control; $\mathrm{NC}=$ no control present.
${ }^{\mathrm{b}}$ Combined shoulder and travel lane width equal to or exceed 14 feet.
${ }^{\mathrm{c}} \mathrm{A}$ stop sign is not currently provided on Technology Park Road.

## Existing Traffic Volumes

In order to determine existing traffic-volume demands and flow patterns within the study area, manual turning movement counts (TMCs) and vehicle classification counts were obtained for the Route 20/Picker Road/Fiske Hill Road intersection from a prior study conducted by VAI. ${ }^{2}$ The TMCs were performed during the weekday morning (6:00 to 9:00 AM) and evening (3:00 to6:00 PM) peak periods on Thursday, May 14, 2020. These time periods were selected for analysis purposes as they are representative of the peak traffic volume hours for both the Project and the adjacent roadway network. Traffic volumes for the Picker Road/Technology Park Road intersection were developed using the TMCs on Picker Road at Route 20 and then assigning the traffic volumes on a proportionate basis to Picker Road and Technology Park Road based on the nature of the abutting land use.

## Traffic-Volume Adjustments

In order to evaluate the potential for fluctuation of traffic volumes within the study area due to the "stay-at-home order" that was issued by the Governor on March 24, 2020 due to the COVID-19 pandemic, recent traffic volume data collected in September 2019 at the adjacent Route 20/Route 49 intersection was used and compared to the May 2020 traffic volume data. Based on a review of traffic count data from MassDOT Continuous Count Station No. 3929 located on I-84 south of Route 20 in Sturbridge ${ }^{3}$ traffic volumes for the month of September are approximately 3.0 percent above average-month conditions. The September 2019 was adjusted to 2020 conditions by applying a general background traffic growth rate of 1.0 percent (discussed in further detail in the General Background Traffic Growth section of this assessment).

A comparison of the projected September 2020 peak-hour traffic volumes along Route 20 at the Route 20/Picker Road/Fiske Hill Road intersection to the corresponding data collected in May 2020 indicates that the May 2020 traffic volumes are approximately 92 percent lower during the weekday

[^1]

Figure 2
morning peak-hour and 40 percent lower during the weekday evening peak-hour. As such, the May 2020 peak-hour traffic volume data was adjusted upward accordingly in order to account for the reduced traffic volumes resulting from the "stay-at-home order".

The 2020 Existing weekday morning and evening peak-hour traffic volumes are graphically depicted on Figure 3.

## Pedestrian and Bicycle Facilities

A comprehensive field inventory of pedestrian and bicycle facilities within the study area was undertaken in July 2020. The field inventory consisted of a review of the location of sidewalks and pedestrian crossing locations along the study roadways and at the study intersections, as well as the location of existing and planned future bicycle facilities. As detailed on Figure 2, sidewalks or other pedestrian accommodations are not currently provided within the study area. Formal bicycle facilities were not identified within the study area; however, both Route 20 and Picker Road generally provide sufficient width (paved shoulder or combined travel lane and paved shoulder) to support bicycle travel in a shared traveled-way configuration. ${ }^{4}$

## Public Transportation

The Town of Sturbridge is a member of the Worcester Regional Transit Authority (WRTA); however, regularly scheduled, fixed-route public transportation services are not currently provided within the Town.

## Motor Vehicle Crash Data

Motor vehicle crash information for the study area intersections was provided by the MassDOT Highway Division Safety Management/Traffic Operations Unit for the most recent five-year period available (2013 through 2017, inclusive) in order to examine motor vehicle crash trends occurring within the study area. The data is summarized by intersection, type, severity, roadway and weather conditions, and day of occurrence, and presented in Table 2.

As can be seen in Table 2, 23 motor vehicle crashes were reported to have occurred at the Route 20/ Fiske Hill Road/Picker Road intersection over the five-year review period, or an average of approximately 4.6 crashes per year, the majority of which occurred on a weekday, during daylight, under clear weather conditions and involved angle-type collisions that resulted in property damage only. The calculated motor vehicle crash rate was found to be below both the MassDOT statewide and District averages for an unsignalized intersection for the MassDOT Highway Division District in which the intersections is located (District 3). No (0) motor vehicle crashes were reported to have occurred at the Picker Road/ Technology Park Road intersection over the five year review period

A review of the MassDOT statewide High Crash Location List indicated that there are no locations within the study area that are included on MassDOT's Highway Safety Improvement Program (HSIP) listing as high crash locations. In addition, no motor vehicle crashes that resulted in a fatality were identified to have occurred at the study area intersection over the five-year review period.

Based on a review of the MassDOT motor vehicle crash data, no discernible safety deficiencies were apparent within the study area. The detailed MassDOT Crash Rate Worksheet and HSIP mapping are provided in the Appendix.

[^2]

WEEKDAY EVENING PEAK HOUR (3:30-4:30 PM)


* Illegal movement.

Table 2
MOTOR VEHICLE CRASH DATA SUMMARY ${ }^{\text {a }}$

|  | Route 20/Picker Road/ Fiske Hill Road | Picker Road/ <br> Technology Park Road |
| :---: | :---: | :---: |
| Traffic Control Type ${ }^{\text {b }}$ | U | U |
| Year: |  |  |
| 2013 | 5 | 0 |
| 2014 | 1 | 0 |
| 2015 | 4 | 0 |
| 2016 | 7 | 0 |
| 2017 | 6 | $\underline{0}$ |
| Total | 23 | 0 |
| Average | 4.6 | 0.0 |
| Rate ${ }^{\text {c }}$ | 0.55 | 0.0 |
| MassDOT Crash Rate ${ }^{\text {d }}$ | 0.57/0.61 | 0.57/0.61 |
| Significant ${ }^{\text {e }}$ | No | No |
| Type: |  |  |
| Angle | 11 | 0 |
| Rear-End | 6 | 0 |
| Head-On | 3 | 0 |
| Sideswipe | 2 | 0 |
| Fixed Object | 1 | 0 |
| Other | 0 | 0 |
| Total | 23 | 0 |
| Conditions: |  |  |
| Clear | 15 | 0 |
| Cloudy | 4 | 0 |
| Snow/Ice | 2 | 0 |
| rain | 2 | 0 |
| Unknown | 0 | 0 |
| Total | $\overline{23}$ | 0 |
| Lighting: |  |  |
| Daylight | 19 | 0 |
| Dawn/Dusk | 1 | 0 |
| Dark (lit) | 3 | 0 |
| Dark (unlit) | 0 | 0 |
| Unknown | 0 | $\underline{0}$ |
| Total | 23 | 0 |
| Day of Week: |  |  |
| Monday through Friday | 19 | 0 |
| Saturday | 2 | 0 |
| Sunday | 2 | $\underline{0}$ |
| Total | 23 | 0 |
| Severity: |  |  |
| Property Damage Only | 13 | 0 |
| Personal Injury | 10 | 0 |
| Fatality | 0 | 0 |
| Unknown | 0 | $\underline{0}$ |
| Total | 23 | 0 |

[^3]
## FUTURE CONDITIONS

Traffic volumes in the study area were projected to the year 2027, which reflects a seven-year planning horizon consistent with MassDOT's Transportation Impact Assessment (TIA) Guidelines. Independent of the Project, traffic volumes on the roadway network in the year 2027 under No-Build conditions include all existing traffic and new traffic resulting from background traffic growth. Anticipated Project-generated traffic volumes superimposed upon the 2027 No-Build traffic volumes reflect 2027 Build traffic volume conditions with the Project.

## Future Traffic Growth

Future traffic growth is a function of the expected land development in the immediate area and the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed estimates an annual percentage increase in traffic growth and applies that percentage to all traffic volumes under study. The drawback to such a procedure is that some turning volumes may actually grow at either a higher or a lower rate at particular intersections.

An alternative procedure identifies the location and type of planned development, estimates the traffic to be generated, and assigns it to the area roadway network. This procedure produces a more realistic estimate of growth for local traffic; however, potential population growth and development external to the study area would not be accounted for in the resulting traffic projections.

To provide a conservative analysis framework, both procedures were used, the salient components of which are described below.

## Specific Development by Others

The Planning Departments of the Towns of Sturbridge and Charlton were contacted in order to determine if there were any projects planned within the study area that would have an impact on future traffic volumes at the study intersection. Based on these discussions, the following projects were identified for inclusion in this assessment:
>Cold Storage Warehouse, 6 Picker Road, Sturbridge, Massachusetts. This Project will entail the construction of a $120,662 \pm$ square foot (sf) cold storage warehouse building that will be located at 6 Picker Road in Sturbridge, Massachusetts.
> Propane Facility, 51 Technology Park Road, Sturbridge, Massachusetts. This project entails the construction of a $10,000 \pm$ sf service building with a 30,000 gallon underground propane storage tank to be located at 51 technology Park Road.
> Marijuana Dispensary, 660 Main Street, Sturbridge, Massachusetts. This project will entail the construction of a 3,292 $\pm$ sf marijuana dispensary to be located at 660 Main Street.
> Marijuana Dispensary, 365 Main Street, Sturbridge, Massachusetts. This project will entail the construction of a $2,592 \pm$ sf marijuana dispensary and $2,400 \pm$ sf of retail space to be located at 365 Main Street.
> Tree House Brewing Expansion, 129 Sturbridge Road Charlton, Massachusetts. This project consists of the expansion of the existing Tree House Brewing brewery located at 129 Sturbridge Road from $43,000 \pm$ sf to $67,718 \pm$ sf, of which $43,000 \pm$ sf will continue as the brewery operation, $20,710 \pm$ sf will be devoted to retail sales and $4,008 \pm \mathrm{sf}$ will consist of a timber frame pavilion to provide additional outdoor space for customers. In addition, a $7,600 \pm \mathrm{sf}$ outdoor "beer garden" area will also be constructed between the main building and the pavilion. This project is not expected to result in an increase in traffic during the weekday commuter peak hours that would exceed the general background traffic growth rate.
> Marijuana Dispensary, 144 Sturbridge Road, Charlton, Massachusetts. This project will entail the construction of a $20,000 \pm$ sf marijuana dispensary to be located at 144 Sturbridge Road.
> Auto Storage and Towing Facility, 299 Sturbridge Road, Charlton, Massachusetts. This project is currently under construction at 299 Sturbridge Road and is not expected to result in an increase in traffic during the weekday commuter peak hours that would exceed the general background traffic growth rate.
> Warehouse Development, Sturbridge Road, Charlton, Massachusetts. This proposed project entails the construction of a $1,400,000 \pm$ sf warehouse to be located off Sturbridge Road east of the Project site.

Traffic volumes associated with the aforementioned specific development projects by others were obtained from the traffic study prepared in support of the project or developed by using trip-generation information available from the Institute of Transportation Engineers (ITE) ${ }^{5}$ for the appropriate land use, and were assigned onto the study area roadway network based on existing traffic patterns where no other information was available. No other developments were identified at this time that are expected to result in an increase in traffic within the study area beyond the general background traffic growth rate.

## General Background Traffic Growth

Traffic-volume data compiled by MassDOT from Continuous Count Station No. 3929 located on I-84 south of Route 20 in Sturbridge were reviewed in order to determine general traffic growth trends in the area. Based on a review of this data, it was determined that traffic volumes within the study area have increased by approximately 0.86 percent per year over the past several years. In order to provide a conservative (high) analysis scenario and a prudent planning condition for the Project, a slightly higher 1.0 percent per year compounded annual background traffic growth rate was used in order to account for future traffic growth and presently unforeseen development within the study area.

## Roadway Improvement Projects

MassDOT and the Town of Sturbridge were consulted in order to determine if there were any planned future roadway improvement projects expected to be complete by 2027 within the study area. Based on this consultation, the following roadway improvement project was identified:
> Route 20 Roadway Improvements (Project No. 608435) - MassDOT is in the process of advancing design plans for the reconstruction of Route 20 between Route 49 and I-84 in Sturbridge that will include roadway reconstruction, widening, traffic signal installation/replacement/upgrades and the potential installation of a median barrier. This project is in the preliminary design stage and is not included in the future condition analysis.

[^4]No other roadway improvement projects aside from routine maintenance activities were identified to be planned within the study area at this time.

## No-Build Traffic Volumes

The 2027 No-Build condition peak-hour traffic-volumes were developed by applying the 1.0 percent per year compounded annual background traffic growth rate to the 2020 Existing peak-hour traffic volumes and then adding the additional traffic associated with the identified specific development projects by others. The resulting 2027 No-Build weekday morning and evening peak-hour traffic volumes are shown on Figure 4.

## PROJECT-GENERATED TRAFFIC

As proposed, the Project will entail the addition of a propane storage facility to the existing Pioneer Oil \& Propane facility. As described previously, Pioneer Oil \& Propane currently delivers oil and propane from the Project site; however, the propane is obtained from an off-site vendor. With the completion of the Project, the propane delivery truck will be able to load fuel at the Project site before making deliveries. The only new traffic associated with the Project will be the addition of one (1) to two (2) bulk propane deliveries per week to fill the storage tanks. For the purpose of this analysis, it was assumed that one (1) additional trip would be generated during the weekday morning and evening peak hours, which is conservative (high).

## Trip Distribution and Assignment

The directional distribution of generated trips to and from the Project site was determined based on the review of existing traffic patterns within the study area during the peak periods. The general trip distribution for the Project is graphically depicted on Figure 5, with the additional traffic that is expected to be generated by the Project assigned on the study area roadway network as shown on Figure 6.

## Build Traffic Volumes

The 2027 Build condition traffic volumes were developed by adding the traffic expected to be generated by the Project to the 2027 No-Build condition traffic volumes. The 2027 Build weekday morning and evening peak-hour traffic-volumes are graphically depicted on Figure 7.

## TRAFFIC OPERATIONS ANALYSIS

In order to assess the potential impact of the Project on the roadway network, a detailed traffic operations and vehicle queue analysis ("LOS analysis") was performed at the study area intersections. Capacity analyses provide an indication of how well transportation facilities serve the traffic demands placed upon them, with vehicle queue analyses providing a secondary measure of the operational characteristics of an intersection or section of roadway under study.

In brief, six levels of service are defined for each type of facility. They are given letter designations ranging from A to F, with level-of-service (LOS) "A" representing the best operating conditions and LOS " F " representing congested or constrained operations. An LOS of " E " is representative of a transportation facility that is operating at its design capacity with an LOS of "D" generally defined as the limit of "acceptable" traffic operations. Since the level-of-service of a traffic facility is a function of the flows placed upon it, such a facility may operate at a wide range of levels of service depending on the time of day, day of week, or period of the year. The Synchro® intersection capacity analysis software, which is


WEEKDAY EVENING PEAK HOUR (3:30-4:30 PM)


* Illegal movement.


## Legend:

XX Entering Trips
(XX) Exiting Trips



WEEKDAY EVENING PEAK HOUR (3:30-4:30 PM)


* Illegal movement.


WEEKDAY EVENING PEAK HOUR (3:30-4:30 PM)


* Illegal movement.
based on the analysis methodologies and procedures presented in the 2010 Highway Capacity Manual $(\mathrm{HCM})^{6}$ for signalized and unsignalized intersections, was used to complete the level-of-service and vehicle queue analyses.


## Analysis Results

Level-of-service and vehicle queue analyses were conducted for 2020 Existing, 2027 No-Build and 2027 Build conditions for the intersections within the study area. The results of the intersection capacity and vehicle queue analyses are summarized on Table 4, with the detailed analysis results presented in the Appendix. Project-related impacts at the study area intersections were identified as follows:

Route 20/Fiske Hill Road/Picker Road - No change in LOS or vehicle queuing is predicted to occur over No-Build conditions. Independent of the Project it was noted that through/right movements from Fiske Hill Road during the weekday morning peak-hour and all movements from Picker Road during both the weekday morning and evening peak hours are currently or are predicted to operate at or over capacity (defined as LOS "E" or "F", respectively) with residual vehicle queues of up to two (2) vehicles on Fiske Hill Road and up to 16 vehicles on Picker Road.

Picker Road/Technology Park Road - All movements were shown to operate at LOS A during both the weekday morning and evening peak hours with negligible vehicle queuing predicted. That being said, motorist delays on the Technology Park Road approach are directly related to operating conditions on the Picker Road approach to Route 20. As such, during the weekday evening peak-hour it is likely that motorist delays are higher than predicted by the analysis model given that vehicle queues on Picker Road approaching Route 20 are predicted to extend to or beyond Technology Park Road.

[^5]Table 4
UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

${ }^{\text {a }}$ Demand in vehicles per hour.
${ }^{\mathrm{b}}$ Average control delay per vehicle (in seconds).
${ }^{\text {c }}$ Level-of-Service.
${ }^{\text {d}}$ Queue length in vehicles.
$\mathrm{NB}=$ northbound; $\mathrm{SB}=$ southbound; $\mathrm{EB}=$ eastbound; $\mathrm{WB}=$ westbound; $\mathrm{LT}=$ left-turning movements; $\mathrm{TH}=$ through movements; $\mathrm{RT}=$ right-turning movements.

## SIGHT DISTANCE ASSESSMENT

Sight distance measurements were performed at the Technology Park Road intersection with Picker Road and at the Route 20/Fiske Hill Road/Picker Road intersection in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO) ${ }^{7}$ requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an on-coming vehicle and safely complete a turning or crossing maneuver with on-coming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 5 presents the measured SSD and ISD at the subject intersections.

Table 5
SIGHT DISTANCE MEASUREMENTS ${ }^{\text {a }}$

|  |  | Feet |
| :--- | :--- | :--- | :--- |
| Intersection/Sight Distance Measurement |  |  |

${ }^{\text {a }}$ Recommended minimum values obtained from A Policy on Geometric Design of Highways and Streets, 7 ${ }^{\text {th }}$ Edition; American Association of State Highway and Transportation Officials (AASHTO); 2018; and based on a 55 mph approach speed for Route 20 and 30 mph approach speed along Picker Road approaching Technology Park Road from the north and 15 mph approach speed approaching from the south.
${ }^{b}$ Values shown are the intersection sight distance for a vehicle turning right or left exiting a roadway under STOP control such that motorists approaching the intersection on the major street should not need to adjust their travel speed to less than 70 percent of their initial approach speed. The gap time for the intersection sight distance values along Route 20 was increased by 0.5 seconds in order to account for the increased time required to cross the additional travel lanes.
${ }^{c}$ With selective trimming/removal of vegetation along the east side of Picker Road north of Technology Park Road.
${ }^{\mathrm{d}}$ Clear line of sight is provided to/from Route 20.

[^6]As can be seen in Table 5, clear line of sight is provided to and from the Technology Park Road along Picker Road from Route 20 which indicates that motorists can perceive and react to potential conflicts that may be present. We note that although the statutory speed limit along Picker Road is 30 mph in the absence of a posted speed limit, the short distance of the roadway between Route 20 and Technology Park Road (approximately 135 feet) would limit actual travel speeds to approximately 15 mph on northbound direction. In addition, with the selective trimming/removal of trees and vegetation located along the east side of Picker Road and north of the Technology Park Road, the available lines of sight to and from Picker Road were found to exceed the recommended minimum distances for the intersection to function in a safe manner (SSD) based on a 30 mph approach speed on southbound direction.

Lines of sight along Route 20 and to and from Picker Road at its intersection with Route 20 were found to exceed the required minimum distances to function in a safe (SSD) and efficient (ISD) manner based on a 55 mph approach speed along Route 20 , which is 5 mph above the posted speed limit in the vicinity of the Project site ( 50 mph ).

## SUMMARY

VAI has prepared a detailed assessment of the potential impacts on the transportation infrastructure associated with the proposed addition of a propane storage facility to the existing Pioneer Oil \& Propane facility located at 59 Technology Park Road in Sturbridge, Massachusetts. The following specific areas have been evaluated as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; under existing and future conditions, both with and without the Project.

Based on this assessment, we have determined that the Project is not expected to result in a material increase in traffic over existing conditions given that propane delivery services are currently provided at the Project site. Any increase in traffic would be associated with the bulk delivery of propane to fill the storage tanks, which is expected to occur one (1) to two (2) times per week. As such, we have concluded that the Project will not result in a material increase in motorist delays or vehicle queueing over existing conditions and that the transportation infrastructure affords sufficient capacity to accommodate the Project in a safe and efficient manner. This conclusion is predicated on implementation of the following specific recommendations that should be advanced as a part of the Project:

1. A STOP-sign control and marked STOP-line should be provided on the Technology Park Road approach to Picker Road in order to reinforce the assignment of the vehicular right-of-way at the intersection and define the desired stopping point for vehicles.
2. All signs and pavement markings to be installed within the Project site shall conform to the applicable standards of the Manual on Uniform Traffic Control Devices (MUTCD). ${ }^{8}$
3. Signs and landscape features located within the site triangle areas of the Project site driveways shall be designed and maintained so as not to restrict lines of sight.
4. Snow windrows located within the sight triangle areas of the Project site driveways shall be promptly removed where such accumulations would inhibit sight lines.
5. Existing trees and vegetation located along the east side of Picker Road and north of the Technology Park Road should be selectively trimmed or removed and maintained so as to provide the necessary lines of sight to and from the north of the intersection.
6. Snow windrows along the Technology Park Road frontage within the sight triangle areas of the Project site roadway shall be promptly removed where such accumulations would inhibit sight lines.

With implementation of the above recommendations, safe and efficient access can be provided to the Project site and the Project can be accommodated within the confines of the existing transportation infrastructure.

[^7]

## APPENDIX

PROJECT SITE PLAN
MANUAL TURNING MOVEMENT COUNT DATA
SEASONAL ADJUSTMENT DATA
CRASH RATE WORKSHEETS AND HIGH CRASH LOCATION MAPPING GENERAL BACKGROUND TRAFFIC GROWTH
BACKGROUND DEVELOPMENT TRAFFIC VOLUME NETWORKS
CAPACITY ANALYSIS WORKSHEETS

PROJECT SITE PLAN


MANUAL TURNING MOVEMENT COUNT DATA

# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301am
Site Code $: 00863301$
Start Date : $5 / 14 / 2020$
Page No $: 1$

|  | Picker Road From North |  |  |  |  | Route 20 <br> From East |  |  |  |  | Gifford Road From South |  |  |  |  | Route 20 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Apo Toat | Right | Thru | Left | Peds | Ano Tolat | Right | Thru | Left | Peds | A00 Tolal | Right | Thru | Left | Peds | App Tolat | Int Tolal |
| 06:00 AM | 0 | 1 | 1 | 0 | 2 | 0 | 53 | 4 | 0 | 57 | 5 | 0 | 1 | 0 | 6 | 0 | 50 | 2 | 0 | 52 | 117 |
| 06:15 AM | 0 | 0 | 0 | 0 | 0 | 1 | 62 | 4 | 0 | 67 | 5 | 0 | 0 | 0 | 5 | 2 | 72 | 1 | 0 | 75 | 147 |
| 06:30 AM | 0 | 0 | 0 | 0 | 0 | 5 | 58 | 23 | 0 | 86 | 12 | 2 | 0 | 0 | 14 | 0 | 72 | 2 | 0 | 74 | 174 |
| 06:45 AM | 0 | 1 | 0 | 0 | 1 | 3 | 78 | 14 | 0 | 95 | 6 | 7 | 0 | 0 | 13 | 1 | 68 | 4 | 0 | 73 | 182 |
| Total | 0 | 2 | 1 | 0 | 3 | 9 | $25 \uparrow$ | 45 | 0 | 305 | 28 | 9 | 1 | 0 | 38 | 3 | 262 | 9 | 0 | 274 | 620 |
| 07:00 AM | 0 | 0 | 1 | 0 | 1 | 2 | 72 | 7 | 0 | 81 | 14 | 1 | 0 | 0 | 15 | 5 | 70 | 1 | 0 | 76 | 173 |
| 07:15 AM | 1 | 0 | 1 | 0 | 2 | 4 | 70 | 9 | 0 | 83 | 13 | 3 | 0 | 0 | 16 | 2 | 80 | 2 | 0 | 84 | 185 |
| 07:30 AM | 0 | 0 | 1 | 0 | 1 | 1 | 93 | 10 | 0 | 104 | 6 | 2 | 0 | 0 | 8 | 2 | 47 | 6 | 0 | 55 | 168 |
| 07:45 AM | 1 | 0 | 1 | 0 | 2 | 10 | 91 | 18 | 0 | 119 | 13 | 1 | 0 | 0 | 14 | 1 | 101 | 3 | 0 | 105 | 240 |
| Total | 2 | 0 | 4 | 0 | 6 | 17 | 326 | 44 | 0 | 387 | 46 | 7 | 0 | 0 | 53 | 10 | 298 | 12 | 0 | 320 | 766 |
| 08:00 AM | 2 | 1 | 1 | 0 | 4 | 4 | 74 | 17 | 0 | 95 | 9 | 1 | 0 | 0 | 10 | 0 | 75 | 4 | 0 | 79 | 188 |
| 08:15 AM | 1 | 0 | 4 | 0 | 5 | 3 | 91 | 12 | 0 | 106 | 7 | 3 | 0 | 0 | 10 | 2 | 78 | 1 | 0 | 81 | 202 |
| 08:30 AM | 1 | 0 | 0 | 0 | 1 | 3 | 90 | 11 | 0 | 104 | 13 | 2 | 0 | 0 | 15 | 1 | 82 | 3 | 0 | 86 | 206 |
| 08:45 AM | 1 | 0 | 0 | 0 | 1 | 5 | 77 | 15 | 0 | 97 | 10 | 0 | 0 | 0 | 10 | 1 | 76 | 2 | 0 | 79 | 187 |
| Total | 5 | 1 | 5 | 0 | 11 | 15 | 332 | 55 | 0 | 402 | 39 | 6 | 0 | 0 | 45 | 4 | 311 | 10 | 0 | 325 | 783 |
| Grand Total | 7 | 3 | 10 | 0 | 20 | 41 | 909 | 144 | 0 | 1094 | 113 | 22 | 1 | 0 | 136 | 17 | 871 | 31 | 0 | 919 | 2169 |
| Apprch \% | 35 | 15 | 50 | 0 |  | 3.7 | 83.1 | 13.2 | 0 |  | 83.1 | 16.2 | 0.7 | 0 |  | 1.8 | 94.8 | 3.4 | 0 |  |  |
| Total \% | 0.3 | 0.1 | 0.5 | 0 | 0.9 | 1.9 | 41.9 | 6.6 | 0 | 50.4 | 5.2 | 1 | 0 | 0 | 6.3 | 0.8 | 40.2 | 1.4 | 0 | 42.4 |  |
| Cars | 7 | 3 | 9 | 0 | 19 | 39 | 849 | 141 | 0 | 1029 | 110 | 21 | 1 | 0 | 132 | 16 | 796 | 30 | 0 | 842 | 2022 |
| \% Cars | 100 | 100 | 90 | 0 | 95 | 95.1 | 93.4 | 97.9 | 0 | 94.1 | 97.3 | 95.5 | 100 | 0 | 97.1 | 94.1 | 91.4 | 96.8 | 0 | 91.6 | 93.2 |
| Trucks | 0 | 0 | 1 | 0 | 1 | 2 | 60 | 3 | 0 | 65 | 3 | 1 | 0 | 0 | 4 | 1 | 75 | 1 | 0 | 77 | 147 |
| \% Trucks | 0 | 0 | 10 | 0 | 5 | 4.9 | 6.6 | 2.1 | 0 | 5.9 | 2.7 | 4.5 | 0 | 0 | 2.9 | 5.9 | 8.6 | 3.2 | 0 | 8.4 | 6.8 |

## Vanasse \& Associates

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301am
Site Code : 00863301
Start Date: 5/14/2020
Page No :2


# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301am
Site Code : 00863301
Start Date : 5/14/2020
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|  | Picker Road From North |  |  |  |  | Route 20 <br> From East |  |  |  |  | Gifford Road From South |  |  |  |  | Route 20 <br> From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Ado Tolal | Right | Thru | Left | Peds | ADD. Tola | Right | Thru | Left | Pads | ADg Tolal | Right | Thru | Left | Peds | Apg ${ }^{\text {t }}$ | Int. Tolal |

Peak Hour Analysis From 06:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Entire Intersection Begins at 07:45 AM

| 07:45 AM | 1 | 0 | 1 | 0 | 2 | 10 | 91 | 18 | 0 | 119 | 13 | 1 | 0 | 0 | 14 | 1 | 101 | 3 | 0 | 105 | 240 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 08:00 AM | 2 | 1 | 1 | 0 | 4 | 4 | 74 | 17 | 0 | 95 | 9 | 1 | 0 | 0 | 10 | 0 | 75 | 4 | 0 | 79 | 188 |
| 08:15 AM | 1 | 0 | 4 | 0 | 5 | 3 | 91 | 12 | 0 | 106 | 7 | 3 | 0 | 0 | 10 | 2 | 78 | 1 | 0 | 81 | 202 |
| 08:30 AM | 1 | 0 | 0 | 0 | 1 | 3 | 90 | 11 | 0 | 104 | 13 | 2 | 0 | 0 | 15 | 1 | 82 | 3 | 0 | 86 | 206 |
| Total Volurne | 5 | 1 | 6 | 0 | 12 | 20 | 346 | 58 | 0 | 424 | 42 | 7 | 0 | 0 | 49 | 4 | 336 | 11 | 0 | 351 | 836 |
| \% App. Total | 41.7 | 8.3 | 50 | 0 |  | 4.7 | 81.6 | 13.7 | 0 |  | 85.7 | 14.3 | 0 | 0 |  | 1.1 | 95.7 | 3.1 | 0 |  |  |
| PHF | . 625 | . 250 | . 375 | . 000 | . 600 | . 500 | . 951 | . 806 | . 000 | . 891 | . 808 | . 583 | . 000 | . 000 | . 817 | . 500 | . 832 | . 688 | . 000 | . 836 | . 871 |
| Cars | 5 | 1 | 5 | 0 | 11 | 19 | 324 | 57 | 0 | 400 | 41 | 6 | 0 | 0 | 47 | 4 | 302 | 11 | 0 | 317 | 775 |
| \% Cars | 100 | 100 | 83.3 | 0 | 91.7 | 95.0 | 93.6 | 98.3 | 0 | 94.3 | 97.6 | 85.7 | 0 | 0 | 95.9 | 100 | 89.9 | 100 | 0 | 90.3 | 92.7 |
| Trucks | 0 | 0 | 1 | 0 | 1 | 1 | 22 | 1 | 0 | 24 | 1 | 1 | 0 | 0 | 2 | 0 | 34 | 0 | 0 | 34 | 61 |
| \% Trucks | 0 | 0 | 16.7 | 0 | 8.3 | 5.0 | 6.4 | 1.7 | 0 | 5.7 | 2.4 | 14.3 | 0 | 0 | 4.4 | 0 | 10.1 | 0 | 0 | 9.7 | 7.3 |

## Vanasse \& Associates

Route 20 at Picker Rd/Gifford Rd Sturbridge, MA
Weather: Clear

File Name : 863301am
Site Code : 00863301
Start Date : 5/14/2020
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## Vanasse \& Associates

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301am
Site Code : 00863301
Start Date : 5/14/2020
Page No : 1

| Groups Printed-Cars |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Plcker Road From North |  |  |  |  | Route 20 <br> From East |  |  |  |  | Gifford Road From South |  |  |  |  | Route 20 From West |  |  |  |  |  |
| Start Time | Right | Thru | Left | Peds | Asp Yotal | Right | Thru | Left | Peds | ADO Tolal | Right | Thru | Left | Peds | Add Tolal | Right | Thru | Left | Peds | App Tolal | Int Total |
| 06:00 AM | 0 | 1 | 1 | 0 | 2 | 0 | 49 | 4 | 0 | 53 | 5 | 0 | 1 | 0 | 6 | 0 | 47 | 2 | 0 | 49 | 110 |
| 06:15 AM | 0 | 0 | 0 | 0 | 0 | 1 | 58 | 4 | 0 | 63 | 5 | 0 | 0 | 0 | 5 | 1 | 65 | 1 | 0 | 67 | 135 |
| 06:30 AM | 0 | 0 | 0 | 0 | 0 | 5 | 56 | 23 | 0 | 84 | 12 | 2 | 0 | 0 | 14 | 0 | 60 | 2 | 0 | 62 | 160 |
| 06:45 AM | 0 | 1 | 0 | 0 | 1 | 2 | 75 | 14 | 0 | 91 | 6 | 7 | 0 | 0 | 13 | 1 | 63 | 4 | 0 | 68 | 173 |
| Total | 0 | 2 | 1 | 0 | 3 | 8 | 238 | 45 | 0 | 291 | 28 | 9 | 1 | 0 | 38 | 2 | 235 | 9 | 0 | 246 | 578 |
| 07:00 AM | 0 | 0 | 1 | 0 | 1 | 2 | 67 | 7 | 0 | 76 | 13 | 1 | 0 | 0 | 14 | 5 | 66 | 1 | 0 | 72 | 163 |
| 07:15 AM | 1 | 0 | 1 | 0 | 2 | 4 | 67 | 8 | 0 | 79 | 13 | 3 | 0 | 0 | 16 | 2 | 74 | 2 | 0 | 78 | 175 |
| 07:30 AM | 0 | 0 | 1 | 0 | 1 | 1 | 85 | 10 | 0 | 96 | 6 | 2 | 0 | 0 | 8 | 2 | 47 | 5 | 0 | 54 | 159 |
| 07:45 AM | 1 | 0 | 0 | 0 | 1 | 10 | 86 | 18 | 0 | 114 | 13 | 1 | 0 | 0 | 14 | 1 | 91 | 3 | 0 | 95 | 224 |
| Total | 2 | 0 | 3 | 0 | 5 | 17 | 305 | 43 | 0 | 365 | 45 | 7 | 0 | 0 | 52 | 10 | 278 | 11 | 0 | 299 | 721 |
| 08:00 AM | 2 | 1 | 1 | 0 | 4 | 4 | 71 | 17 | 0 | 92 | 8 | 1 | 0 | 0 | 9 | 0 | 68 | 4 | 0 | 72 | 177 |
| 08:15 AM | 1 | 0 | 4 | 0 | 5 | 3 | 84 | 12 | 0 | 99 | 7 | 2 | 0 | 0 | 9 | 2 | 71 | 1 | 0 | 74 | 187 |
| 08:30 AM | 1 | 0 | 0 | 0 | 1 | 2 | 83 | 10 | 0 | 95 | 13 | 2 | 0 | 0 | 15 | 1 | 72 | 3 | 0 | 76 | 187 |
| 08:45 AM | 1 | 0 | 0 | 0 | 1 | 5 | 68 | 14 | 0 | 87 | 9 | 0 | 0 | 0 | 9 | 1 | 72 | 2 | 0 | 75 | 172 |
| Total | 5 | 1 | 5 | 0 | 11 | 14 | 306 | 53 | 0 | 373 | 37 | 5 | 0 | 0 | 42 | 4 | 283 | 10 | 0 | 297 | 723 |
| Grand Total | 7 | 3 | 9 | 0 | 19 | 39 | 849 | 141 | 0 | 1029 | 110 | 21 | 1 | 0 | 132 | 16 | 796 | 30 | 0 | 842 | 2022 |
| Apprch \% | 36.8 | 15.8 | 47.4 | 0 |  | 3.8 | 82.5 | 13.7 | 0 |  | 83.3 | 15.9 | 0.8 | 0 |  | 1.9 | 94.5 | 3.6 | 0 |  |  |
| Total \% | 0.3 | 0.1 | 0.4 | 0 | 0.9 | 1.9 | 42 | 7 | 0 | 50.9 | 5.4 | 1 | 0 | 0 | 6.5 | 0.8 | 39.4 | 1.5 | 0 | 41.6 |  |

# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301am
Site Code : 00863301
Start Date : 5/14/2020
Page No :1

|  | Picker Road From North |  |  |  |  | Route 20 <br> From East |  |  |  |  | Gifford Road From South |  |  |  |  | Route 20 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Lett | Peds | App Tolal | Right ! | Thru | Left | Peds | Ado. Tolal | Right | Thru | Left | Peds | Apa. Tolal | Right | Thru | Left | Peds | Apd Tolal | Int. Total |
| 06:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 7 |
| 06:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 1 | 7 | 0 | 0 | 8 | 12 |
| 06:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 | 12 | 14 |
| 06:45 AM | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 5 | 9 |
| Total | 0 | 0 | 0 | 0 | 0 | 1 | 13 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 1 | 27 | 0 | 0 | 28 | 42 |
| 07:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 5 | 1 | 0 | 0 | 0 | 1 | 0 | 4 | 0 | 0 | 4 | 10 |
| 07:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 1 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 6 | 10 |
| 07:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 9 |
| 07:45 AM | 0 | 0 | 1 | 0 | 1 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 10 | 16 |
| Total | 0 | 0 | 1 | 0 | 1 | 0 | 21 | 1 | 0 | 22 | 1 | 0 | 0 | 0 | 1 | 0 | 20 | 1 | 0 | 21 | 45 |
| 08:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 1 | 0 | 0 | 0 | 1 | 0 | 7 | 0 | 0 | 7 | 11 |
| 08:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 7 | 0 | 1 | 0 | 0 | 1 | 0 | 7 | 0 | 0 | 7 | 15 |
| 08:30 AM | 0 | 0 | 0 | 0 | 0 | 1 | 7 | 1 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 10 | 19 |
| 08:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 1 | 0 | 10 | 1 | 0 | 0 | 0 | 1 | 0 | 4 | 0 | 0 | 4 | 15 |
| Total | 0 | 0 | 0 | 0 | 0 | 1 | 26 | 2 | 0 | 29 | 2 | 1 | 0 | 0 | 3 | 0 | 28 | 0 | 0 | 28 | 60 |
| Grand Total | 0 | 0 | 1 | 0 | 1 | 2 | 60 | 3 | 0 | 65 | 3 | 1 | 0 | 0 | 4 | 1 | 75 | 1 | 0 | 77 | 147 |
| Apprch \% | 0 | 0 | 100 | 0 |  | 3.1 | 92.3 | 4.6 | 0 |  | 75 | 25 | 0 | 0 |  | 1.3 | 97.4 | 1.3 | 0 |  |  |
| Total \% | 0 | 0 | 0.7 | 0 | 0.7 | 1.4 | 40.8 | 2 | 0 | 44.2 | 2 | 0.7 | 0 | 0 | 2.7 | 0.7 | 51 | 0.7 | 0 | 52.4 |  |

# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Wreather: Clear

> File Name $: 863301 \mathrm{pm}$ Site Code $: 00863301$
> Start Date $: 5 / 14 / 2020$
> Page No $: 1$

Groups Printed- Cars - Trucks - Buses

|  | Picker Road From North |  |  |  |  | Route 20 From East |  |  |  |  | Gifford Road From South |  |  |  |  | Route 20 From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App tolal | Right | Thru | Left | Peds | Abs Tasi | Right | Thru | Left | Peds | ADO Tolat | Right | Thru | Left | Peds | Avo. Toat | Int Total |
| 03:00 PM | 1 | 0 | 0 | 0 | 1 | 2 | 145 | 22 | 0 | 169 | 14 | 0 | 0 | 0 | 14 | 4 | 160 | 1 | 0 | 165 | 349 |
| 03:15 PM | 1 | 0 | 2 | 0 | 3 | 3 | 153 | 24 | 0 | 180 | 14 | 0 | 0 | 0 | 14 | 3 | 137 | 1 | 0 | 141 | 338 |
| 03:30 PM | 8 | 9 | 8 | 0 | 25 | 1 | 160 | 29 | 0 | 190 | 18 | 2 | 2 | 0 | 22 | 2 | 134 | 0 | 0 | 136 | 373 |
| 03:45 PM | 5 | 1 | 4 | 0 | 10 | 1 | 195 | 15 | 0 | 211 | 20 | 0 | 0 | 0 | 20 | 3 | 144 | 4 | 0 | 151 | 392 |
| Total | 15 | 10 | 14 | 0 | 39 | 7 | 653 | 90 | 0 | 750 | 66 | 2 | 2 | 0 | 70 | 12 | 575 | 6 | 0 | 593 | 1452 |


| 04:00 PM | 2 | 2 | 4 | 0 | 8 | 1 | 154 | 21 | 0 | 176 | 26 | 1 | 0 | 0 | 27 | 4 | 134 | 3 | 0 | 141 | 352 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 04:15 PM | 5 | 0 | 2 | 0 | 7 | 1 | 148 | 19 | 0 | 168 | 23 | 1 | 0 | 0 | 24 | 4 | 157 | 0 | 0 | 161 | 360 |
| 04:30 PM | 5 | 1 | 6 | 0 | 12 | 1 | 153 | 19 | 0 | 173 | 24 | 1 | 0 | 0 | 25 | 5 | 144 | 0 | 0 | 149 | 359 |
| 04:45 PM | 4 | 0 | 1 | 0 | 5 | 0 | 145 | 22 | 0 | 167 | 19 | 0 | 0 | 0 | 19 | 5 | 133 | 1 | 0 | 139 | 330 |
| Total | 16 | 3 | 13 | 0 | 32 | 3 | 600 | 81 | 0 | 684 | 92 | 3 | 0 | 0 | 95 | 18 | 568 | 4 | 0 | 590 | 1401 |


| 05:00 PM | 2 | 0 | 6 | 0 | 8 | 1 | 180 | 27 | 0 | 208 | 17 | 0 | 1 | 0 | 18 | 8 | 143 | 0 | 0 | 151 | 385 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 152 | 20 | 0 | 172 | 26 | 0 | 0 | 0 | 26 | 2 | 149 | 0 | 0 | 151 | 349 |
| 05:30 PM | 1 | 0 | 0 | 0 | 1 | 1 | 167 | 24 | 0 | 192 | 15 | 0 | 1 | 0 | 16 | 4 | 134 | 1 | 0 | 139 | 348 |
| 05:45 PM | 1 | 0 | 0 | 0 | 1 | 0 | 151 | 19 | 0 | 170 | 16 | 0 | 0 | 0 | 16 | 3 | 127 | 0 | 0 | 130 | 317 |
| Total | 4 | 0 | 6 | 0 | 10 | 2 | 650 | 90 | 0 | 742 | 74 | 0 | 2 | 0 | 76 | 17 | 553 | 1 | 0 | 571 | 1399 |


| Grand Total | 35 | 13 | 33 | 0 | 81 | 12 | 1903 | 261 | 0 | 2176 | 232 | 5 | 4 | 0 | 241 | 47 | 1696 | 11 | 0 | 1754 | 4252 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Apprch \% | 43.2 | 16 | 40.7 | 0 |  | 0.6 | 87.5 | 12 | 0 |  | 96.3 | 2.1 | 1.7 | 0 |  | 2.7 | 96.7 | 0.6 | 0 |  |  |
| Total \% | 0.8 | 0.3 | 0.8 | 0 | 1.9 | 0.3 | 44.8 | 6.1 | 0 | 51.2 | 5.5 | 0.1 | 0.1 | 0 | 5.7 | 1.1 | 39.9 | 0.3 | 0 | 41.3 |  |
| Cars | 34 | 13 | 30 | 0 | 77 | 12 | 1845 | 261 | 0 | 2118 | 232 | 5 | 4 | 0 | 241 | 46 | 1656 | 10 | 0 | 1712 | 4148 |
| \% Cars | 97.1 | 100 | 90.9 | 0 | 95.1 | 100 | 97 | 100 | 0 | 97.3 | 100 | 100 | 100 | 0 | 100 | 97.9 | 97.6 | 90.9 | 0 | 97.6 | 97.6 |
| Trucks | 1 | 0 | 3 | 0 | 4 | 0 | 58 | 0 | 0 | 58 | 0 | 0 | 0 | 0 | 0 | 1 | 39 | 1 | 0 | 41 | 903 |
| \% Trucks | 2.9 | 0 | 9.1 | 0 | 4.9 | 0 | 3 | 0 | 0 | 2.7 | 0 | 0 | 0 | 0 | 0 | 2.1 | 2.3 | 9.1 | 0 | 2.3 | 2.4 |
| Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 1 |
| \% Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0.1 | 0 | 0 | 0.1 | 0 |

## Vanasse \& Associates

Route 20 at Picker Rd/Gifford Rd Sturbridge, MA
Weather: Clear

File Name : 863301pm
Site Code : 00863301
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# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301pm
Site Code : 00863301
Start Date : 5/14/2020
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|  | Picker Road From North |  |  |  |  | Route 20 <br> From East |  |  |  |  | Gifford Road <br> From South |  |  |  |  | Route 20 <br> From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Pads | ADP Tolal | Right | Thru | Left | Peds | Apo Tomat | Right | Thru | Left | Peds | App Tolal | Right | Thru | Left | Peds | ApD Tolal | Inl. Tolal |

Peak Hour Analysis From 03:00 PM to 05:45 PM - Peak 1 of 1


# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301pm
Site Code : 00863301
Start Date : 5/14/2020
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# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301pm
Site Code : 00863301
Start Date : 5/14/2020
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| Groups Printed-Cars |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | int Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picker Road From North |  |  |  |  | Route 20 From East |  |  |  |  | Gifford Road From South |  |  |  |  | Route 20 From West |  |  |  |  |  |
| Start Time | Right | Thru | Left | Peds | App Tolal | Right | Thru | [ Left | Peds | ADp Tolal | Right | Thru! | Left \| | Peds | App. Tolal | Right | Thru | Left | Peds | Apo Tolat |  |
| 03:00 PM | 1 | 0 | 0 | 0 | 1 | 2 | 141 | 22 | 0 | 165 | 14 | 0 | 0 | 0 | 14. | 4 | 155 | 1 | 0 | 160 | 340 |
| 03:15 PM | 1 | 0 | 2 | 0 | 3 | 3 | 139 | 24 | 0 | 166 | 14 | 0 | 0 | 0 | 14 | 3 | 129 | 1 | 0 | 133 | 316 |
| 03:30 PM | 8 | 9 | 8 | 0 | 25 | 1 | 153 | 29 | 0 | 183 | 18 | 2 | 2 | 0 | 22 | 2 | 130 | 0 | 0 | 132 | 362 |
| 03:45 PM | 4 | 1 | 4 | 0 | 9 | 1 | 191 | 15 | 0 | 207 | 20 | 0 | 0 | 0 | 20 | 3 | 141 | 4 | 0 | 148 | 384 |
| Total | 14 | 10 | 14 | 0 | 38 | 7 | 624 | 90 | 0 | 721 | 66 | 2 | 2 | 0 | 70 | 12 | 555 | 6 | 0 | 573 | 1402 |
| 04:00 PM | 2 | 2 | 4 | 0 | 8 | 1 | 151 | 21 | 0 | 173 | 26 | 1 | 0 | 0 | 27 | 4 | 130 | 3 | 0 | 137 | 345 |
| 04:15 PM | 5 | 0 | 2 | 0 | 7 | 1 | 143 | 19 | 0 | 163 | 23 | 1 | 0 | 0 | 24 | 4 | 156 | 0 | 0 | 160 | 354 |
| 04:30 PM | 5 | 1 | 6 | 0 | 12 | 1 | 147 | 19 | 0 | 167 | 24 | 1 | 0 | 0 | 25 | 5 | 140 | 0 | 0 | 145 | 349 |
| 04:45 PM | 4 | 0 | 1 | 0 | 5 | 0 | 141 | 22 | 0 | 163 | 19 | 0 | 0 | 0 | 19 | 4 | 130 | 0 | 0 | 134 | 321 |
| Total | 16 | 3 | 13 | 0 | 32 | 3 | 582 | 81 | 0 | 666 | 92 | 3 | 0 | 0 | 95 | 17 | 556 | 3 | 0 | 576 | 1369 |
| 05:00 PM | 2 | 0 | 3 | 0 | 5 | 1 | 175 | 27 | 0 | 203 | 17 | 0 | 1 | 0 | 18 | 8 | 140 | 0 | 0 | 148 | 374 |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 149 | 20 | 0 | 169 | 26 | 0 | 0 | 0 | 26 | 2 | 147 | 0 | 0 | 149 | 344 |
| 05:30 PM | 1 | 0 | 0 | 0 | 1 | 1 | 164 | 24 | 0 | 189 | 15 | 0 | 1 | 0 | 16 | 4 | 131 | 1 | 0 | 136 | 342 |
| 05:45 PM | 1 | 0 | 0 | 0 | 1 | 0 | 151 | 19 | 0 | 170 | 16 | 0 | 0 | 0 | 16 | 3 | 127 | 0 | 0 | 130 | 317 |
| Total | 4 | 0 | 3 | 0 | 7 | 2 | 639 | 90 | 0 | 731 | 74 | 0 | 2 | 0 | 76 | 17 | 545 | 1 | 0 | 563 | 1377 |
| Grand Total | 34 | 13 | 30 | 0 | 77 | 12 | 1845 | 261 | 0 | 2118 | 232 | 5 | 4 | 0 | 241 | 46 | 1656 | 10 | 0 | 1712 | 4148 |
| Apprch \% | 44.2 | 16.9 | 39 | 0 |  | 0.6 | 87.1 | 12.3 | 0 |  | 96.3 | 2.1 | 1.7 | 0 |  | 2.7 | 96.7 | 0.6 | 0 |  |  |
| Total \% | 0.8 | 0.3 | 0.7 | 0 | 1.9 | 0.3 | 44.5 | 6.3 | 0 | 51.1 | 5.6 | 0.1 | 0.1 | 0 | 5.8 | 1.1 | 39.9 | 0.2 | 0 | 41.3 |  |

# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

| Start Time | Groups Printed- Trucks |  |  |  |  |  |  |  |  |  |  |  |  |  |  | File Name : 863301pm <br> Site Code : 00863301 <br> Start Date : 5/14/2020 <br> Page No : 1 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picker Road From North |  |  |  |  | Route 20 <br> From East |  |  |  |  | Gifford Road From South |  |  |  |  | Route 20 From West |  |  |  |  |  |
|  | Right | Thru | Left | Peds | Apo Toat | Right | Thru | Left | Peds | Apo Tolal | Right I | Thrul | Left | Peds | Aob Tolal | Right | Thru | Left | Peds | Ase Tolal | Int Total |
| 03:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 5 | 9 |
| 03:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 14 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 7 | 21 |
| 03:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 11 |
| 03:45 PM | 1 | 0 | 0 | 0 | 1 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 8 |
| Total | 1 | 0 | 0 | 0 | 1 | 0 | 29 | 0 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 0 | 0 | 19 | 49 |
| 04:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 7 |
| 04:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 6 |
| 04:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 10 |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 1 | 0 | 5 | 9 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 18 | 0 | 0 | 18 | 0 | 0 | 0 | 0 | 0 | 1 | 12 | 1 | 0 | 14 | 32 |
| 05:00 PM | 0 | 0 | 3 | 0 | 3 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 11 |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 2 | 5 |
| 05:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 3 | 6 |
| 05:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total | 0 | 0 | 3 | 0 | 3 | 0 | 11 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 8 | 22 |
| Grand Total | 1 | 0 | 3 | 0 | 4 | 0 | 58 | 0 | 0 | 58 | 0 | 0 | 0 | 0 | 0 | 1 | 39 | 1 | 0 | 41 | 103 |
| Apprch \% | 25 | 0 | 75 | 0 |  | 0 | 100 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 2.4 | 95.1 | 2.4 | 0 |  |  |
| Total \% | 1 | 0 | 2.9 | 0 | 3.9 | 0 | 56.3 | 0 | 0 | 56.3 | 0 | 0 | 0 | 0 | 0 | 1 | 37.9 | 1 | 0 | 39.8 |  |

# Vanasse \& Associates 

Route 20 at Picker Rd / Gifford Rd
Sturbridge, MA
Weather: Clear

File Name : 863301pm Site Code : 00863301
Start Date : 5/14/2020
Page No : 1

| Groups Printed- Buses |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Picker Road From North |  |  |  |  | Route 20 <br> From East |  |  |  |  | Gifford Road From South |  |  |  |  | Route 20 <br> From West |  |  |  |  |  |
| Start Time | Right | Thru | Left ! | Peds | App Tolal | Right | Thru $\}$ | Left | Peds | ADD Tolal | Right | Thru | Left | Peds | ADD Tolal | Right | Thru | Left | Peds | Agp Toial | Int. Tolal |
| 03:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 03:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 1 |
| 03:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 03:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 1 |
| 04:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 04:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 04:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 04:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 05:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 05:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 05:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 05:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 1 |
| Apprch \% | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 100 | 0 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 100 | 0 | 0 | 100 |  |






| Route 49 <br> From North |  |  |  | Route 20 <br> From East |  |  | Route 20 <br> From West |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Right | App. Total | Thru | Right | App. Total | Left | Thru | App. Total |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 07:30 AM |  |  |  |  |  |  |  |  |  |
| 07:30 AM | 9 | 106 | 145 | 88 | 4 | 92 | 55 | 115 | 170 |
| 07:45 AM | 5 | 122 | 127 | 116 | 9 | 125 | 59 | 126 | 185 |
| 08:00 AM | 7 | 88 | 95 | 101 | 5 | 106 | 74 | 124 | 198 |
| 08:15 AM | 6 | 101 | 107 | 104 | 7 | 111 | 53 | 103 | 156 |
| Total Volume | 27 | 417 | 444 | 409 | 25 | 434 | 241 | 468 | 709 |
| \% App. Total | 6.1 | 93.9 |  | 94.2 | 5.8 |  | 34 | 66 |  |
| PHF | . 750 | . 855 | . 874 | . 881 | . 694 | ${ }_{8} 88$ | . 814 | . 929 | . 895 |
| Cars | 21 | 393 | 414 | 384 | 24 | 408 | 219 | 444 | 663 |
| \% Cars | 77.8 | 94.2 | 93.2 | 93.9 | 96.0 | 94.0 | 90.9 | 94.9 | 93.5 |
| Trucks | 6 | 24 | 30 | 25 | 1 | 26 | 22 | 24 | 46 |
| \% Trucks | 22.2 | 5.8 | 6.8 | 6.1 | 4.0 | 6.0 | 9.1 | 5.1 | 6.5 |



|  |  | te 49 |  |  | te 20 |  |  | te 20 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | North |  |  | East |  |  | West |  |
| Start Time | Left | Right | App. Total | Thru | Right | App. Total | Left | Thru | App. Total |
| Peak Hour Analysis From 07:00 | 5 AM - |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection | 07:30 |  |  |  |  |  |  |  |  |
| 07:30 AM | 9 | 106 | 115 | 88 | 4 | 92 | 55 | 115 | 170 |
| 07:45 AM | 5 | 122 | 127 | 116 | 9 | 125 | 59 | 126 | 185 |
| 08:00 AM | 7 | 88 | 95 | 101 | 5 | 106 | 74 | 124 | 198 |
| 08:15 AM | 6 | 101 | 107 | 104 | 7 | 111 | 53 | 103 | 156 |
| Total Volume | 27 | 417 | 444. | 409 | 25 | 434 | 241 | 468 | 709 |
| \% App. Total | 6.1 | 93.9 |  | 94.2 | 5.8 |  | 34 | 66 |  |
| PHF | . 750 | 855 | . 874 | . 881 | . 694 | . 868 | . 814 | . 929 | . 895 |
| Cars | 21 | 393 | $4 \% 4$ | 384 | 24 | 408 | 219 | 444 | 663 |
| \% Cars | 77.8 | 94.2 | 93.2 | 93.9 | 96.0 | 94.0 | 90.9 | 94.9 | 93.5 |
| Trucks | 6 | 24 | 30 | 25 | 1 | 26 | 22 | 24 | 46 |
| \% Trucks | 22.2 | 5.8 | 6.8 | 6.1 | 4.0 | 6.0 | 9.1 | 5.1 | 6.5 |




[^8]| ENW Street: Route 20 <br> City/State : Charlton, MA <br> Weather : Clear |  |  |
| :---: | :---: | :---: |
|  |  | Route 49 From North |
| Start Time | Left | Right |
| Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 |  |  |
| Peak Hour for Entire Intersection Begins at 07:30 AM |  |  |
| 07:30 AM ! | 9 | 106 |
| 07:45 AM | 5 | 122 |
| 08:00 AM | 7 | 88 |
| 08:15 AM | 6 | 101 |
| Total volume | 27 | 417 |
| \% App. Total | 6.1 | 93.9 |
| PHF | . 750 | . 855 |
| Cars | 21 | 393 |
| \% Cars | 77.8 | 94.2 |
| Trucks | 6 | 24 |
| \% Trucks | 22.2 | 5.8 |

File Name : 83840002
Site Code $: 83840002$
Start Date $: 9 / 25 / 2019$
Page No : 3



File Name: 83840002
Site Code : 83840002
Start Date : $9 / 25 / 2019$
Page No : 5



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Peak Hour Analysis From 03:00 PM to 05:45 PM - Peak 1 of 1
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Peak Hour for Each Approach Begins at:
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+30 mins.
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Massachusetts Highway Department
3929：Monthly Hourly Volume for September 2015

| $\begin{aligned} & \stackrel{\rightharpoonup}{\text { a }} \\ & \stackrel{y}{5} \\ & \stackrel{y}{c} \end{aligned}$ | $\begin{aligned} & \text { Nin } \\ & \stackrel{\circ}{\text { in }} \end{aligned}$ | $\underset{\substack{N\\}}{N}$ | $\begin{gathered} \infty \\ \sim \\ \end{gathered}$ | $\begin{aligned} & \infty \\ & \stackrel{\infty}{0} \\ & \stackrel{\infty}{0} \end{aligned}$ | $$ | $\stackrel{\text { M }}{\stackrel{\text { N }}{N}}$ | $\begin{aligned} & \text { Ñ } \\ & \underset{\sim}{n} \end{aligned}$ | $\begin{aligned} & \text { in } \\ & \text { in } \end{aligned}$ | $\begin{aligned} & \text { ry } \\ & 0 \\ & 0 \end{aligned}$ | $\begin{aligned} & \underset{\sim}{\tilde{\circ}} \\ & \underset{心}{6} \end{aligned}$ | $\underset{\sim}{N}$ | $\begin{aligned} & \underset{\sim}{7} \\ & \text { in } \end{aligned}$ | $\begin{aligned} & \stackrel{i}{0} \\ & \underset{\sim}{m} \end{aligned}$ | $\begin{aligned} & \vec{y} \\ & \dot{寸} \\ & \dot{寸} \end{aligned}$ | $\stackrel{\stackrel{\circ}{\circ}}{\substack{\circ \\ \hline}}$ | $\begin{aligned} & \text { ® } \\ & \text { ion } \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 号 } \\ & \stackrel{0}{0} \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{0} \\ & \underset{\omega}{\oplus} \end{aligned}$ | $\begin{aligned} & \text { ơ } \\ & \stackrel{+}{\circ} \\ & \text { 心 } \end{aligned}$ |  |  | $\begin{aligned} & \text { 员 } \\ & \text { 唝 } \end{aligned}$ | $\begin{aligned} & \stackrel{\infty}{\stackrel{1}{0}} \\ & \stackrel{\rightharpoonup}{0} \end{aligned}$ |  | 哃 | $\begin{aligned} & \infty \\ & \stackrel{\infty}{0} \\ & \underset{\sim}{2} \end{aligned}$ | $\begin{aligned} & \text { on } \\ & \text { N} \\ & \text { m } \end{aligned}$ | $\begin{aligned} & N \\ & \stackrel{N}{\star} \\ & \stackrel{N}{N} \end{aligned}$ |  | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
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| $\stackrel{\stackrel{\sim}{N}}{\substack{\underset{\sim}{\sim} \\ \hline}}$ | $\stackrel{\stackrel{\sim}{7}}{ }$ | $\stackrel{i \pi}{\aleph}$ | $\stackrel{8}{\sim}$ | $\begin{array}{r} \text { to } \\ \stackrel{0}{0} \end{array}$ | $\stackrel{g}{\mathrm{~N}}$ | $\begin{aligned} & \stackrel{\circ}{0} \\ & \underset{\sim}{n} \end{aligned}$ | $\stackrel{\text { n }}{\text { n }}$ | $\begin{aligned} & 00 \\ & \stackrel{0}{0} \\ & \hline \end{aligned}$ | $\stackrel{\infty}{\stackrel{\infty}{\underset{\sim}{1}}}$ | $\underset{\sim}{\underset{\sim}{\mathrm{g}}}$ | 荷 | $\begin{aligned} & \infty \\ & \stackrel{\sim}{\square} \end{aligned}$ | $\underset{\overrightarrow{7}}{\mathrm{t}}$ | 管 | $\begin{aligned} & \underset{\sim}{\mathbf{m}} \end{aligned}$ | $\stackrel{\text { ®a }}{\stackrel{\sim}{\square}}$ | $\stackrel{o}{G}$ | $\underset{\underset{\sim}{N}}{\underset{\sim}{N}}$ |  | \％ | $\stackrel{\circ}{\underset{\sim}{0}}$ | $$ | $\stackrel{\bullet 0}{\underset{\sim}{7}}$ | $\stackrel{\stackrel{n}{0}}{\stackrel{-}{1}}$ | m | － | － |  |  | $\begin{aligned} & \text { "I } \\ & \stackrel{0}{0} \\ & \text { dex } \end{aligned}$ |
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|  | $\stackrel{0}{7}$ | 僉 | $\begin{aligned} & \infty \\ & \stackrel{\infty}{寸} \end{aligned}$ | す | $\begin{aligned} & \stackrel{\circ}{0} \\ & \underset{e}{2} \end{aligned}$ | $\stackrel{0}{\circ}$ | $\stackrel{\sim}{\text { N }}$ | 品 | $\begin{aligned} & \stackrel{\circ}{\mathrm{m}} \\ & \hline \end{aligned}$ | 而 | $\begin{aligned} & \underset{\sim}{\sim} \end{aligned}$ | $\begin{aligned} & \text { 윽 } \\ & \underset{\sim}{n} \end{aligned}$ | $\stackrel{0}{\oplus}$ | $$ | $\stackrel{\infty}{\stackrel{\infty}{\sim}}$ | 骨 | $\stackrel{\text { g. }}{\underset{\sim}{0}}$ | 哃 | oio | $\underset{\sim}{\hat{N}}$ | $\stackrel{\underset{\sim}{-}}{\stackrel{1}{2}}$ | $\stackrel{\text { 而 }}{ }$ | $\stackrel{\infty}{\infty}$ | 腐 | $$ | 禁 | 翑 |  |  |  |
| $\begin{aligned} & \circ \stackrel{\circ}{\circ} \\ & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \hline \end{aligned}$ | $\underset{\sim}{\infty}$ | o! | 苛 | 俞 | $\stackrel{\sim}{i n}$ | $\stackrel{\rightharpoonup}{0}$ | $\begin{aligned} & \text { 品 } \end{aligned}$ | $\stackrel{m}{\underset{\sim}{n}}$ | $\stackrel{\underset{\sim}{\sim}}{\sim}$ | $\stackrel{\substack{\sim \\ \sim}}{\circ}$ | $\underset{\substack{\text { 品 } \\ \hline}}{ }$ | 合 | $\stackrel{\rightharpoonup}{\mathrm{N}}$ | $\begin{aligned} & \text { 㖒 } \\ & \text { n } \end{aligned}$ | $\underset{\sim}{\sim}$ | $\begin{aligned} & \stackrel{\infty}{0} \\ & \underset{\sim}{0} \end{aligned}$ | $\underset{\sim}{\underset{\sim}{m}}$ | $\overrightarrow{\overrightarrow{7}}$ | $\begin{aligned} & \stackrel{0}{\mathbf{0}} \\ & \underset{\sim}{2} \end{aligned}$ | 㯰 | $\begin{aligned} & \text { 品 } \\ & \hline \end{aligned}$ | $\stackrel{\rightharpoonup}{\underset{\sim}{w}}$ | $\begin{aligned} & \text { 䛔 } \\ & \text { n } \end{aligned}$ | $\underset{\sim}{\sim}$ | $\stackrel{\text { 尔 }}{ }$ | $\stackrel{\text { N }}{\text { N }}$ | 器 |  |  |  |
| 扁 品 | $\underset{\sim}{\underset{N}{n}}$ | $\begin{aligned} & \stackrel{-}{\sim} \\ & \text { m } \end{aligned}$ | 弟 | 菏 | $\begin{aligned} & \infty \infty \infty \\ & \underset{e n}{\infty} \end{aligned}$ | $\begin{aligned} & \text { M M } \\ & \substack{0 \\ \hline} \end{aligned}$ | 哭 | $\stackrel{m}{\underset{m}{m}}$ | $\stackrel{\circ}{9}$ | 畣 | $\begin{aligned} & \text { 号 } \\ & \text { 筫 } \end{aligned}$ | $\underset{\sim}{\text { J }}$ | $\stackrel{\cong \sim}{\substack{\sim}}$ | $\underset{\sim}{\underset{\sim}{2}}$ | $\begin{aligned} & \text { O. } \\ & \text { 品 } \end{aligned}$ | 淮 | $\underset{\sim}{\underset{\sim}{\infty}}$ | 呙 | $\stackrel{\text { ón }}{\stackrel{\sim}{n}}$ | $\stackrel{\text { m}}{\underset{\sim}{2}}$ | N్N్N | $\stackrel{\overrightarrow{\mathbf{o}}}{\substack{0 \\ \hline}}$ | $\stackrel{N}{\mathrm{G}}$ | $\underset{\sim}{\underset{\sim}{\sim}}$ | $\underset{\sim}{\text { IN }}$ | 岗 | － |  |  |  |
|  | $\stackrel{\circ 0}{\stackrel{\circ}{\sim}}$ | 第 | 侖 | $\stackrel{\circ}{\stackrel{\circ}{m}} \underset{\sim}{m}$ | $\begin{aligned} & \text { 岩 } \end{aligned}$ | $\begin{aligned} & \text { m } \\ & \underset{\sim}{m} \end{aligned}$ |  | $\stackrel{\text { No }}{\substack{0}}$ | $\underset{\sim}{\text { 守 }}$ | 僉 | $\begin{aligned} & \text { ion } \\ & \text { in } \end{aligned}$ | $\begin{aligned} & \stackrel{\circ}{0} \\ & \hline 0 \end{aligned}$ | $\stackrel{\substack{\infty \\ \sim}}{2}$ | 欲 | $\stackrel{m}{\infty}$ | $\underset{\underset{J}{7}}{\substack{n}}$ | $\underset{\substack{\stackrel{m}{6} \\ \hline}}{ }$ | 号 | $\underset{\sim}{\underset{\sim}{n}}$ | $\stackrel{-r}{\sim}$ | ~ow | $\stackrel{m}{m}$ | $\begin{gathered} \tilde{m} \\ \tilde{\sim} \end{gathered}$ | $\stackrel{\text { n}}{\infty}$ | $\underset{\sim}{\ddot{\infty}}$ | $\stackrel{N}{N}$ | $\stackrel{\AA}{N}$ |  |  |  |
| $\stackrel{\stackrel{\circ}{\ddot{H}}}{\text { m }} \underset{\sim}{m}$ | $\stackrel{\sim}{\sim}$ | $\begin{aligned} & \overrightarrow{\mathrm{p}} \\ & \stackrel{\mathrm{~m}}{\mathrm{~m}} \end{aligned}$ | $\stackrel{\text { Ü }}{\underset{\sim}{*}}$ | 蔌 | $\underset{\sim}{\widetilde{\sim}}$ | $\begin{aligned} & 0 \\ & \stackrel{\circ}{n} \end{aligned}$ | $\stackrel{9}{-}$ | $\underset{\sim}{\underset{\sim}{7}}$ | $\begin{aligned} & \infty \\ & \stackrel{\infty}{寸} \end{aligned}$ | $\stackrel{\otimes}{\vec{\sigma}}$ |  | $\begin{aligned} & \text { on } \\ & \text { nem } \end{aligned}$ | 尺্ঠ̈ | $\begin{aligned} & \infty \\ & \stackrel{\infty}{e} \end{aligned}$ | $\underset{\sim}{\text { d }}$ | $\begin{aligned} & \infty \\ & \stackrel{\infty}{\circ} \end{aligned}$ | $\stackrel{m}{\stackrel{m}{n}}$ | 泡 | $\begin{aligned} & \text { ig } \\ & \text { 翤 } \end{aligned}$ | $\begin{aligned} & \text { N } \\ & \text { O} \end{aligned}$ | $\underset{\sim}{\vec{J}}$ | $\begin{gathered} \\ \stackrel{0}{m} \end{gathered}$ | $\begin{aligned} & \stackrel{m}{\sim} \end{aligned}$ | $\stackrel{\text { o }}{\text { on }}$ | $\underset{\sim}{\tilde{m}}$ | $\stackrel{\circ}{\infty}$ | 品 |  |  |  |
|  | $\begin{gathered} \text { 子 } \\ \text { Non } \end{gathered}$ | 蕾 | $\underset{\substack{\mathrm{m} \\ \hline}}{ }$ | 落 | $\stackrel{N}{\tilde{y}}$ | $\begin{aligned} & \text { a } \\ & \text { 岱 } \end{aligned}$ | M | $\begin{gathered} \hat{m} \\ \text { mon } \end{gathered}$ | $\vec{F}$ | $\begin{aligned} & \text { or } \\ & \text { 等 } \end{aligned}$ | 志 | $\begin{aligned} & \stackrel{0}{6} \\ & \stackrel{0}{0} \end{aligned}$ | $\underset{\substack{\text { In } \\ \hline}}{ }$ | ざ | $\underset{\sim}{\stackrel{N}{5}}$ | $\stackrel{0}{2}$ |  | $\begin{aligned} & \text { g. } \\ & \text { in } \end{aligned}$ | $\underset{\sim}{\text { tu }}$ | $\stackrel{n}{i n}$ | $\begin{gathered} \text { Non } \\ \underset{\sim}{\mathbf{N}} \end{gathered}$ | Eice | $\begin{aligned} & \text { ® } \\ & \text { in } \end{aligned}$ | $\stackrel{\infty}{\underset{\sim}{g}}$ | $\underset{\sim}{\underset{\sim}{7}}$ | $\begin{aligned} & \text { 品 } \\ & \hline \end{aligned}$ | $\stackrel{9}{\text { m }}$ |  |  |  |
| $\begin{aligned} & \text { 肙品 } \\ & \text { 品 } \end{aligned}$ | $\begin{aligned} & \text { en } \\ & \text { m } \end{aligned}$ | $\begin{aligned} & \infty \\ & \stackrel{\infty}{\infty} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { No } \\ & \text { Ơn } \end{aligned}$ | $\underset{\substack{\text { un } \\ \underset{\sim}{0}}}{ }$ | $\begin{aligned} & \underset{\sim}{\tilde{N}} \end{aligned}$ | 先 | $\begin{aligned} & \text { ơo } \\ & \underset{\sim}{0} \end{aligned}$ | $\underset{\sim}{\text { ¢ }}$ | $\stackrel{\stackrel{\Gamma}{\infty}}{\stackrel{\circ}{f}}$ | $\underset{\substack{i n \\ \sim}}{ }$ | 僉 | $\begin{aligned} & \infty \\ & \stackrel{0}{0} \\ & \stackrel{0}{0} \end{aligned}$ | $\stackrel{\sim}{\sim}$ | $\underset{\sim}{\sim}$ | 总 | 荌 | $\overrightarrow{7}$ | $\stackrel{\sim}{\sim}$ | $\underset{\sim}{\text { ry }}$ |  | op | $\stackrel{\text { O}}{\substack{0}}$ | 侖 | $\begin{gathered} \text { n } \\ \text { 䫆 } \end{gathered}$ | $\underset{\sim}{\text { ®/ }}$ | $\stackrel{\sim}{\sim}$ | ～ |  |  |  |
|  | $\begin{aligned} & \text { প্m } \\ & \text { inem } \end{aligned}$ | $\begin{aligned} & \text { eo } \\ & \stackrel{\infty}{m} \end{aligned}$ | $\underset{\tilde{W}}{\tilde{W}}$ | $\underset{\sim}{\text { N }}$ | $\begin{aligned} & \overrightarrow{0} \\ & \text { B } \end{aligned}$ | N | $\stackrel{\infty}{\infty}$ | $\begin{aligned} & \text { 若 } \\ & \text { n } \end{aligned}$ | $\begin{aligned} & \infty \\ & \stackrel{\infty}{m} \\ & \stackrel{m}{0} \end{aligned}$ | $\begin{aligned} & \text { تِ } \\ & \text { O } \end{aligned}$ | $\begin{aligned} & \text { ow } \\ & \underset{\sim}{0} \end{aligned}$ | 咢 | $\underset{\sim}{\text { J }}$ | $\stackrel{\text { g }}{\substack{\text { on }}}$ | $\begin{aligned} & \stackrel{9}{0} \\ & \stackrel{\sim}{0} \end{aligned}$ | $\stackrel{J}{\stackrel{J}{g}}$ | $\begin{aligned} & \text { 品 } \\ & \text { 品 } \end{aligned}$ | $\underset{\substack{\text { © } \\ \text { N }}}{ }$ | $\underset{\sim}{\underset{\sim}{N}}$ | $\stackrel{\stackrel{\rightharpoonup}{\mathbf{w}}}{ }$ | 蔏 | $\stackrel{\infty}{\stackrel{\infty}{m}}$ | $\begin{aligned} & \text { 品 } \end{aligned}$ | $\begin{gathered} \underset{\sim}{\underset{\sim}{*}} \end{gathered}$ | $\stackrel{g}{-9}$ | $\stackrel{\sim}{\sim}$ | $\stackrel{\text { g }}{\text { d }}$ |  |  |  |
| 荅荷 | $\stackrel{\text { じ }}{\substack{0}}$ | $\begin{aligned} & \text { of } \\ & \stackrel{0}{0} \end{aligned}$ | $\begin{aligned} & \underset{a}{a} \\ & 0 \end{aligned}$ | 点 | $\begin{gathered} \check{\infty} \\ \underset{\sim}{\infty} \end{gathered}$ | $\stackrel{\infty}{\text { ® }}$ | $\stackrel{\underset{m}{7}}{\stackrel{\rightharpoonup}{n}}$ | $\underset{\sim}{\text { 翤 }}$ | $\begin{aligned} & \vec{o} \\ & \text { 夺 } \end{aligned}$ | N్స్ | 荅 | $\begin{aligned} & \text { 盆 } \\ & \text { 品 } \end{aligned}$ | $\overrightarrow{\underset{\sim}{0}}$ | $\stackrel{\infty}{\stackrel{\infty}{N}}$ | $\overrightarrow{\stackrel{\rightharpoonup}{e}}$ | $\stackrel{m}{\underset{\sim}{f}}$ | $\begin{aligned} & \text { 品 } \\ & \tilde{3} \end{aligned}$ | 答 | 弟 | 莒 | $\underset{\sim}{\text { 尔 }}$ | $\underset{\sim}{\sim}$ | $\begin{aligned} & \text { 号 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 鮞 } \end{aligned}$ | $\stackrel{\stackrel{y}{*}}{\substack{\text { n }}}$ | 尔 | 盗 |  |  |  |
|  | $\stackrel{\text { èm }}{\text { an }}$ | $\begin{aligned} & \stackrel{\circ}{\%} \\ & \text { m } \end{aligned}$ |  | $\begin{aligned} & \text { o } \\ & \text { 品 } \end{aligned}$ | 洺 | $\begin{aligned} & \text { し0 } \\ & \text { n } \end{aligned}$ | $\stackrel{9}{\infty}$ | $\begin{aligned} & \text { ®o } \\ & \text { Non } \end{aligned}$ | $\stackrel{\substack{\mathrm{z}}}{ }$ |  | 莒 | $\stackrel{\rightharpoonup}{\stackrel{\rightharpoonup}{m}}$ | ơ̆ | $\stackrel{m}{\substack{0 \\ \sim}}$ | $\stackrel{\mathbb{E}}{\stackrel{\sim}{\sim}}$ | $\begin{aligned} & \stackrel{\text { O}}{\text { U }} \end{aligned}$ | $\begin{aligned} & \text { M } \\ & \underset{\circ}{\circ} \end{aligned}$ | $\begin{gathered} \text { 苟 } \\ \text { g } \end{gathered}$ | $\underset{\substack{0 \\ \underset{\sim}{0} \\ \hline}}{ }$ | $\stackrel{\text { © }}{\substack{0}}$ | $\stackrel{\text { un }}{\underset{\sim}{\infty}}$ | $\underset{\sim}{\mathbf{0}}$ | স্ড | $\stackrel{\rightharpoonup}{n}$ | $\begin{aligned} & \text { ざ } \\ & \text { OG } \end{aligned}$ | － | 志 |  |  |  |
|  | $\begin{aligned} & \text { © } \\ & \text { Op } \end{aligned}$ | $\begin{aligned} & \stackrel{9}{\circ} \\ & \end{aligned}$ | ন্ত | $\underset{\sim}{\tilde{N}}$ |  | $\stackrel{\text { og }}{\text { g }}$ | $\begin{gathered} 9 \\ \underset{\sim}{\sim} \end{gathered}$ | $\underset{\sim}{\hat{N}}$ | 售 | $\begin{aligned} & \mathscr{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\stackrel{\Im}{7}$ | $\begin{aligned} & \infty \\ & \underset{m}{m} \\ & \hline \end{aligned}$ | $\underset{\sim}{\sim}$ | $\stackrel{\infty}{\underset{\sim}{\infty}}$ | 合 | $\begin{aligned} & \text { © } \\ & \stackrel{0}{0} \end{aligned}$ | N | $\begin{aligned} & \stackrel{\circ}{6} \\ & \stackrel{y}{6} \end{aligned}$ | $\underset{\sim}{\underset{\sim}{4}}$ | 菏 | $\begin{aligned} & \text { +্ } \\ & \text { N } \end{aligned}$ | $\underset{\sim}{\underset{\sim}{0}}$ | N | 等 | $\stackrel{m}{\underset{\sim}{\infty}}$ | 㞻 | － |  |  |  |
| 谷 珨 | $\stackrel{\text { M }}{\substack{\infty \\ \sim}}$ | $\stackrel{\underset{\sim}{\infty}}{\stackrel{\infty}{\sim}}$ | $\stackrel{\circ}{\text { O゙ }}$ | $\overrightarrow{7}$ | $\begin{aligned} & \text { op } \\ & \text { men } \end{aligned}$ | $\stackrel{(m)}{\stackrel{m}{m}}$ | $\stackrel{m}{\stackrel{m}{\mathrm{~m}}}$ | $\stackrel{\text { B}}{\stackrel{\rightharpoonup}{0}}$ | 莒 | $\begin{aligned} & \stackrel{\sim}{\sigma} \\ & \text { O} \end{aligned}$ | $\overrightarrow{\mathbf{0}}$ | $\underset{\sim}{\infty}$ | 品 | 荷 | $\stackrel{\rightharpoonup}{\sim}$ | $\stackrel{\substack{\infty \\ \infty \\ \hline \\ \hline}}{ }$ | $\stackrel{\text { n }}{\underset{\sim}{7}}$ | $\underset{\sim}{\text { g }}$ | $\begin{aligned} & \infty \\ & \text { ๗. } \\ & \text { © } \end{aligned}$ | $\underset{\sim}{J}$ | $\stackrel{\square}{\text { ¢ }}$ | 号 | $\begin{aligned} & \stackrel{\sim}{\circ} \\ & \stackrel{\sim}{\circ} \end{aligned}$ | $\stackrel{\infty}{\stackrel{\infty}{8}}$ | $\stackrel{\infty}{\stackrel{\infty}{0}}$ | $\begin{aligned} & 0 \\ & \stackrel{0}{\sim} \end{aligned}$ | － |  |  |  |
| 翤袋 | N | $\stackrel{\circ}{\circ}$ | 筞 | $\vec{m}$ | $\vec{~}$ | $\begin{aligned} & \stackrel{\circ}{7} \\ & \end{aligned}$ | 罝 | $\stackrel{\stackrel{\circ}{0}}{\stackrel{\circ}{\sim}}$ | $\stackrel{\widehat{\infty}}{\underset{N}{0}}$ | $\stackrel{\stackrel{\rightharpoonup}{\mathrm{p}}}{ }$ | $\begin{aligned} & \text { gion } \\ & \text { 號 } \end{aligned}$ | 苚 | $\stackrel{!}{\underset{\sim}{0}}$ | $\stackrel{\rightharpoonup}{N}$ | $\stackrel{\sim}{\underset{\sim}{\infty}}$ | ت゙ | $\underset{\sim}{\underset{\sim}{0}}$ | $\stackrel{\text { N }}{\sim}$ | $\begin{aligned} & \text { O} \\ & \text { N } \end{aligned}$ | 朵 | $\underset{\sim}{\underset{\sim}{d}}$ | $\underset{\sim}{\text { IN }}$ | $\underset{\underset{\sim}{R}}{ }$ | N | $\begin{gathered} \text { N } \\ \stackrel{\sim}{n} \end{gathered}$ | $\stackrel{\text { त }}{ }$ | 荷 |  |  |  |
| 우N | 渵 | $\stackrel{\text { no }}{\sim}$ | 号 | $\underset{\sim}{\mathscr{\infty}}$ | $\underset{\sim}{\text { üg }}$ | $\begin{aligned} & \stackrel{\sim}{0} \\ & \underset{\sim}{7} \end{aligned}$ | $\stackrel{\text { N }}{\sim}$ | $\underset{\sim}{\underset{\sim}{2}}$ | $\stackrel{\rightharpoonup}{N}$ | $\underset{\sim}{\sim}$ | $\underset{\sim}{\underset{\sim}{J}}$ | $\stackrel{\mathscr{O}}{\substack{0 \\ \hline}}$ | $\stackrel{\tilde{\tilde{y}}}{\underset{\sim}{x}}$ | $\stackrel{\overrightarrow{( }}{\underset{\sim}{c}}$ | $\underset{\sim}{\underset{\sim}{W}}$ | $\stackrel{\stackrel{g}{\tilde{\sim}}}{\substack{2}}$ | $\stackrel{\underset{\sim}{\sim}}{\sim}$ |  | $\underset{\sim}{\underset{\sim}{\hat{G}}}$ | $\underset{\sim}{\underset{\sim}{7}}$ | $\stackrel{\circ}{\underset{\sim}{n}}$ | 晜 | $\underset{\sim}{\underset{\sim}{n}}$ | $\underset{\sim}{\tilde{\sim}}$ | $\begin{aligned} & \text { 芯 } \end{aligned}$ | $\stackrel{\text { ¢ }}{\substack{\text { ¢ }}}$ | $\stackrel{\infty}{\text { ¢ }}$ |  |  |  |
| Oị 曾 | － | 号 | 哭 | － | 号 | 砍 | $\underset{\sim}{\sim}$ | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{-}{\square} \end{aligned}$ | $\stackrel{9}{\text { ¢ }}$ | $\stackrel{\substack{\text { n } \\ \sim}}{\sim}$ | $\stackrel{\sim}{\sim}$ | － | 亳 | $\begin{aligned} & \stackrel{\circ}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | 品 | $\begin{aligned} & \underset{\sim}{\underset{-}{0}} \end{aligned}$ | $\stackrel{0}{\underset{\sim}{\square}}$ | $\stackrel{0}{\sim}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{0} \\ & \underset{\sim}{n} \end{aligned}$ | 关 | $\stackrel{\sim}{\sim}$ | $\stackrel{\text { ! }}{\stackrel{\circ}{\sim}}$ | İ | $\stackrel{\text { ® }}{\text { \％}}$ | $\stackrel{\sim}{\sim}$ | N | ¢ิ |  |  |  |
| 䕎 N | 菏 |  | 合 | $\stackrel{0}{0}$ | － | 笑 | 营 | $\begin{aligned} & \text { or } \\ & \underset{\sim}{0} \end{aligned}$ | $\stackrel{\mathrm{m}}{7}$ | 品 | $\stackrel{\infty}{m}$ | 㫛 | 㖮 | $\stackrel{\text { à }}{\substack{\text { a }}}$ | 喜 | 욱 | 员 | $\stackrel{\%}{7}$ | 嵩 | － | 哭 | ¢ | － | $\stackrel{\text { ¢ }}{\substack{\text { ¢ }}}$ | － | $\stackrel{\sim}{\sim}$ | $\underset{\infty}{ }$ |  |  |  |
| Ö | ～ | \％ | $\stackrel{1}{\square}$ | $\stackrel{\sim}{\sim}$ | n | － | $\stackrel{\rightharpoonup}{\infty}$ | $\stackrel{\rightharpoonup}{\infty}$ | $\stackrel{\text { ® }}{ }$ | $\stackrel{9}{\sim}$ | ～${ }_{\text {n }}$ | $\stackrel{\infty}{\sim}$ | 迢 | 品 | $\stackrel{\infty}{8}$ | 岕 | －0 | 品 | $\stackrel{\square}{\infty}$ | 嗅 | 员 | $\stackrel{\square}{8}$ | ${ }^{-1}$ | $\stackrel{\text { m }}{\text { m }}$ | 呙 | $\stackrel{\bullet}{\circ}$ | 옷 |  |  |  |
| 융 응 | \％ | 啚 | $\stackrel{\text { \％}}{\substack{8 \\ 0}}$ | $\stackrel{m}{\text { m }}$ | $\stackrel{\text { ¢ }}{\sim}$ | in | ${ }_{\text {in }}^{\text {n }}$ | $\overbrace{0}^{9}$ | $\stackrel{9}{6}$ | N | N | $\stackrel{7}{4}$ | 㽞 | 河 | 7 | 嵒 | \％ | 只 | $\stackrel{\infty}{\sim}$ | $\stackrel{9}{6}$ | 7 | \％ | \％ | 岁 | ¢ | \％ | 운 |  |  |  |
| 운 | $\stackrel{\square}{\mathrm{m}}$ | $\stackrel{\text { ¢ }}{\substack{\text { \％}}}$ | 吕 | 岩 | $\stackrel{\infty}{m}$ | $\stackrel{\rightharpoonup}{\text { m }}$ | 品 | \％ | No | 品 | $\stackrel{\circ}{5}$ | 帯 | $\stackrel{\sim}{\sim}$ | N | $\stackrel{\text { m }}{\substack{\text { a }}}$ | 宫 | 尌 | \％ | 等 | 品 | 8 | i | in | $\stackrel{\infty}{*}$ | \％ | 守 | 尔 |  |  |  |
| $\stackrel{8}{i}$ | N | in | ㅁNN | $\stackrel{6}{6}$ | 䂞 | 吕 | \％ | 奀 | ${ }_{\infty}^{\infty}$ | \％ | 志 | $\stackrel{n}{\sim}$ | $\stackrel{\rightharpoonup}{i}$ | N | 寽 | 㟋 | N | $\stackrel{\rightharpoonup}{\infty}$ | $\stackrel{7}{\sim}$ | 号 | \％ | 等 | ～ | ${ }_{\text {\％}}^{\text {¢ }}$ | N | \％ | 号 |  |  |  |
| 융 品 | 웅 | ¢1 | $\stackrel{m}{\sim}$ | $\begin{aligned} & \infty \\ & \stackrel{\infty}{\infty} \end{aligned}$ | 䫆 | F | 용 | $\stackrel{\infty}{\sim}$ | 誌 | 高 | 号 | 怘 | 涌 | $\stackrel{\square}{\sim}$ | $\stackrel{m}{\square}$ | $\stackrel{m}{\sim}$ | 品 | $\overrightarrow{6}$ | N． | \％ | ㅅNN | N | $\vec{\circ}$ | $\stackrel{\text { in }}{\text { O }}$ | 号 | $\stackrel{\text { m }}{0}$ | \％ |  |  |  |
|  |  |  |  |  |  |  |  |  | $\overrightarrow{7}$ | $\sim$ | $\stackrel{m}{7}$ | $\pm$ | ๑ |  |  | $\underset{\sim}{\infty}$ | 9 | 구 |  | N |  |  |  |  |  |  |  |  |  |  |

## INTERSECTION CRASH RATE WORKSHEET


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3


| STA. | CITY/TOWN | ROUTE/STREET | LOCATION | 2010 | 2011 | 2012 | 2013 | 2014 | 2015 | 2016 | 2017 | 2018 | 2019 | Average Annual Growth Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3929 | Sturbridge | $1-84$ | South of Route 20 | 55.400 | 53.645 | 51,486 | 52,177 | 52.522 | 55.467 | 55.862 | 56,868 | 57.167 | 57.566 | 0.86\% |
| \% |  |  |  |  |  |  |  |  |  |  |  |  |  |  |



WEEKDAY EVENING PEAK HOUR (3:30-4:30 PM)



WEEKDAY EVENING PEAK HOUR (3:30-4:30 PM)


## CAPACITY ANALYSIS

Route 20/Fiske Hill Road and Picker Road
Picker Road/Technology Park Road

|  | 7 | $\rightarrow$ | 7 | 7 | $\downarrow$ | 4 | 4 | 4 | 7 |  | $\dagger$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $4_{4}$ |  | 1 | 个t |  |  | F |  |  | $\oplus$ |  |
| Traffic Volume (vph) | 11 | 668 | 4 | 58 | 756 | 20 | 0 | 7 | 42 | 6 | 1 | 5 |
| Future Volume (vph) | 11 | 668 | 4 | 58 | 756 | 20 | 0 | 7 | 42 | 6 | 1 | 5 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 16 | 12 | 12 | 12 | 12 |
| Storage Length (ft) | 0 |  | 0 | 240 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 0 |  | 0 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Utill. Factor | 0.95 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Fit |  | 0.999 |  |  | 0.996 |  |  | 0.885 |  |  | 0.946 |  |
| Flt Protected |  | 0.999 |  | 0.950 |  |  |  |  |  |  | 0.976 |  |
| Satd. Flow (prot) | 0 | 3282 | 0 | 1770 | 3393 | 0 | 0 | 1836 | 0 | 0 | 1617 | 0 |
| Flt Permitted |  | 0.999 |  | 0.950 |  |  |  |  |  |  | 0.976 |  |
| Satd. Flow (perm) | 0 | 3282 | 0 | 1770 | 3393 | 0 | 0 | 1836 | 0 | 0 | 1617 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 823 |  |  | 845 |  |  | 358 |  |  | 407 |  |
| Travel Time (s) |  | 18.7 |  |  | 19.2 |  |  | 8.1 |  |  | 9.3 |  |
| Adj. Flow (vph) | 13 | 795 | 5 | 65 | 849 | 22 | 0 | 9 | 51 | 10 | 2 | 8 |
| Lane Group Flow (vph) | 0 | 813 | 0 | 65 | 871 | 0 | 0 | 60 | 0 | 0 | 20 | 0 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary <br> ```Area Type: Other``` <br> Control Type: Unsignalized




Platoon blocked, \%

Mov Cap-2 Maneuver - . . . . . . . 60 - 6471

| Stage 1 | - | - | - | - | - | - | - | 347 | - | 230 | 301 | - |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| Stage 2 | - | - | - | - | - | - | - | 270 | - | 465 | 377 | - |


| Approach | EB | WB | NB | SB |
| :--- | :---: | :---: | :---: | :---: |
| HCM Control Delay, s | 0.3 | 0.7 | 22.7 | 48.2 |
| HCM LOS |  | $C$ | E |  |


| Minor Lane/Major Mvmt | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 263 | 783 | - | - | 819 | - | - |
| HCM Lane V/C Ratio | 0.227 | 0.017 | - | - | 0.08 | - | -0.194 |
| HCM Control Delay (s) | 22.7 | 9.7 | 0.1 | - | 9.8 | - | - |
| HCM Lane LOS | C | A | A | - | A | - | - |
| HCM 95th \%tile Q(veh) | 0.9 | 0.1 | - | - | 0.3 | - | - |
|  |  | 0.7 |  |  |  |  |  |


|  | $\dagger$ | $\rightarrow$ | 7 | 7 | $\downarrow$ |  | 4 | $\dagger$ | 7 | $t$ | 1 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 41 |  | \% | 中t |  |  | $\dagger$ |  |  | \$ |  |
| Traffic Volume (vph) | 7 | 832 | 13 | 84 | 965 | 4 | 0 | 4 | 87 | 18 | 12 | 20 |
| Future Volume (vph) | 7 | 832 | 13 | 84 | 965 | 4 | 0 | 4 | 87 | 18 | 12 | 20 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 16 | 12 | 12 | 12 | 12 |
| Storage Length (t) | 0 |  | 0 | 240 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 0 |  | 0 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (tt) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 823 |  |  | 845 |  |  | 358 |  |  | 327 |  |
| Travel Time (s) |  | 18.7 |  |  | 19.2 |  |  | 8.1 |  |  | 7.4 |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.88 | 0.88 | 0.88 | 0.86 | 0.86 | 0.86 | 0.50 | 0.50 | 0.50 |
| Heavy Vehicles (\%) | 0\% | 2\% | 0\% | 0\% | 3\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 5\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 926 | 0 | 95 | 1102 | 0 | 0 | 106 | 0 | 0 | 100 | 0 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

```
Area Type:
Other
```

Control Type: Unsignalized



|  | $t$ | $\rightarrow$ | 7 | 7 | 4 | 4 | 4 | $\dagger$ | $p$ | * | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | A1\% |  | 7 | 性 |  |  | $t$ |  |  | $\ddagger$ |  |
| Traffic Volume (vph) | 23 | 807 | 4 | 62 | 843 | 27 | 0 | 8 | 45 | 7 | 1 | 8 |
| Future Volume (vph) | 23 | 807 | 4 | 62 | 843 | 27 | 0 | 8 | 45 | 7 | 1 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width ( ft ) | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 16 | 12 | 12 | 12 | 12 |
| Storage Length (ft) | 0 |  | 0 | 240 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 0 |  | 0 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (tt) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Utill. Factor | 0.95 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Fit |  | 0.999 |  |  | 0.995 |  |  | 0.886 |  |  | 0.935 |  |
| FIt Protected |  | 0.999 |  | 0.950 |  |  |  |  |  |  | 0.978 |  |
| Satd. Flow (prot) | 0 | 3285 | 0 | 1770 | 3390 | 0 | 0 | 1837 | 0 | 0 | 1615 | 0 |
| Flt Permitted |  | 0.999 |  | 0.950 |  |  |  |  |  |  | 0.978 |  |
| Satd. Flow (perm) | 0 | 3285 | 0 | 1770 | 3390 | 0 | 0 | 1837 | 0 | 0 | 1615 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (fi) |  | 823 |  |  | 845 |  |  | 358 |  |  | 307 |  |
| Travel Time (s) |  | 18.7 |  |  | 19.2 |  |  | 8.1 |  |  | 7.0 |  |
| Adj. Flow (vph) | 27 | 961 | 5 | 70 | 947 | 30 | 0 | 10 | 55 | 12 | 2 | 13 |
| Lane Group Flow (vph) | 0 | 993 | 0 | 70 | 977 | 0 | 0 | 65 | 0 | 0 | 27 | 0 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Other <br> Control Type: Unsignalized | Other |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |




| Minor Lane/Major Mvmt | NBLIT | EBL | EBT | EBR | WBL | WBT | WBR S | SBLn1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Capacity (veh/h) | 169 | 714 | - | - | 709 | - | - | 67 |
| HCM Lane V/C Ratio | 0.382 | 0.038 | - | - | 0.098 | - | - | 0.398 |
| HCM Control Delay (s) | 38.9 | 10.2 | 0.4 | - | 10.6 | - | - | 90.5 |
| HCM Lane LOS | E | B | A | - | B | - | - | F |
| HCM 95th \%tile Q(veh) | 1.6 | 0.1 | . | - | 0.3 | . | . | 1.5 |


|  | 4 | $\rightarrow$ | 1 | 7 | $\leftarrow$ | 4 | 4 | $\uparrow$ | 7 | * | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | AT |  | ${ }^{7}$ | 49 |  |  | 1 |  |  | $\$$ |  |
| Traffic Volume (vph) | 12 | 936 | 14 | 90 | 1129 | 5 | 0 | 4 | 93 | 25 | 13 | 32 |
| Future Volume (vph) | 12 | 936 | 14 | 90 | 1129 | 5 | 0 | 4 | 93 | 25 | 13 | 32 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width ( ft ) | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 16 | 12 | 12 | 12 | 12 |
| Storage Length (ft) | 0 |  | 0 | 240 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 0 |  | 0 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (tt) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance ( t ) |  | 823 |  |  | 845 |  |  | 358 |  |  | 387 |  |
| Travel Time (s) |  | 18.7 |  |  | 19.2 |  |  | 8.1 |  |  | 8.8 |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.88 | 0.88 | 0.88 | 0.86 | 0.86 | 0.86 | 0.50 | 0.50 | 0.50 |
| Heavy Vehicles (\%) | 0\% | 2\% | 0\% | 0\% | 3\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 5\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 1045 | 0 | 102 | 1289 | 0 | 0 | 113 | 0 | 0 | 140 | 0 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: Control Type: Unsignalized |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |




| Approach | EB | WB | NB | SB |
| :--- | :---: | :---: | ---: | ---: |
| HCM Control Delay, s | 0.4 | 0.8 | 28.1 | $\$ 1528$ |
| HCM LOS |  |  | D | F |



|  | $\stackrel{ }{*}$ |  |  |  |  |  | 4 | $\uparrow$ | 7 |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow_{\text {¢ }}$ |  | ${ }^{7}$ | 性 |  |  | $\hat{\beta}$ |  |  | \$ |  |
| Traffic Volume (vph) | 24 | 807 | 4 | 62 | 843 | 27 | 0 | 8 | 45 | 7 | 1 | 8 |
| Future Volume (vph) | 24 | 807 | 4 | 62 | 843 | 27 | 0 | 8 | 45 | 7 | 1 | 8 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (tt) | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 16 | 12 | 12 | 12 | 12 |
| Storage Length (tt) | 0 |  | 0 | 240 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 0 |  | 0 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length ( t ) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 0.95 | 0.95 | 0.95 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.999 |  |  | 0.995 |  |  | 0.886 |  |  | 0.935 |  |
| Flt Protected |  | 0.999 |  | 0.950 |  |  |  |  |  |  | 0.978 |  |
| Satd. Flow (prot) | 0 | 3285 | 0 | 1770 | 3390 | 0 | 0 | 1837 | 0 | 0 | 1615 | 0 |
| Flt Permitted |  | 0.999 |  | 0.950 |  |  |  |  |  |  | 0.978 |  |
| Satd. Flow (perm) | 0 | 3285 | 0 | 1770 | 3390 | 0 | 0 | 1837 | 0 | 0 | 1615 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance ( t ) |  | 823 |  |  | 845 |  |  | 358 |  |  | 537 |  |
| Travel Time (s) |  | 18.7 |  |  | 19.2 |  |  | 8.1 |  |  | 12.2 |  |
| Adj. Flow (vph) | 29 | 961 | 5 | 70 | 947 | 30 | 0 | 10 | 55 | 12 | 2 | 13 |
| Lane Group Flow (vph) | 0 | 995 | 0 | 70 | 977 | 0 | 0 | 65 | 0 | 0 | 27 | 0 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: | Other |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |  |  |  |  |  |  |




|  | $\rangle$ | $\rightarrow$ | 7 | 7 |  |  | 4 | $\uparrow$ | 7 |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * $\uparrow$ |  | \% | 性 |  |  | $\hat{\beta}$ |  |  | ¢ |  |
| Traffic Volume (vph) | 12 | 936 | 14 | 90 | 1129 | 5 | 0 | 4 | 93 | 25 | 13 | 33 |
| Future Volume (vph) | 12 | 936 | 14 | 90 | 1129 | 5 | 0 | 4 | 93 | 25 | 13 | 33 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 12 | 12 | 12 | 12 | 12 | 12 | 12 | 16 | 12 | 12 | 12 | 12 |
| Storage Length (tt) | 0 |  | 0 | 240 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 0 |  | 0 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (t) |  | 823 |  |  | 845 |  |  | 358 |  |  | 547 |  |
| Travel Time (s) |  | 18.7 |  |  | 19.2 |  |  | 8.1 |  |  | 12.4 |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.88 | 0.88 | 0.88 | 0.86 | 0.86 | 0.86 | 0.50 | 0.50 | 0.50 |
| Heavy Vehicles (\%) | 0\% | 2\% | 0\% | 0\% | 3\% | 0\% | 0\% | 0\% | 0\% | 0\% | 0\% | 5\% |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 1045 | 0 | 102 | 1289 | 0 | 0 | 113 | 0 | 0 | 142 | 0 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: |  |  |  |  |  |  |  |  |  |  |  |  |

Control Type: Unsignalized

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 80.9 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * $\uparrow$ |  | ${ }^{7}$ | 中 ${ }^{\text {a }}$ |  |  | $\uparrow$ |  |  | $\uparrow$ |  |
| Traffic Vol, veh/h | 12 | 936 | 14 | 90 | 1129 | 5 | 0 | 4 | 93 | 25 | 13 | 33 |
| Future Vol, veh/h | 12 | 936 | 14 | 90 | 1129 | 5 | 0 | 4 | 93 | 25 | 13 | 33 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | 240 | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 88 | 88 | 88 | 86 | 86 | 86 | 50 | 50 | 50 |
| Heavy Vehicles, \% | 0 | 2 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| Mvmt Flow | 13 | 1017 | 15 | 102 | 1283 | 6 | 0 | 5 | 108 | 50 | 26 | 66 |


| Major/Minor | Major1 |  |  | Major2 |  |  | Minor1 |  |  | Minor2 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1289 | 0 | 0 | 1032 | 0 | 0 | - | 2544 | 516 | 2027 | 2548 | 645 |  |
| Stage 1 | - | - | - | - | - | - | - | 1051 | - | 1490 | 1490 | - |  |
| Stage 2 | - | - | - | - | - | - | - | 1493 | - | 537 | 1058 | - |  |
| Critical Hdwy | 4.1 | - | - | 4.1 | - | - | - | 6.5 | 6.9 | 7.5 | 6.5 | 7 |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | - | 5.5 | - | 6.5 | 5.5 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | - | 5.5 | - | 6.5 | 5.5 | - |  |
| Follow-up Hdwy | 2.2 | - | - | 2.2 | - | - | - | 4 | 3.3 | 3.5 | 4 | 3.35 |  |
| Pot Cap-1 Maneuver | 545 | - | - | 681 | - | - | 0 | 27 | 509 | - 35 | 27 | 408 |  |
| Stage 1 | - | - | - | - | - | - | 0 | 306 | - | 132 | 189 | - |  |
| Stage 2 | - | - | - | - | - | - | 0 | 188 | - | 501 | 304 | - |  |
| Platoon blocked, \% |  | - | - |  | - | - |  |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | 545 | - | - | 681 | - | - | - | 22 | 509 | ~ 20 | ~ 22 | 408 |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | - | 22 | - | ~ 20 | $\sim 22$ | - |  |
| Stage 1 | - | - | - | - | - | - | - | 289 | - | 125 | 161 | - |  |
| Stage 2 | - | - | - | - | - | - | - | 160 | - | 366 | 287 | - |  |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |  |
| HCM Control Delay, s | 0.4 |  |  | 0.8 |  |  | 28.1 |  |  | 1499.6 |  |  |  |
| HCM LOS |  |  |  |  |  |  | D |  |  | F |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR S | BLn1 |  |  |  |  |
| Capacity (veh/h) |  | 266 | 545 | - | - | 681 | - | - | 37 |  |  |  |  |
| HCM Lane V/C Ratio |  | 0.424 | 0.024 | - | - | 0.15 | - | - | 3.838 |  |  |  |  |
| HCM Control Delay (s) |  | 28.1 | 11.8 | 0.3 | - | 11.2 | - | \$1 | 499.6 |  |  |  |  |
| HCM Lane LOS |  | D | B | A | - | B | - | - | F |  |  |  |  |
| HCM 95th \%tile Q(veh) |  | 2 | 0.1 | - | - | 0.5 | - | - | 16.4 |  |  |  |  |
| Notes |  |  |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ : Volume exceeds cap | pacity | \$: Delay exceeds 300s |  |  |  | +: Computation Not Defined |  |  |  | *: All major volume in platoon |  |  |  |

Picker Road/Technology Park Road

|  | 1 | 4 |  | $p$ | * | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M |  | $\hat{\beta}$ |  |  | $\uparrow$ |
| Traffic Volume (vph) | 2 | 0 | 34 | 4 | 0 | 10 |
| Future Volume (vph) | 2 | 0 | 34 | 4 | 0 | 10 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 11 | 12 | 12 | 12 | 12 | 12 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | $1: 00$ | 1.00 | 1.00 |
| Frt |  |  | 0.987 |  |  |  |
| Fit Protected | 0.950 |  |  |  |  |  |
| Satd. Flow (prot) | 1711 | 0 | 1839 | 0 | 0 | 1863 |
| Fll Permitted | 0.950 |  |  |  |  |  |
| Satd. Flow (perm) | 1711 | 0 | 1839 | 0 | 0 | 1863 |
| Link Speed (mph) | 30 |  | 30 |  |  | 30 |
| Link Distance ( ft ) | 665 |  | 407 |  |  | 330 |
| Travel Time (s) | 15.1 |  | 9.3 |  |  | 7.5 |
| Adj. Flow (vph) | 2 | 0 | 37 | 4 | 0 | 11 |
| Lane Group Flow (vph) | 2 | 0 | 41 | 0 | 0 | 11 |
| Sign Control | Stop |  | Free |  |  | Free |

Intersection Summary

```
Area Type:
Other
```

Control Type: Unsignalized

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :--- |
| Int Delay, s/veh | 0.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M' |  | T |  |  | -1 |
| Traffic Vol, veh/h | 2 | 0 | 34 | 4 | 0 | 10 |
| Future Vol, veh/h | 2 | 0 | 34 | 4 | 0 | 10 |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 0 | 37 | 4 | 0 | 11 |


| Major/Minor | Minor1 | Major1 |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | :--- | :--- |
| Major2 |  |  |  |  |  |  |
| Conflicting Flow All | 50 | 39 | 0 | 0 | 41 | 0 |
| $\quad$ Stage 1 | 39 | - | - | - | - | - |
| Stage 2 | 11 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | -2.218 | - |  |
| Pot Cap-1 Maneuver | 959 | 1033 | - | -1568 | - |  |
| $\quad$ Stage 1 | 983 | - | - | - | - | - |
| $\quad$ Stage 2 | 1012 | - | - | - | - | - |
| Platoon locked, \% |  |  | - | - | - |  |
| Mov Cap-1 Maneuver | 959 | 1033 | - | - | 1568 | - |
| Mov Cap-2 Maneuver | 959 | - | - | - | - | - |
| Stage 1 | 983 | - | - | - | - | - |
| Stage 2 | 1012 | - | - | - | - | - |


| Approach | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 8.8 | 0 | 0 |
| HCM LOS | A |  |  |


| Minor Lane/Major Mvmt | NBT | NBRWBLLI | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | - | -959 | 1568 | - |
| HCM Lane V/C Ratio | - | -0.002 | - | - |
| HCM Control Delay (s) | - | - | 8.8 | 0 |
| HCM Lane LOS | - | - | A | A |
| HCM | - |  |  |  |
| H5th \%tile Q(veh) | - | - | 0 | 0 |


|  | $t$ | 4 | 4 | 7 |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * |  | $\dagger$ |  |  | 4 |
| Trafic Volume (vph) | 5 | 0 | 13 | 2 | 0 | 45 |
| Future Volume (vph) | 5 | 0 | 13 | 2 | 0 | 45 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 11 | 12 | 12 | 12 | 12 | 12 |
| Link Speed (mph) | 30 |  | 30 |  |  | 30 |
| Link Distance (ft) | 530 |  | 327 |  |  | 410 |
| Travel Time (s) | 12.0 |  | 7.4 |  |  | 9.3 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 5 | 0 | 16 | 0 | 0 | 49 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary  <br> Area Type:  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Control Type: Unsignalize |  |  |  |  |  |  |




| Approach | WB | NB | SB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay, s | 8.8 | 0 | 0 |


| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | - | -942 | 1602 | - |
| HCM Lane V/C Ratio | - | -0.006 | - | - |
| HCM Control Delay (s) | - | - | 8.8 | 0 |

## 2027 No-Build Wkdy AM Peak

6: Picker Road \& Tehnology Park Road

|  | 7 | 4 | $\uparrow$ | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \% |  | $\hat{\dagger}$ |  |  | $\uparrow$ |
| Traffic Volume (vph) | 2 | 0 | 52 | 6 | 0 | 14 |
| Future Volume (vph) | 2 | 0 | 52 | 6 | 0 | 14 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 11 | 12 | 12 | 12 | 12 | 12 |
| Lane Utili. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Fit |  |  | 0.985 |  |  |  |
| Flt Protected | 0.950 |  |  |  |  |  |
| Satd. Flow (prot) | 1711 | 0 | 1835 | 0 | 0 | 1863 |
| FIt Permitted | 0.950 |  |  |  |  |  |
| Satd. Flow (perm) | 1711 | 0 | 1835 | 0 | 0 | 1863 |
| Link Speed (mph) | 30 |  | 30 |  |  | 30 |
| Link Distance (ft) | 453 |  | 307 |  |  | 430 |
| Travel Time (s) | 10.3 |  | 7.0 |  |  | 9.8 |
| Adj. Flow (vph) | 2 | 0 | 57 | 7 | 0 | 15 |
| Lane Group Flow (vph) | 2 | 0 | 64 | 0 | 0 | 15 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: OtherControl Type: Unsignalized |  |  |  |  |  |  |
|  |  |  |  |  |  |  |




| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | - | -927 | 1538 | - |
| HCM Lane V/C Ratio | - | -0.002 | - | - |
| HCM Control Delay (s) | - | - | 8.9 | 0 |


|  | $\downarrow$ | 4 | 4 | 1 |  | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \% |  | t |  |  | 4 |
| Traffic Volume (vph) | 7 | 0 | 19 | 2 | 0 | 63 |
| Future Volume (vph) | 7 | 0 | 19 | 2 | 0 | 63 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 11 | 12 | 12 | 12 | 12 | 12 |
| Link Speed (mph) | 30 |  | 30 |  |  | 30 |
| Link Distance (ft) | 500 |  | 387 |  |  | 350 |
| Travel Time (s) | 11.4 |  | 8.8 |  |  | 8.0 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 8 | 0 | 23 | 0 | 0 | 68 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: OtherControl Type: Unsignalized |  |  |  |  |  |  |
|  |  |  |  |  |  |  |


|  |  |  |  |  |  |  |  |
| :--- | ---: | :--- | ---: | ---: | ---: | ---: | :--- |


| Major/Minor | Minor1 | Major1 | Major2 |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Conflicting Flow All | 90 | 22 | 0 | 0 | 23 | 0 |



| Minor Lane/Major Mvmt | NBT | NBRWBLn1 | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | - | -910 | 1592 | - |
| HCM Lane V/C Ratio | - | -0.008 | - | - |
| HCM Control Delay (s) | - | - | 9 | 0 |
| - |  |  |  |  |
| HCM Lane LOS | - | - | A | A |
| HCM 95th \%tile Q(veh) | - | - | 0 | 0 |
| H | - |  |  |  |


|  |  |  | ¢ |  | * | $\dagger$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \% |  | 个 |  |  | $\uparrow$ |
| Traffic Volume (vph) | 2 | 0 | 52 | 7 | 0 | 14 |
| Future Volume (vph) | 2 | 0 | 52 | 7 | 0 | 14 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  | 0.983 |  |  |  |
| Flt Protected | 0.950 |  |  |  |  |  |
| Satd. Flow (prot) | 1770 | 0 | 1831 | 0 | 0 | 1863 |
| Flt Permitted | 0.950 |  |  |  |  |  |
| Satd. Flow (perm) | 1770 | 0 | 1831 | 0 | 0 | 1863 |
| Link Speed (mph) | 30 |  | 30 |  |  | 30 |
| Link Distance (ft) | 512 |  | 537 |  |  | 200 |
| Travel Time (s) | 11.6 |  | 12.2 |  |  | 4.5 |
| Adj. Flow (vph) | 2 | 0 | 57 | 8 | 0 | 15 |
| Lane Group Flow (vph) | 2 | 0 | 65 | 0 | 0 | 15 |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: |  |  |  |  |  |  |

Control Type: Unsignalized

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 2 | 0 | 52 | 7 | 0 | 14 |
| Future Vol, veh/h | 2 | 0 | 52 | 7 | 0 | 14 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 0 | 57 | 8 | 0 | 15 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 76 | 61 | 0 | 0 | 65 | 0 |
| Stage 1 | 61 | - | - | - | - | - |
| Stage 2 | 15 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 927 | 1004 | - | - | 1537 | - |
| Stage 1 | 962 | - | - | - | - | - |
| Stage 2 | 1008 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 927 | 1004 | - | - | 1537 | - |
| Mov Cap-2 Maneuver | 927 | - | - | - | - | - |
| Stage 1 | 962 | - | - | - | - | - |
| Stage 2 | 1008 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 8.9 |  | 0 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 927 | 1537 | - |
| HCM Lane V/C Ratio |  | - | - | 0.002 | - | - |
| HCM Control Delay (s) |  | - | - | 8.9 | 0 | - |
| HCM Lane LOS |  | - | - | A | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0 | 0 | - |


|  | $\checkmark$ | 4 |  |  | $\pm$ | $\frac{1}{7}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * |  | $\uparrow$ |  |  | 4 |
| Traffic Volume (vph) | 8 | 0 | 19 | 2 | 0 | 63 |
| Future Volume (vph) | 8 | 0 | 19 | 2 | 0 | 63 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Width (ft) | 11 | 12 | 12 | 12 | 12 | 12 |
| Link Speed (mph) | 30 |  | 30 |  |  | 30 |
| Link Distance (ft) | 411 |  | 547 |  |  | 190 |
| Travel Time (s) | 9.3 |  | 12.4 |  |  | 4.3 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 9 | 0 | 23 | 0 | 0 | 68 |
| Sign Control | Stop |  | Free |  |  | Free |

## Intersection Summary <br> Area Type: Other

Control Type: Unsignalized

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 8 | 0 | 19 | 2 | 0 | 63 |
| Future Vol, veh/h | 8 | 0 | 19 | 2 | 0 | 63 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 0 | 21 | 2 | 0 | 68 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 90 | 22 | 0 | 0 | 23 | 0 |
| Stage 1 | 22 | - | - | - | - | - |
| Stage 2 | 68 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 910 | 1055 | - | - | 1592 | - |
| Stage 1 | 1001 | - | - | - | - | - |
| Stage 2 | 955 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 910 | 1055 | - | - | 1592 | - |
| Mov Cap-2 Maneuver | 910 | - | - | - | - | - |
| Stage 1 | 1001 | - | - | - | - | - |
| Stage 2 | 955 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 9 |  | 0 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 910 | 1592 | - |
| HCM Lane V/C Ratio |  | - | - | 0.01 | - | - |
| HCM Control Delay (s) |  | - | - | 9 | 0 | - |
| HCM Lane LOS |  | - | - | A | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0 | 0 | - |


[^0]:    ${ }^{1}$ The statutory or "prima facie" speed is defined in M.G.L. c. $90 \S 17$, as the speed which would be deemed reasonable and proper to operate a motor vehicle.

[^1]:    ${ }^{2}$ Transportation Impact Assessment, Proposed Cold Storage Warehouse, 6 Picker Road, Sturbridge, Massachusetts; VAI; July 2020.
    ${ }^{3}$ MassDOT Traffic Volumes for the Commonwealth of Massachusetts; 2020.

[^2]:    ${ }^{4} \mathrm{~A}$ minimum combined travel lane and paved shoulder width of 14 -feet is required to support bicycle travel in a shared traveledway condition.

[^3]:    ${ }^{\text {a }}$ Source: MassDOT Safety Management/Traffic Operations Unit records, 2013 through 2017.
    ${ }^{\mathrm{b}}$ Traffic Control Type: $\mathrm{U}=$ unsignalized; $\mathrm{TS}=$ traffic signal.
    ${ }^{\mathrm{c}}$ Crash rate per million vehicles entering the intersection.
    ${ }^{\mathrm{d}}$ Statewide/District crash rate.
    ${ }^{\mathrm{e}}$ The intersection crash rate is significant if it is found to exceed the MassDOT statewide and/or District crash rate for the MassDOT Highway Division District in which the Project is located (District 3).

[^4]:    ${ }^{5}$ Trip Generation, $10^{\text {th }}$ Edition; Institute of Transportation Engineers; Washington, DC; 2017.

[^5]:    ${ }^{6}$ Highway Capacity Manual, Transportation Research Board; Washington, DC; 2010.

[^6]:    ${ }^{7}$ A Policy on Geometric Design of Highway and Streets, 7th Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2018.

[^7]:    ${ }^{8}$ Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.

[^8]:    N/S Street : Route 49
    City/State: Charlton, MA

[^9]:    N/S Street: Route 49
    EN Streer: Route 20
    City/State : Chartton, MA
    Weather : Clear

[^10]:    N/S Street : Route 49 City/State : Charlton, MA

