STORMWATER MANAGEMENT REPORT

"55+ MANUFACTURED HOUSING COMMUNITY" LOT 3 BERRY FARMS ROAD STURBRIDGE, MA 01566

Prepared for: Justin Stelmok 557 Southwest Cutoff Worcester, MA 01607



4-28-23

March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023



Table of Contents

List of Sections

I. Introduction

- A. Scope of Analysis
- B. Site Description
- C. Proposed Development

II. Hydrologic Analysis

- A. Purpose
- B. Methodology
- C. Selection of Storm Events
- D. Soils Classification
- E. Pre-Development Model Summary
- F. Post-Development Model Summary
- G. Summary of Peak Stormwater Discharge Rates

III: Compliance with Stormwater Standards

- A. Standard 1 Computations to Show That Discharge Does Not Cause Scour or Erosion
- B. Standard 2 Peak Rate Attenuation
- C. Standard 3 Recharge
- D. Standard 4 Required Water Quality
- E. Standard 5 Land Uses with Higher Pollutant Loads
- F. Standard 6 Critical Areas
- G. Standard 7 Redevelopment
- H. Standard 8 Construction Period Controls
- I. Standard 9 Operation and Maintenance Plan
- J. Standard 10 Illicit Discharges to Drainage System

List of Tables

- 1 Pre vs. Post-Development Stormwater Runoff and Volume Summary
- 2 Standard 3 & 4 Recharge & Water Quality Volume Calculations
- **3** Stormwater Management Calculations TSS Removal

List of Appendices

- A. MA-DEP Stormwater Checklist
- B. Figures 1.1 and 1.2 USGS Site Maps
- C. Figure 2 Ortho Imagery
- FEMA FIRM Flood Plain Mapping NCRS Soil Mapping Rawls Table NOAA Rainfall Data
- D. Pre-Development HydroCAD Drainage Calculations
- E. Post-Development HydroCAD Drainage Calculations
- F. Additional Drainage Calculation Worksheets
- G. Construction Period Stormwater Pollution Prevention Plan & Weekly Inspection Form
- **H.** Stormwater Management System Long-Term Operation & Maintenance (O & M) Plan

A. Scope of Analysis

The project Applicant, Justin Stelmok, retained McClure Engineering, Inc. (McClure) to prepare this engineering analysis of pre and post-development drainage runoff conditions for the proposed "55+ Manufactured Housing Community" Plan for the property located at Lot 3 Berry Farms Road, Sturbridge, MA (Site).

This Stormwater Management Report provides the required analysis of the proposed stormwater system for compliance with the Town of Sturbridge Bylaw requirements, and the Massachusetts 310 CMR 10.00 Wetland Protection Regulations as promulgated by the Commissioner of the Massachusetts Department of Environmental Protection (MassDEP) pursuant to the authority granted under the Wetland Protection Act, M.G.L. c. 131 sec. 40 (WPA). The analysis includes pre- and post- conditions hydrologic modeling, and hydraulic sizing of the conveyance systems, sizing and analysis of Stormwater Best Management Practices (BMPs) of structural or non-structural techniques for managing stormwater to prevent or reduce non-point source pollutants from entering surface waters or ground waters. This report will demonstrate that the stormwater management system as designed and laid out at Lot 3 Berry Farms Road, Sturbridge, MA, complies with the referenced regulations.

A copy of the "MA-DEP Checklist for Stormwater Report" is included as Appendix A.

B. Site Description

The subject site consists of approximately 41.5 acres. The property lies on the northern side of Main Street and along the Southbridge Town Line. The property is shown as Lot 3 of the Berry Farms Road Definitive Subdivision. The site is located within the Town of Sturbridge Rural Residential zoning district. The existing site consists of mostly wooded area, as well as wetlands. The site has previously been logged and some existing logging trails still exist throughout the property. The site topography slopes generally in a westerly direction towards a valley containing wetlands. The site is surrounded by wetlands on the western boundary, as well as (3) vernal pools as determined by LEC Environmental.

The site is located within an area of minimal flood hazard (Zone X) per Flood Insurance Rate Map (FIRM) Worcester County Massachusetts (All Jurisdictions), Map Number 25027C0933E, effective on 07/04/2011 (see Appendix C).

C. Proposed Construction

The proposed site layout is for the construction of a 55+ Manufactured Housing Community. The community is proposed with (4) 20' wide private roads, (3) cul-de-sacs, (1) emergency access drive through the Town of Southbridge, a common clubhouse and active open space area, and (67) total units. The community will be serviced by municipal water and sewer through Berry Farms Road. The stormwater management system for the site consists of country style drainage, including swales and rain gardens with minimal structures for conveyance. Rain gardens will be placed between all units, and will act as a stormwater structure, but also on-site landscaping and yard separation/privacy barrier. Other than a single deep sump and hooded catch basin in the parking lot for the club house, all stormwater will be conveyed on the surface to rain gardens. These rain gardens will provide for peak flow attenuation, water quality treatment, and groundwater recharge. A total of (77) rain gardens are proposed, with the majority being smaller rain gardens positioned between units which will detain and treat runoff from the units, roads, and driveway. A few larger secondary rain gardens are also proposed. A single large infiltration basin is proposed within an existing natural depression. Interception trenches are proposed behind the units on Roads A and D to convey clean runoff from the undeveloped portions of the property towards the existing discharge points of the property.

The "Special Permit and Site Plan, Blueberry Hill Estates, 55+ Manufactured Housing Community, Lot 3 Berry Farms Road, Sturbridge, MA" Plan Set prepared by McClure Engineering, Inc., dated 4/28/23 provides details of the complete stormwater management system design.

A. <u>Purpose</u>

The purpose of this analysis is to determine the peak rate of stormwater runoff leaving the site and to design a stormwater management system that will prevent offsite flooding impacts. MassDEP Stormwater Management Policy, Standard No. 2, requires that post-development peak stormwater discharge rates shall not exceed predevelopment levels.

B. Methodology

The pre- and post-development stormwater runoff has been analyzed using HydroCAD, a stormwater modeling computer program. HydroCAD is a collection of techniques for the generation and routing of hydrographs, including Soil Conservation Service (SCS) Technical Release No. 20 (TR-20) and SCS Technical Release 55 (TR-55), Urban Hydrology for Small Watersheds. The analysis routes completely through one node at a time determining each outflow hydrograph before considering the next node.

The subcatchments have been modeled using SCS methods. Curve numbers, which are based upon the type of development and soil classifications, coupled with the time of concentration have been used to generate the peak storm flow for each area. The detailed information and results are provided in this report.

Hydrology Computer Model:	HydroCAD 10.0 $\ensuremath{\mathbb{C}}$ 2013 Applied Microcomputer Systems, drainage modeling software;
Hydrologic Methodology:	TR-55 Methodology is used for analysis of peak flow and infiltration basin sizing.
Watershed Areas:	Watershed areas are calculated using AutoCAD software based on the subcatchment areas delineated on topographic mapping included as "Pre-Development Drainage" and "Post-Development Drainage". The areas shown, times of concentration and runoff coefficients are all consistent with the TR-55 drainage calculation method.

C. Selection of Storm Events

The intensity for each storm event was determined from the National Oceanic and Atmospheric Administration National Weather Service Atlas 14 Point Precipitation Frequency Estimates (See Appendix C). Evaluations were based upon a Type III, 24-hour storm. Rainfall frequency and intensity used in this analysis are as follows:

Design Storm Event	Rainfall Intensity
2 year	3.24 inches
10 year	5.05 inches
25 year	6.18 inches
100 year	7.93 inches

D. Soils Classification

Site soils classifications were obtained from the following sources:

 Advanced soil mapping performed by the U.S. Department of Agriculture's Natural Resources Conservation Service (NRCS), "Soil Survey of Worcester County, Massachusetts, Southern Part." (See Appendix C for detailed soil information). The soils descriptions are mapped as follows:

71B - Ridgebury Fine Sandy Loam - "HSG D"

305C - Paxton Fine Sandy Loam - "HSG C"

307C – Paxton Fine Sandy Loam – "HSG C"

312B - Woodbridge Fine Sandy Loam - "HSG C"

2.) On site soil testing performed by Peter Engle, P.E. (SE#14009) on 9/3/20 and 4/12/23

Testing pit locations and results are shown on the Existing Conditions Plans within the Plan Set.

Soil Permeability (k):

Design permeability (k) value: k = 2.41 in / hr (Rawls Rate for Loamy Sand based upon on-site soil testing) k = 1.02 in / hr (Rawls Rate for Sandy Loam based upon on-site soil testing)

E. <u>Pre-Development Model Summary</u>

The pre-development hydrologic model analyzes the existing stormwater runoff from the site to (8) analysis points. The analysis points are: Southbridge Parcel 019-048 (0 Cournoyer Blvd), Wetland Series A (Flags A56-87), Wetland Series A (Flags A47-A56 and downstream Vernal Pool), Wetland Series A (Flags A32-A47), Wetland Series A (Flags A23-32 and upstream Vernal Pool), Wetland Series B (off-site), Wetland Series E (Vernal Pool), and Southbridge Parcel 032-092 Idlewood Street. The graphical presentation of the pre-development model is shown in Appendix D.

F. Post-Development Model Summary

The configuration of the post development analysis points, sub-catchments, ponds and reaches are generally configured as the pre-model. The post-development subcatchment has been broken into several smaller subcatchments for the analysis, in order to properly size the proposed rain gardens, infiltration basin, pipe network, etc. The analysis points are the same as the pre-development model. The graphical presentation of the post-development model is shown in Appendix E. For ease of the model, areas of the proposed units have been modeled as Residential Development – 1/8 acre lots with 65% impervious surface. The model should be considered conservative as the average lot is closer to 55% impervious surface as taken from the site plan. The unit sizes used in the analysis are also the largest units that will be made available to prospective buyers, and it is very unlikely all units will be this size.

G. Summary of Peak Stormwater Discharge Rates

The Pre- and Post-Analyses HydroCAD Reports of the 2, 10, 25 and 100 year frequency storms are provided in Appendix D and E respectively. The following summary table present results for the pre- and post-development analysis for the 2, 10, 25 and 100 year, 24-hr storm events at the analysis point as previously described. The table shows that post peak rate of runoff is less than or equal to that of pre-existing peak rate of runoff for all the storms as studied.

Table No. 1

Analysis Point	1. Southbridge	Parcel 019-048	0 Courno	ver Blvd)
	1. Ooutibriugo		o oounio	yor Diva)

	Pre-Development (cfs)	Post-Development (cfs)
2 Year Storm	2.58	2.56
10 Year Storm	6.90	6.86
25 Year Storm	9.96	9.90
100 Year Storm	15.00	14.91

"55+ Manufactured Housing Community", Lot 3 Berry Farms Road, Sturbridge, MA – Stormwater Management Report

Table No. 2 Analysis Point 2: Wetland A (A56-A87)

	Pre-Development (cfs)	Post-Development (cfs)
2 Year Storm	7.95	7.92
10 Year Storm	22.72	20.93
25 Year Storm	33.38	31.68
100 Year Storm	51.05	47.25

Table No. 3

Analysis Point 3: Wetland (A47-A56 / downstream A series Vernal Pool)

	Pre-Development (cfs)	Post-Development (cfs)
2 Year Storm	0.34	0.33
10 Year Storm	2.55	2.07
25 Year Storm	4.48	3.58
100 Year Storm	7.93	6.69

Table No. 4

Analysis Point 4: Wetland Series A (A32-A47)

	Pre-Development (cfs)	
2 Year Storm	3.07	2.65
10 Year Storm	9.20	8.81
25 Year Storm	13.66	13.2
100 Year Storm	21.07	18.44

Table No. 5

Analysis Point 5: Wetland Series A (A23-A32/Upstream Vernal Pool)

	Pre-Development (cfs)	Post-Development (cfs)
2 Year Storm	2.87	2.77
10 Year Storm	8.60	7.22
25 Year Storm	12.80	10.22
100 Year Storm	19.79	18.49

Table No. 6 Analysis Point 6: Wetland Series B

	Pre-Development (cfs)	Post-Development (cfs)
2 Year Storm	1.57	1.37
10 Year Storm	4.36	4.34
25 Year Storm	6.35	6.35
100 Year Storm	9.62	9.58

Table No. 7

Analysis Point 7: Wetland Series E

	Pre-Development (cfs)	Post-Development (cfs)
2 Year Storm	1.61	1.4
10 Year Storm	6.18	5.98
25 Year Storm	9.74	9.23
100 Year Storm	15.87	15.48

Table No. 8

Analysis Point 8: Southbridge Parcel 032-092 Idlewood Street

	Pre-Development (cfs)	Post-Development (cfs)
2 Year Storm	0.40	0.08
10 Year Storm	1.05	1.04
25 Year Storm	1.50	1.46
100 Year Storm	2.23	2.18

A. <u>Standard 1 – Computations to Show That Discharge Does Not Cause Scour or Erosion</u>

No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

Proposed Full Compliance:

The site drainage system has been designed from calculations based upon the 100-year design storm event using the peak flows predicted by the HydroCAD 10 Dynamic Modelling Program. The Manning's Equation has been used to size the drainage system pipe runs.

Manning's Equation: Q = A 1.486 R2/3 S1/2 / n Where: Q = Flow Discharge, cfs A = Cross Sectional Area of Wetted Perimeter n = Manning Coefficient of Channel Roughness R = Hydraulic Radius (A/WP) WP = Wetted Perimeter S = Slope of Energy Gradient

No new untreated discharges are proposed for the development. All stormwater discharges for the site will have been conveyed through water quality treatment BMPs which meet Standard 4 prior to discharge. All stormwater discharges will also be conveyed to either rip rap outfalls or perforated pipe level spreaders to reduce runoff velocities and to prevent erosion or sedimentation of downstream discharge points. Rip rap outfalls and level spreaders as shown on the site plans have been designed for the applicable flows and velocities directed towards them.

B. Standard 2 – Peak Rate Attenuation

Stormwater management systems must be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates. This Standard may be waived for land subject to coastal storm flowage.

Proposed Full Compliance:

The peak rate attenuation analyses and summaries have been reported in hydrologic analysis provided in Section D of this report documenting there is no increase to off-site peak flow rates. A review of FEMA Flood Insurance Rate Map (FIRM) #25027C0933E (reduced scale provided in Appendix C) was reviewed for this site. The site is located in an area of minimal flood hazard (Zone X). The analysis as submitted indicates that there will be no increase in rate of runoff that would cause an increase of the flooding downstream.

C. Standard 3 – Recharge

Loss of annual recharge to ground water shall be eliminated or minimized through the use of infiltration measures including environmentally sensitive site design, low impact development techniques, best management practices, and good operation and maintenance. At a minimum, the annual recharge from the post-development conditions based on soil type. This Standard is met when the storm water management system is design to infiltrate the required recharge volume as determined in accordance with the Massachusetts Storm water Handbook.

Proposed Full Compliance:

The majority of the stormwater runoff from the proposed impervious area will be directed to the proposed rain gardens. Rain gardens within areas of fill and which will meet the required separation to groundwater and bedrock will provide for groundwater recharge as well as peak flow attenuation and water quality treatment. Rain gardens in areas of cut which will not meet the required separation to groundwater or bedrock will be lined with a 10 mil. impervious poly barrier and will only provide for peak flow attenuation and water quality treatment. A single large

infiltration basin is also proposed and will provide for significant ground water recharge. All together the rain gardens and the infiltration basin will far exceed the required recharge volume. See Appendix F for computations of Standards 3 and 4. The following is a summary of the recharge for the three basins

- 1. Required Recharge Volume
 - a. Impervious Area, as obtained from proposed Site Plan:

103,640 s.f. HSG B 219,110 s.f. HSG C

10,730 s.f. HSG D

- Required recharge volume Rv = F x Impervious Area (F = target depth factor) Rv = 103,640 sf x 0.35 in/sf + 219,110 sf x 0.25 in/sf + 10,730 sf x 0.1 in/sf = 7,678 c.f.
- 2. Provided Recharge Volume
 - a. The proposed infiltration basin alone provides for 8,159 c.f. of storage volume below the lowest outlet (bottom of basin 704.00, orifice in outlet control structure at elevation 705.40), which is enough to meet the recharge volume requirement. According to the HydroCAD model, the basin infiltrates 18,906 c.f. during a two year storm event. The typical infiltrating rain garden between units provides for 265 c.f. of storage volume and provides for 840 c.f. of groundwater recharge during a two year storm event. Rain garden 2.1 provides for 3,865 c.f. of storage volume and provides for 10,643 c.f. of groundwater recharge during a two year storm event. Rain garden 3.1 provides for 1,156 c.f. of storage volume and provides for 3,456 c.f. of storage volume and provides for 7,326 c.f. of groundwater recharge during a two year storm event. Rain garden 4.1 provides for 1,920 c.f. of storage volume and provides for 4,076 c.f. of groundwater recharge during a two year storm event. Rain garden 5.1 provides for 1,526 c.f. of storage volume and provides for 4,076 c.f. of groundwater recharge during a two year storm event. Rain garden 5.1 provides for 1,526 c.f. of storage volume and provides for 994 c.f. of groundwater recharge during a two year storm event.
- 3. Drawdown within 72 hours:

T = 12 x Provided Recharge Volume / (Rawls Rate x Basin Bottom Area) Infiltration Basin T= 8.7 hours Rain Garden 5.1 T = 8.2 hours Rain Garden 4.1 T = 9.5 hours Rain Garden 4.2 T = 9.9 hours Rain Garden 3.1 T = 8.1 hours Rain Garden 2.1 T = 4.6 hours Typical between unit Rain Garden T= 4.4 hours

D. Standard 4 – Water Quality

Stormwater management systems must be designed to remove 80% of the average annual post construction load of Total Suspended Solids (TSS). This standard is met when:

- a. Suitable practices for source control and pollution prevention are identified in a long-term pollution prevention plan and thereafter implemented and maintained;
- b. Stormwater BMPs are sized to capture the required water quality volume determined in accordance with the Massachusetts Stormwater Handbook; and
- c. Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook.

Proposed Full Compliance:

TSS removal percentage computations are provided in Appendix F for the BMP treatment train as designed. There are several treatment trains created for the proposed drainage system:

- a. Runoff from pavement travels to an infiltrating rain garden with a sediment forebay and is discharged to a non-critical area. TSS removal equals 90%. The typical rain garden has an impervious area of 5,000 s.f. directed towards it, and therefore has a required water quality volume of 210 c.f. (0.5" water quality depth). During a two year storm event, the typical rain garden has an inflow of 1,150 c.f., therefore meeting the water quality volume requirement.
- b. Runoff from pavement travels to a lined rain garden with a sediment forebay and is discharged to a critical area (Vernal Pool). TSS removal equals 90%. The typical rain garden has an impervious area of 5,000 s.f. directed towards it, and therefore has a required water quality volume of 420 c.f. (1.0" water quality depth). During a two year storm event, the typical rain garden has an inflow of 1,150 c.f., therefore meeting the water quality volume requirement.
- c. Runoff from pavement travels to an infiltrating rain garden with (2) sediment forebays (44% pretreatment) and is discharged to a critical area (vernal pool). TSS removal equals 93%. The typical rain garden has an impervious area of 5,000 s.f. directed towards it, and therefore has a required water quality volume of 420 c.f. (1.0" water quality depth). During a two year storm event, the typical rain garden has an inflow of 1,150 c.f., therefore meeting the water quality volume requirement.
- d. Runoff from pavement travels to an infiltrating rain garden with a sediment forebay and is discharged to an additional infiltrating rain garden prior to discharge to a non-critical area. TSS removal equals 99%. The typical rain garden has an impervious area of 5,000 s.f. directed towards it, and therefore has a required water quality volume of 210 c.f. (0.5" water quality depth). During a two year storm event, the typical rain garden has an inflow of 1,150 c.f., therefore meeting the water quality volume requirement.
- e. Runoff from pavement travels to a lined rain garden with a sediment forebay and is discharged to an infiltrating rain garden prior to discharge to a non-critical area. TSS removal equals 99%. The typical rain garden has an impervious area of 5,000 s.f. directed towards it, and therefore has a required water quality volume of 210 c.f. (0.5" water quality depth). During a two year storm event, the typical rain garden has an inflow of 1,150 c.f., therefore meeting the water quality volume requirement.
- f. Runoff from pavement travels to a lined rain garden with a sediment forebay (90% pretreatment) and is discharged to an infiltrating rain garden prior to discharge to a critical area (vernal pool). TSS removal equals 99%. The typical rain garden has an impervious area of 5,000 s.f. directed towards it, and therefore has a required water quality volume of 420 c.f. (1.0" water quality depth). During a two year storm event, the typical rain garden has an inflow of 1,150 c.f., therefore meeting the water quality volume requirement.
- g. Runoff from pavement travels to a rain garden with a Rain Guardian pretreatment device and is discharged to a non-critical area. TSS removal equals 90%. Rain garden 2.1 has an impervious area of 32,675 s.f. directed towards it, and therefore has a required water quality volume of 1,360 c.f. (0.5" water quality depth). During a two year storm event, rain garden 2.1 has an inflow of 12,150 c.f., therefore meeting the water quality volume requirement.
- h. Runoff from pavement travels to a rain garden (lined or infiltrating) with a sediment forebay (90% pretreatment) and is discharged to an infiltration basin prior to discharge to a critical area (vernal pool). TSS removal equals 98%. The typical rain garden has an impervious area of 5,000 s.f. directed towards it, and therefore has a required water quality volume of 420 c.f. (1.0" water quality depth). During a two year storm event, the typical rain garden has an inflow of 1,150 c.f., therefore meeting the water quality volume requirement. The infiltration basin has an impervious area of 116,880 s.f. directed towards it, and therefore has a required water quality volume of 9,740 c.f. (1.0" water quality depth). During a two year storm event, the infiltration basin has an inflow of 9,740 c.f. (1.0" water quality depth). During a two year storm event, the infiltration basin has an inflow of 31,784 c.f., therefore meeting the water quality volume requirement.
- i. Runoff from pavement travels to an infiltration basin with a sediment forebay from a deep sump and hooded catch basin (44% pretreatment) prior to discharge to a critical area (vernal pool). TSS removal equals 85%. The deep sump catch basin has an impervious area of 6,100 s.f. directed towards it, and therefore has a required water quality volume of 510 c.f. (1.0" water quality depth).

"55+ Manufactured Housing Community", Lot 3 Berry Farms Road, Sturbridge, MA – Stormwater Management Report

During a two year storm event, the deep sump catch basin has an inflow of 1,776 c.f., therefore meeting the water quality volume requirement. The infiltration basin has an impervious area of 116,880 s.f. directed towards it, and therefore has a required water quality volume of 9,740 c.f. (1.0" water quality depth). During a two year storm event, the infiltration basin has an inflow of 31,784 c.f., therefore meeting the water quality volume requirement.

All discharges from pavement are treated to a minimum of 85% TSS removal and all discharges to or near a critical area (Vernal Pools) are treated for 44% pretreatment prior to infiltration.

Rain gardens are proposed with outlet control devices which include orifices above full soil depth, however these are for control of large storm flows. No water quality flows bypass any rain gardens therefore meeting the requirements of Standard 4 and providing adequate water quality treatment:

	High Orifice Elev	WQ Event Peak	High Orifice Storm Event	Discharges To	Pretreatment %	WQ Event Treat %
RG1	674.00	672.58	2 year	RG5.1	25%	90%
RG2	676.00	674.44	10 year	RG5.1	25%	90%
RG3	678.00	676.66	2 year	RG5.1	25%	90%
RG6	676.00	673.50	10 year	RG4.1	25%	90%
RG7	677.50	675.00	10 year	RG4.1	25%	90%
RG8	678.00	675.50	10 year	RG4.1	25%	90%
RG9	679.50	677.00	10 year	RG4.1	25%	90%
RG10	681.50	679.00	10 year	RG4.1	25%	90%
RG11	684.00	681.50	10 year	RG4.1	25%	90%
RG12	688.50	687.47	2 year	RG4.2	25%	90%
RG13	697.00	695.90	2 year	RG4.2	25%	90%
RG14	703.75	702.82	2 year	RG2.1	25%	90%
RG15L	709.25	709.00	2 year	GW & Inf. Basin	25%	90%
RG15H	718.00	717.60	2 year	GW & Inf. Basin	25%	90%
RG16	732.75	732.31	2 year	GW & Inf. Basin	25%	90%
RG17	734.50	732.90	10 year	Inf. Basin	25%	90%
RG18	732.00	730.40	10 year	Inf. Basin	25%	90%
RG19	728.50	726.90	10 year	Inf. Basin	25%	90%
RG20	726.50	724.90	10 year	Inf. Basin	25%	90%
RG21	727.25	727.05	2 year	Outfall VP E	44%	90%
RG21 RG22	727.25	726.75	10 year	Outfall VP E	44%	90%
RG22 RG23	728.75	727.25	10 year	Inf. Basin	25%	90%
RG23 RG24	733.25	731.75	10 year	Inf. Basin	25%	90%
RG24 RG25	734.25	732.75	10 year	Inf. Basin	25%	90%
RG25 RG26	746.00	744.50	2 year	Inf. Basin	25%	90 <i>%</i>
RG20 RG27L	750.50	749.00		Inf. Basin	25%	90%
RG27L RG27H	757.00		2 year	Inf. Basin	25%	90%
RG27H RG28H	756.25	756.30	2 year		25% 25%	90% 90%
RG28L	736.25 748.50	755.10 747.33	10 year	Inf. Basin Inf. Basin	25% 25%	90% 90%
RG20L RG29			2 year		25% 25%	90% 90%
	742.00	740.82	2 year	Inf. Basin		
RG30	730.00	728.52	10 year	Inf. Basin	25%	90%
RG31	731.00	730.10	10 year	RG2.1	25%	90%
RG32	733.50	731.60	10 year	RG2.1	25%	90%
RG33	735.50	733.60	10 year	RG2.1	25%	90%
RG34	738.50	736.60	10 year	RG2.1	25%	90%
RG35	740.00	738.10	10 year	RG2.1	25%	90%
RG36	739.00	737.10	10 year	Outfall Wet A	25%	90%
RG37	738.50	736.60	10 year	Outfall Wet A	25%	90%
RG38	736.50	734.60	10 year	Outfall Wet A	25%	90%
RG39	734.00	732.10	10 year	Outfall Wet A	25%	90%
RG40	732.00	731.00	2 year	Outfall North	25%	90%
RG41	731.00	730.40	2 year	Outfall North	25%	90%
RG42	731.00	730.34	2 year	GW & Outfall Wet		90%
RG43	732.50	731.80	2 year	GW & Outfall Wet		90%
RG44	736.00	735.30	2 year	GW & Outfall Wet		90%
RG45	737.50	736.80	2 year	GW & Outfall Wet		90%
RG46	739.00	738.30	2 year	GW & Outfall Wet		90%
RG47	739.00	738.30	2 year	GW & Outfall Wet	A 25%	90%

"55+ Manufactured Housing Community", Lot 3 Berry Farms Road, Sturbridge, MA – Stormwater Management Report

March 31, 202 Rev. November 10, 202 Rev. November 10, 202 Rev. April 28, 202 RG48 737.50 736.80 2 year GW & RG2.1 25% 90% RG49 734.50 733.30 10 year GW & RG2.1 25% 90% RG50 731.00 729.80 10 year GW & RG2.1 25% 90% RG51 694.00 692.97 2 year RG4.2 25% 90% RG52 695.50 694.40 10 year Outfall Wet A 25% 90% RG53 692.50 691.40 10 year Outfall Wet A 25% 90% RG54 689.00 687.90 10 year Outfall Wet A 25% 90%	nc.
Rev. April 28, 202 RG48 737.50 736.80 2 year GW & RG2.1 25% 90% RG49 734.50 733.30 10 year GW & RG2.1 25% 90% RG50 731.00 729.80 10 year GW & RG2.1 25% 90% RG51 694.00 692.97 2 year RG4.2 25% 90% RG52 695.50 694.40 10 year Outfall Wet A 25% 90% RG53 692.50 691.40 10 year Outfall Wet A 25% 90% RG54 689.00 687.90 10 year Outfall Wet A 25% 90%	
RG48737.50736.802 yearGW & RG2.125%90%RG49734.50733.3010 yearGW & RG2.125%90%RG50731.00729.8010 yearGW & RG2.125%90%RG51694.00692.972 yearRG4.225%90%RG52695.50694.4010 yearOutfall Wet A25%90%RG53692.50691.4010 yearOutfall Wet A25%90%RG54689.00687.9010 yearOutfall Wet A25%90%	
RG49734.50733.3010 yearGW & RG2.125%90%RG50731.00729.8010 yearGW & RG2.125%90%RG51694.00692.972 yearRG4.225%90%RG52695.50694.4010 yearOutfall Wet A25%90%RG53692.50691.4010 yearOutfall Wet A25%90%RG54689.00687.9010 yearOutfall Wet A25%90%	23
RG50731.00729.8010 yearGW & RG2.125%90%RG51694.00692.972 yearRG4.225%90%RG52695.50694.4010 yearOutfall Wet A25%90%RG53692.50691.4010 yearOutfall Wet A25%90%RG54689.00687.9010 yearOutfall Wet A25%90%	
RG51694.00692.972 yearRG4.225%90%RG52695.50694.4010 yearOutfall Wet A25%90%RG53692.50691.4010 yearOutfall Wet A25%90%RG54689.00687.9010 yearOutfall Wet A25%90%	
RG52695.50694.4010 yearOutfall Wet A25%90%RG53692.50691.4010 yearOutfall Wet A25%90%RG54689.00687.9010 yearOutfall Wet A25%90%	
RG53 692.50 691.40 10 year Outfall Wet A 25% 90% RG54 689.00 687.90 10 year Outfall Wet A 25% 90%	
RG54 689.00 687.90 10 year Outfall Wet A 25% 90%	
······································	
RG55 688.00 686.31 10 year Outfall Wet A 25% 90%	
RG57 689.25 688.85 2 year GW & RG4.2 25% 90%	
RG58 694.25 693.85 2 year GW & RG4.2 25% 90%	
RG59 689.75 689.35 2 year GW & RG4.2 25% 90%	
RG60 686.25 685.60 2 year GW & RG4.2 25% 90%	
RG61 683.25 682.60 2 year GW & RG4.1 25% 90%	
RG62 680.75 680.10 2 year GW & RG4.1 25% 90%	
RG63 678.25 677.60 2 year GW & RG4.1 25% 90%	
RG64 677.25 676.60 2 year GW & RG4.1 25% 90%	
RG65 677.25 675.80 2 year GW & RG4.1 25% 90%	
RG66 677.75 676.30 2 year GW & RG4.1 25% 90%	
RG69 677.75 676.26 10 year GW & Outfall VPA1 44% 90%	
RG70 673.75 673.21 2 year GW & Outfall VPA1 44% 90%	
RG2.1 701.75 699.62 10 year GW & Outfall Wet A 25%/90% 90%/99	%
RG3.1 686.00 684.02 10 year GW & Outfall VPA2 44% 90%	
RG4.1 671.25 671.11 2 year GW & Outfall Wet A 90% 99%	
RG4.2 678.25 678.08 2 year GW & Outfall Wet A 90% 99%	
RG5.1 671.75 671.39 2 year GW & Outfall VPA1 44%/90% 90%/99%	%

Per the EPA Region 1 BMP Performance Extrapolation Tool and the MA Stormwater Handbook, all treatment trains will also provide for a minimum 60% phosphorous removal as well.

The TSS removal computations are provided in Appendix F.

A "Long Term Operation and Maintenance Plan" is being provided as Appendix H.

E. Standard 5 – Land Uses with Higher Potential Pollutant Loads

For land uses with higher potential pollutant loads, source control and pollution prevention shall be implemented in accordance with the Massachusetts Storm water Handbook to eliminate or reduce the discharge of storm water runoff from such land uses to the maximum extent practicable. If through source control and/or pollution prevention, all land uses with higher potential pollutant loads cannot be completely protected from exposure to rain, snow, snow melt, or storm water runoff, the proponent shall use the specific storm water BMP's determined by the Department to be suitable for such use as provided in the Massachusetts Storm water Handbook.

Proposed Full Compliance:

• Not applicable - the Site is not a Land Use with High Potential Pollutant Loads.

F. Standard 6 – Critical Areas

Storm water discharges to a Zone II or Interim Wellhead Protection Area of a public water supply and storm water discharges near or any other critical area require the use of the specific storm water best management practices determined by the Department to be suitable for managing discharges to such area as provided in the Massachusetts Storm water Handbook.

Proposed Full Compliance:

• The site does discharge to or near critical areas: three on- and off-site vernal pools. All discharges from pavement are treated to a minimum of 85% TSS removal and all discharges to or near a critical area (vernal

pools) are treated for 44% pretreatment prior to infiltration. The 1" water quality depth was used to ensure the treatment BMPs treat the required water quality volumes.

G. <u>Standard 7 - Redevelopment</u>

A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable; Standard 2, Standard 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions.

Proposed Full Compliance:

• The Site is not considered a redevelopment, and all of the standards will be fully met.

H. Standard 8 – Construction Period Controls

A plan to control construction related impacts including erosion sedimentation and other pollution prevention sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) must be implemented.

Proposed Full Compliance:

- Draft Weekly Construction Period Inspection Report is provided as Appendix G.
- Project will disturb > 1 Acre, therefore an EPA–NPDES Stormwater General Permit is required.
- The construction period erosion and sedimentation controls are outlined on the referenced site plans along with the sequence for implementation and construction phasing.

I. Standard 9 – Operation and Maintenance Plan

A long term operation and maintenance plan must be developed and implemented to ensure that storm water management systems function as designed.

Proposed Full Compliance:

• Long Term Operation and Maintenance Plan is included in the Stormwater Management Report, Appendix H.

J. <u>Standard 10 – Illicit Discharges to Drainage System</u>

All illicit discharges to the stormwater management system are prohibited.

Proposed Full Compliance:

• The Long Term Operation and Maintenance Plan provided in Appendix H addresses illicit discharges to drainage system and includes an Illicit Discharge Compliance Statement signed by the applicant.

Tables No. 1-8 provide a summary of off-site Pre- and Post-Development peak runoff flow rates and volumes.

Appendix A includes a copy of the "MA-DEP Checklist for Stormwater Report".

Appendix B & C includes maps and information regarding rainfall data and soils for the site.

Appendix D & E includes the complete Pre-Development and Post-Development *HydroCAD* drainage calculation reports figures for your review.

Appendix F provides additional stormwater calculations relating to compliance with the MA Stormwater Management Standards

Appendix G provides a DRAFT "Weekly Construction Period Inspection Report"

Appendix H provides a "Long Term Stormwater Operation & Maintenance Plan"

The "Special Permit and Site Plan, Blueberry Hill Estates, 55+ Manufactured Housing Community, Lot 3 Berry Farms Road, Sturbridge, MA" Plan Set prepared by McClure Engineering, Inc., dated 4/1/22, Rev 11/10/22 provides details of the complete stormwater management system design.

McClure Engineering, Inc. March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023

APPENDIX A

MA-DEP STORMWATER CHECKLIST

"55+ Manufactured Housing Community", Lot 3 Berry Farms Road, Sturbridge, MA – Stormwater Management Report



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

A Stormwater Report must be submitted with the Notice of Intent permit application to document

compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for

the Stormwater Report (which should provide more substantive and detailed information) but is offered

Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist.

Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and

the Stormwater Report must contain the engineering computations and supporting information set forth in

here as a tool to help the applicant organize their Stormwater Management documentation for their

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report

certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Longterm Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.



Registered Professional Engineer Block and Signature

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

New development

Redevelopment

Mix of New Development and Redevelopment



LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- No disturbance to any Wetland Resource Areas
- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
 - Credit 1
 - Credit 2
 - Credit 3
- Use of "country drainage" versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
- Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- □ Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe):

Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- Calculations provided to show that post-development peak discharge rates do not exceed predevelopment rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24hour storm.

Standard 3: Recharge

- Soil Analysis provided.
- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.

\boxtimes	Static
-------------	--------

Simple Dynamic Dynamic Field¹

- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - Site is comprised solely of C and D soils and/or bedrock at the land surface
 - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - Solid Waste Landfill pursuant to 310 CMR 19.000
 - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- · Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- · Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
- Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - is within the Zone II or Interim Wellhead Protection Area
 - \boxtimes is near or to other critical areas
 - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - involves runoff from land uses with higher potential pollutant loads.
- The Required Water Quality Volume is reduced through use of the LID site Design Credits.
- Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist (continued)
Standard 4: Water Quality (continued)
The BMP is sized (and calculations provided) based on:
The ½" or 1" Water Quality Volume or
The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.
Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)
 The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report. The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted <i>prior</i> <i>to</i> the discharge of stormwater to the post-construction stormwater BMPs.
The NPDES Multi-Sector General Permit does <i>not</i> cover the land use.
LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
All exposure has been eliminated.
All exposure has <i>not</i> been eliminated and all BMPs selected are on MassDEP LUHPPL list.
The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.
Standard 6: Critical Areas
The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
Critical areas and BMPs are identified in the Stormwater Report.



Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The project is subject to the Stormwater Management Standards only to the maximum Exten	t
Practicable as a:	

Limited Project

Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.

Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area

Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff

Bike Path and/or Foot Path

Redevelopment Project

Redevelopment portion of mix of new and redevelopment.

Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has *not* been included in the Stormwater Report but will be submitted *before* land disturbance begins.
- The project is *not* covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - Name of the stormwater management system owners;
 - Party responsible for operation and maintenance;
 - Schedule for implementation of routine and non-routine maintenance tasks;
 - Plan showing the location of all stormwater BMPs maintenance access areas;
 - Description and delineation of public safety features;
 - Estimated operation and maintenance budget; and
 - Operation and Maintenance Log Form.
- The responsible party is *not* the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

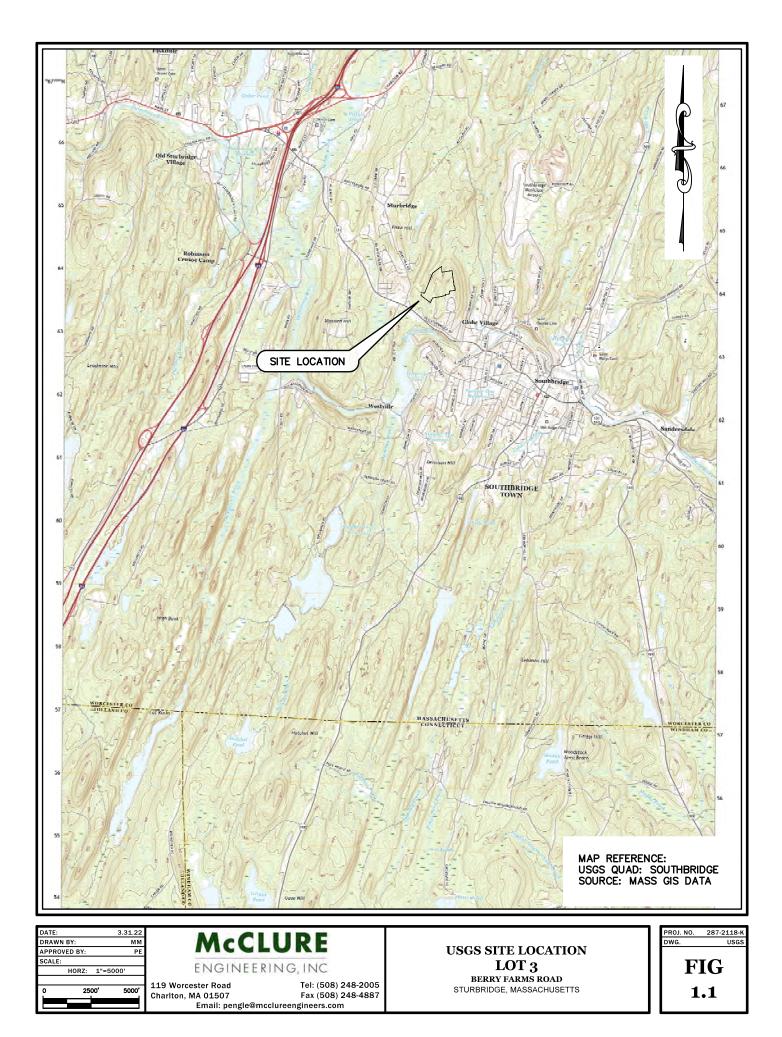
- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted *prior to* the discharge of any stormwater to post-construction BMPs.

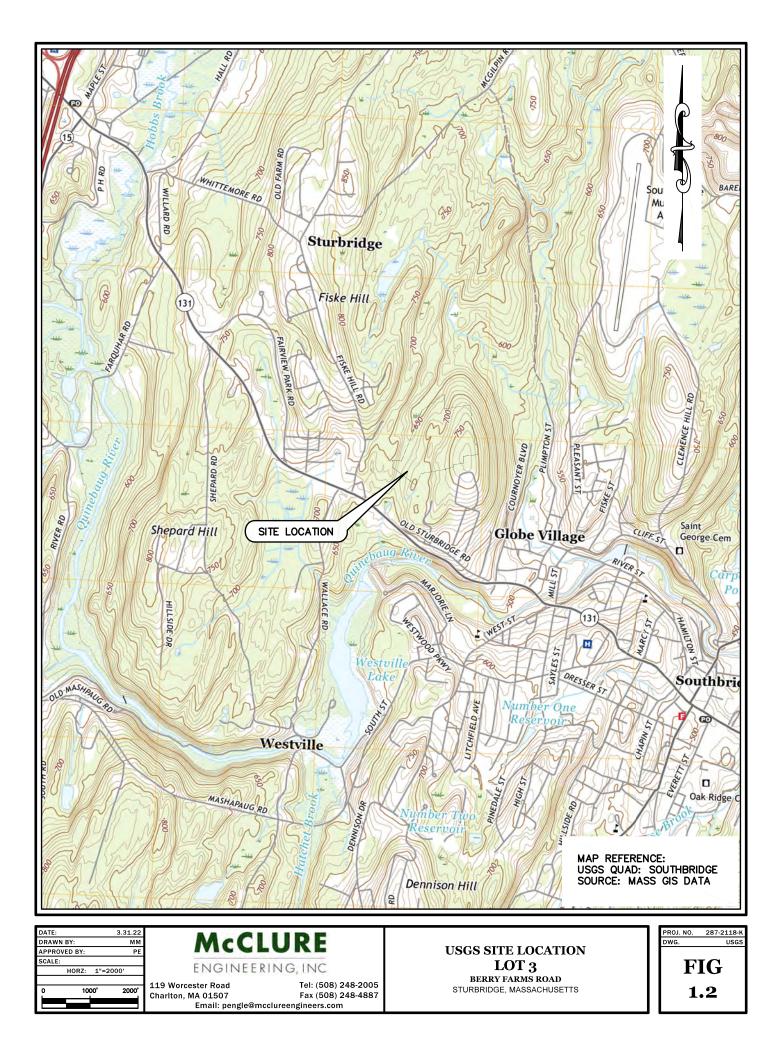
McClure Engineering, Inc. March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023

APPENDIX B

USGS – Figure 1

"55+ Manufactured Housing Community", Lot 3 Berry Farms Road, Sturbridge, MA – Stormwater Management Report





McClure Engineering, Inc. March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023

APPENDIX C

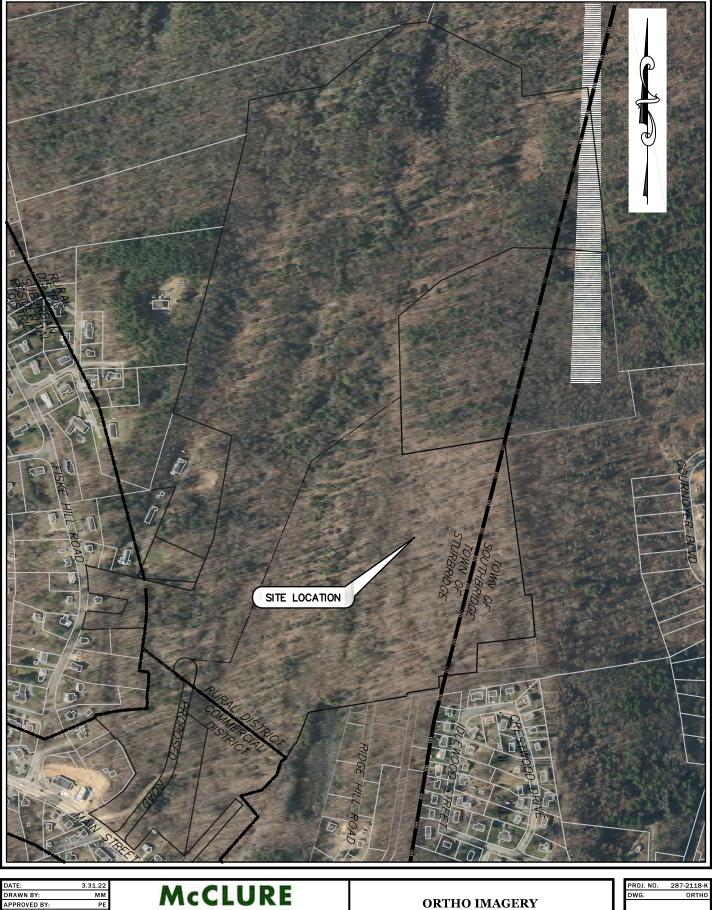
FEMA - FLOOD PLAIN MAPPING

NCRS SOIL MAPPING

ON-SITE SOIL TESTING LOGS

RAWLS TABLE

NOAA PRECIPITATION FREQUENCY ESTIMATES



DATE:	3.	31.22	M
DRAWN BY:		MM	MACL
APPROVED BY:		PE	
SCALE:			ENIQUEIE
HORZ:	1"=500'		ENGINEE
			110 Waxaatay Daad
0 2	50	500'	119 Worcester Road
			Charlton, MA 01507
			Email: pengle@



ORTHO IMAGERY LOT 3 BERRY FARMS ROAD STURBRIDGE, MASSACHUSETTS



National Flood Hazard Layer FIRMette



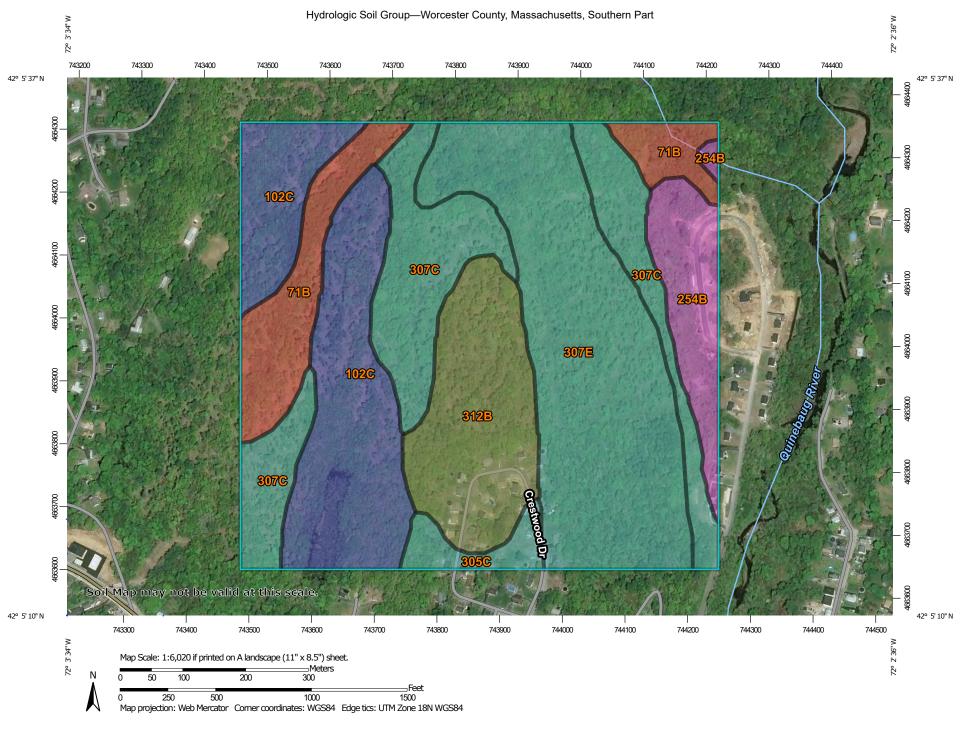
Legend

72°3'27"W 42°5'37"N SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT Without Base Flood Elevation (BFE) Zone A. V. A9 With BFE or Depth Zone AE, AO, AH, VE, AR SPECIAL FLOOD HAZARD AREAS **Regulatory Floodway** 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X Future Conditions 1% Annual Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X OTHER AREAS OF FLOOD HAZARD Area with Flood Risk due to Levee Zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs OTHER AREAS Area of Undetermined Flood Hazard Zone D - - - - Channel, Culvert, or Storm Sewer GENERAL STRUCTURES LIIII Levee, Dike, or Floodwall TOWN OF SOUTHBRIDGE 20.2 Cross Sections with 1% Annual Chance TOWN OF STURBRIDGE 250334 17.5 Water Surface Elevation 250337 AREA OF MINIMAL FLOOD HAZARD **Coastal Transect** Base Flood Elevation Line (BFE) Limit of Study Jurisdiction Boundary TOWNOF SOUTHBRIDGE **Coastal Transect Baseline** _ ----250334 OTHER **Profile Baseline** 25027C0933E FEATURES Hydrographic Feature eff. 7/4/2011 **Digital Data Available** No Digital Data Available MAP PANELS Unmapped The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location. This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/31/2022 at 10:34 AM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for 72°2'50"W 42°5'10"N Feet 1:6.000 unmapped and unmodernized areas cannot be used for regulatory purposes. 250 500 1,000 1.500 2.000

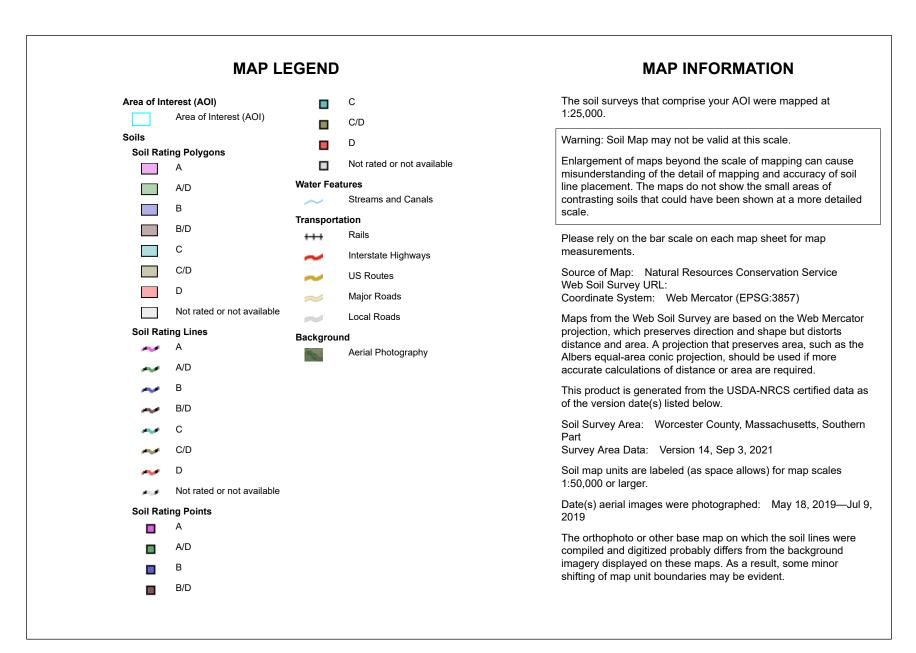
Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

MassGIS Soil Map





USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey





Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
71B	1B Ridgebury fine sandy loam, 3 to 8 percent slopes, extremely stony		12.8	9.5%
102C	Chatfield-Hollis-Rock outcrop complex, 0 to 15 percent slopes	В	28.2	20.9%
254B	Merrimac fine sandy loam, 3 to 8 percent slopes	A	9.1	6.7%
305C	Paxton fine sandy loam, 8 to 15 percent slopes	С	3.2	2.4%
307C	Paxton fine sandy loam, 8 to 15 percent slopes, extremely stony	С	25.3	18.8%
307E	Paxton fine sandy loam, 15 to 35 percent slopes, extremely stony	C	38.0	28.3%
312B	Woodbridge fine sandy loam, 0 to 8 percent slopes, extremely stony	C/D	17.9	13.3%
Totals for Area of Inter	rest		134.5	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

Table 2.3.3. 1982 Rawls Rates¹⁸

Texture Class	NRCS Hydrologic Soil Group (HSG)	Infiltration Rate Inches/Hour			
Sand	A	8.27			
Loamy Sand	A	2.41			
Sandy Loam	В	1.02			
Loam	В	0.52			
Silt Loam	С	0.27			
Sandy Clay Loam	С	0.17			
Clay Loam	D	0.09			
Silty Clay Loam	D	0.06			
Sandy Clay	D	0.05			
Silty Clay	D	0.04			
Clay	D	0.02			

 ¹⁸ Rawls, Brakensiek and Saxton, 1982
 Volume 3: Documenting Compliance with the Massachusetts Stormwater Management Standards



NOAA Atlas 14, Volume 10, Version 3 Location name: Sturbridge, Massachusetts, USA* Latitude: 42.091°, Longitude: -72.0529° Elevation: 691.19 ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

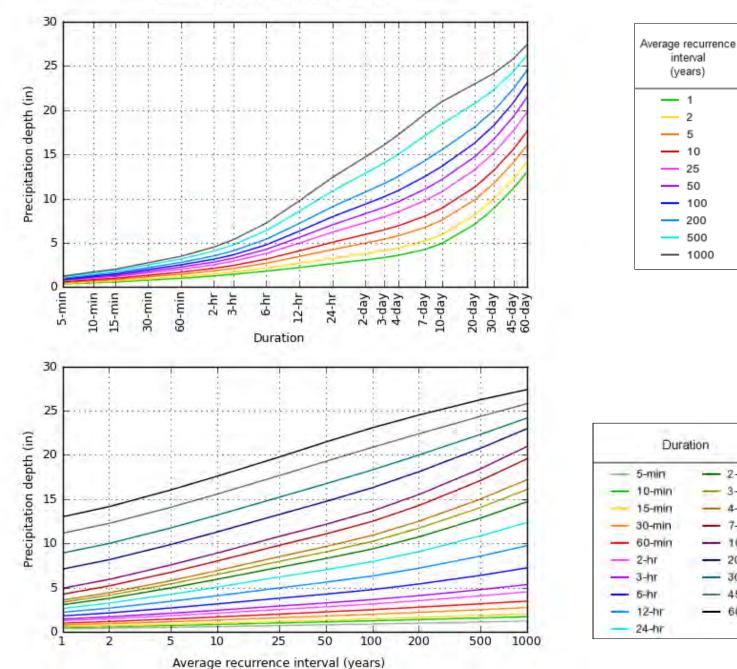
PDS-	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹									
Duration				Average	recurrence	interval (ye	ars)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.338 (0.263-0.428)	0.399 (0.310-0.506)	0.499 (0.386-0.635)	0.582 (0.448-0.745)	0.696 (0.518-0.930)	0.782 (0.570-1.07)	0.871 (0.616-1.24)	0.967 (0.652-1.41)	1.10 (0.714-1.67)	1.21 (0.763-1.87)
10-min	0.479 (0.373-0.607)	0.566 (0.440-0.717)	0.707 (0.547-0.899)	0.824 (0.635-1.06)	0.985 (0.734-1.32)	1.11 (0.808-1.51)	1.23 (0.872-1.75)	1.37 (0.923-2.00)	1.56 (1.01-2.37)	1.71 (1.08-2.65)
15-min	0.564 (0.439-0.714)	0.665 (0.517-0.843)	0.831 (0.643-1.06)	0.969 (0.747-1.24)	1.16 (0.864-1.55)	1.30 (0.951-1.78)	1.45 (1.03-2.06)	1.61 (1.09-2.36)	1.83 (1.19-2.78)	2.01 (1.27-3.12)
30-min	0.768 (0.597-0.972)	0.906 (0.704-1.15)	1.13 (0.877-1.44)	1.32 (1.02-1.69)	1.58 (1.18-2.11)	1.77 (1.29-2.43)	1.98 (1.40-2.80)	2.19 (1.48-3.21)	2.50 (1.62-3.79)	2.74 (1.73-4.25)
60-min	0.971 (0.756-1.23)	1.15 (0.891-1.45)	1.43 (1.11-1.82)	1.67 (1.29-2.14)	2.00 (1.49-2.67)	2.25 (1.64-3.07)	2.50 (1.77-3.55)	2.78 (1.87-4.06)	3.16 (2.05-4.80)	3.46 (2.19-5.37)
2-hr	1.25 (0.976-1.57)	1.46 (1.14-1.84)	1.82 (1.42-2.30)	2.11 (1.64-2.69)	2.52 (1.89-3.36)	2.82 (2.08-3.85)	3.14 (2.25-4.47)	3.51 (2.37-5.11)	4.06 (2.64-6.13)	4.52 (2.87-6.98)
3-hr	1.43 (1.13-1.79)	1.68 (1.32-2.11)	2.09 (1.64-2.64)	2.44 (1.90-3.09)	2.91 (2.19-3.87)	3.25 (2.41-4.44)	3.63 (2.62-5.18)	4.08 (2.76-5.92)	4.76 (3.10-7.17)	5.34 (3.40-8.22)
6-hr	1.79 (1.42-2.23)	2.13 (1.69-2.66)	2.69 (2.12-3.36)	3.15 (2.47-3.96)	3.79 (2.88-5.02)	4.25 (3.18-5.79)	4.76 (3.47-6.79)	5.40 (3.67-7.79)	6.38 (4.17-9.56)	7.23 (4.61-11.1)
12-hr	2.20 (1.76-2.72)	2.67 (2.13-3.31)	3.44 (2.73-4.27)	4.08 (3.21-5.09)	4.95 (3.79-6.54)	5.60 (4.21-7.59)	6.31 (4.62-8.96)	7.18 (4.90-10.3)	8.55 (5.60-12.7)	9.74 (6.23-14.8)
24-hr	2.63 (2.11-3.23)	3.24 (2.60-3.98)	4.23 (3.38-5.22)	5.05 (4.01-6.27)	6.18 (4.76-8.12)	7.02 (5.30-9.46)	7.93 (5.84-11.2)	9.07 (6.21-12.9)	10.8 (7.12-16.1)	12.4 (7.94-18.7)
2-day	3.05 (2.47-3.72)	3.78 (3.05-4.61)	4.96 (3.99-6.07)	5.94 (4.75-7.32)	7.28 (5.65-9.50)	8.28 (6.29-11.1)	9.36 (6.94-13.2)	10.7 (7.37-15.2)	12.8 (8.47-18.9)	14.7 (9.46-22.1)
3-day	3.33 (2.71-4.05)	4.12 (3.34-5.01)	5.41 (4.37-6.60)	6.48 (5.20-7.96)	7.95 (6.19-10.3)	9.03 (6.89-12.1)	10.2 (7.60-14.3)	11.7 (8.07-16.6)	14.1 (9.29-20.6)	16.1 (10.4-24.1)
4-day	3.57 (2.91-4.32)	4.41 (3.59-5.35)	5.78 (4.68-7.03)	6.92 (5.57-8.47)	8.48 (6.62-11.0)	9.63 (7.37-12.8)	10.9 (8.12-15.2)	12.5 (8.62-17.6)	15.0 (9.93-22.0)	17.2 (11.1-25.7)
7-day	4.24 (3.47-5.10)	5.18 (4.24-6.25)	6.72 (5.48-8.14)	8.00 (6.48-9.75)	9.76 (7.66-12.6)	11.1 (8.50-14.7)	12.5 (9.34-17.4)	14.3 (9.89-20.0)	17.1 (11.4-24.9)	19.6 (12.7-29.1)
10-day	4.92 (4.04-5.90)	5.92 (4.86-7.11)	7.55 (6.17-9.10)	8.90 (7.23-10.8)	10.8 (8.47-13.8)	12.1 (9.35-16.0)	13.6 (10.2-18.9)	15.5 (10.8-21.7)	18.4 (12.3-26.8)	21.0 (13.6-31.1)
20-day	7.09 (5.87-8.44)	8.14 (6.73-9.71)	9.86 (8.12-11.8)	11.3 (9.24-13.6)	13.3 (10.5-16.8)	14.7 (11.3-19.1)	16.3 (12.1-22.0)	18.1 (12.7-25.1)	20.8 (13.9-29.9)	23.0 (14.9-33.9)
30-day	8.90 (7.40-10.6)	9.98 (8.29-11.9)	11.7 (9.71-14.0)	13.2 (10.8-15.8)	15.2 (12.0-19.1)	16.7 (12.9-21.5)	18.3 (13.6-24.4)	20.0 (14.0-27.6)	22.3 (15.0-32.1)	24.2 (15.8-35.6)
45-day	11.1 (9.31-13.2)	12.3 (10.2-14.5)	14.1 (11.7-16.7)	15.6 (12.9-18.6)	17.7 (14.0-21.9)	19.3 (14.8-24.5)	20.9 (15.4-27.4)	22.4 (15.8-30.7)	24.4 (16.4-34.8)	25.8 (16.8-37.8)
60-day	13.0 (10.9-15.3)	14.1 (11.8-16.7)	16.0 (13.4-19.0)	17.6 (14.6-21.0)	19.8 (15.7-24.4)	21.5 (16.5-27.1)	23.1 (17.0-30.1)	24.5 (17.3-33.5)	26.2 (17.7-37.4)	27.4 (17.9-40.1)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

Back to Top



PDS-based depth-duration-frequency (DDF) curves Latitude: 42.0910°, Longitude: -72.0529°

NOAA Atlas 14, Volume 10, Version 3

Created (GMT): Thu Dec 9 18:26:44 2021

1

2 5 10

> 25 50

100 200 500

2-day

3-day

- 4-day

7-day

10-day

20-day

30-day

45-day

- 60-day

Back to Top

Maps & aerials

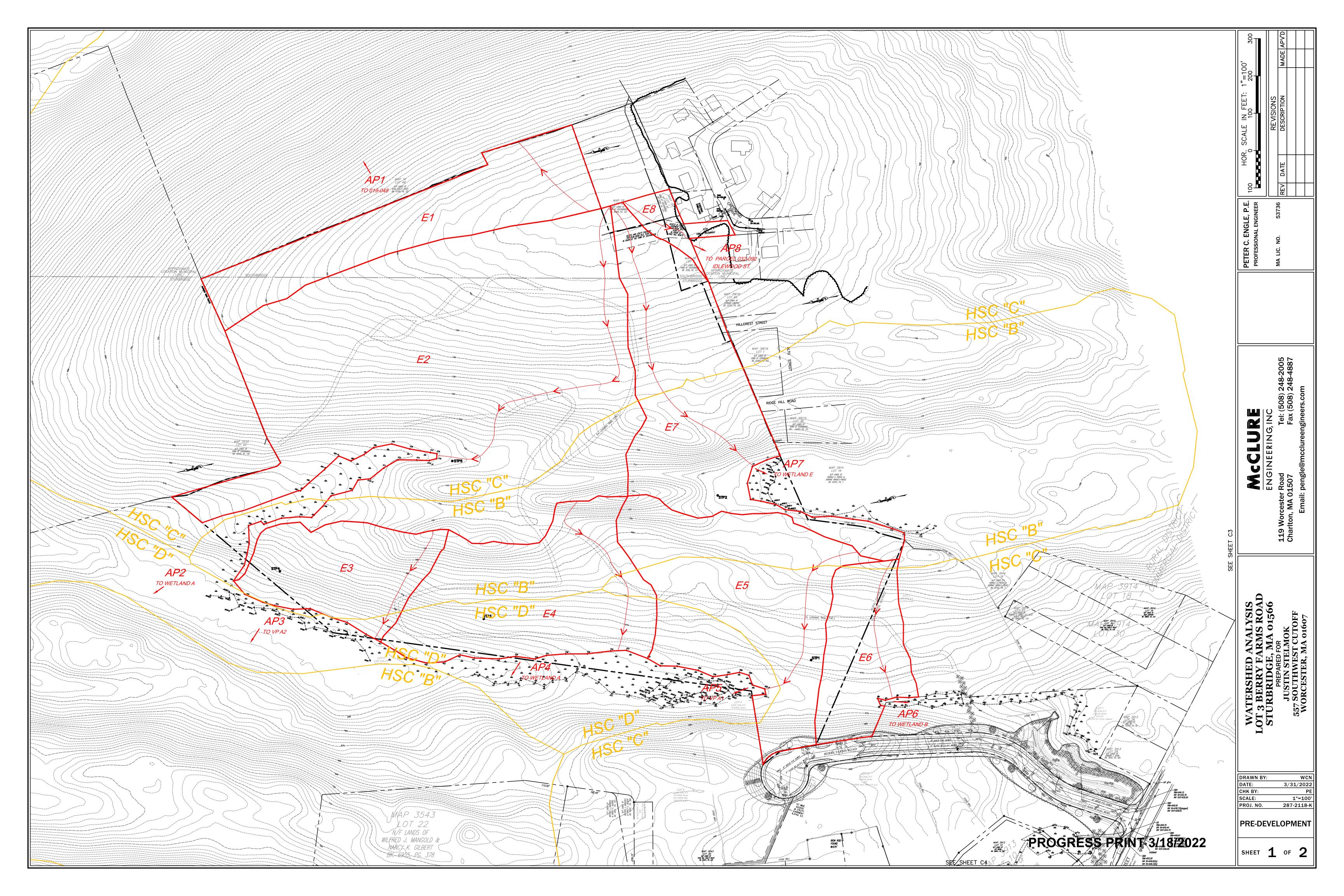
Small scale terrain

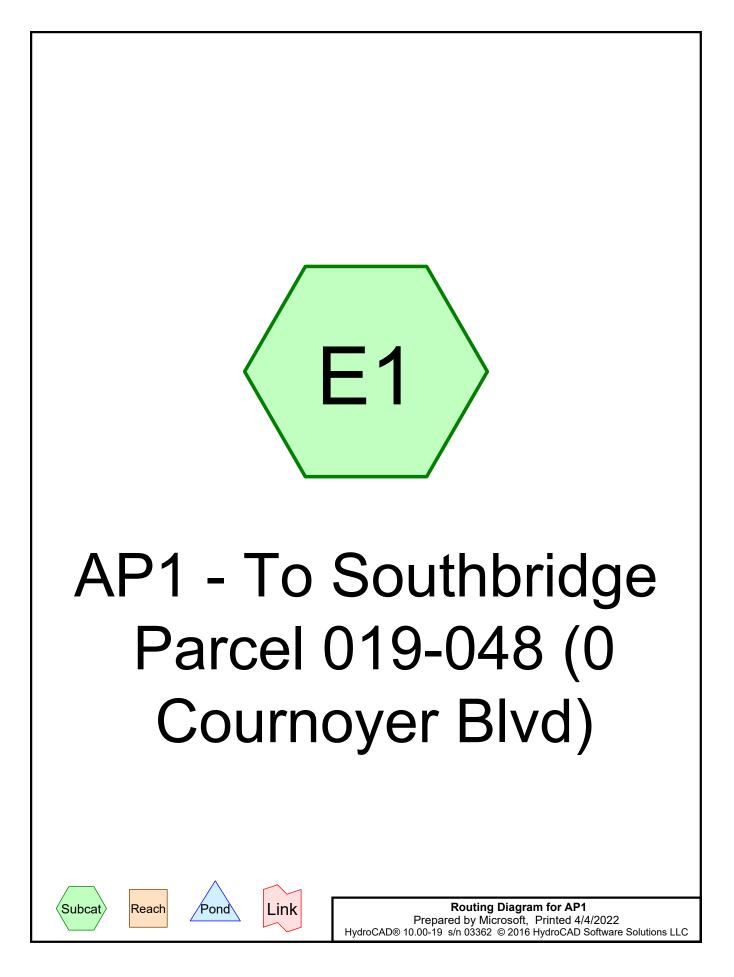
McClure Engineering, Inc. March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023

APPENDIX D

PRE-DEVELOPMENT HYDROCAD DRAINAGE CALCULATIONS

"55+ Manufactured Housing Community", Lot 3 Berry Farms Road, Sturbridge, MA – Stormwater Management Report





Runoff = 2.58 cfs @ 12.23 hrs, Volume= 11,834 cf, Depth> 0.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

_	A	rea (sf)	CN E	Description		
	1	67,515	70 V	Voods, Go	od, HSG C	
	167,515 100.00% Pervious Area				ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.7	50	0.0200	0.07		Sheet Flow,
	2.4	220	0.0950	1.54		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
_	15.1	270	Total			

Runoff = 6.90 cfs @ 12.22 hrs, Volume= 28,841 cf, Depth> 2.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN [Description		
	1	67,515	70 \	Noods, Go	od, HSG C	
	167,515 100.00% Pervious Area					a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.7	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
	2.4	220	0.0950	1.54		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
-	15.1	270	Total			

Runoff = 9.96 cfs @ 12.21 hrs, Volume= 41,026 cf, Depth> 2.94"

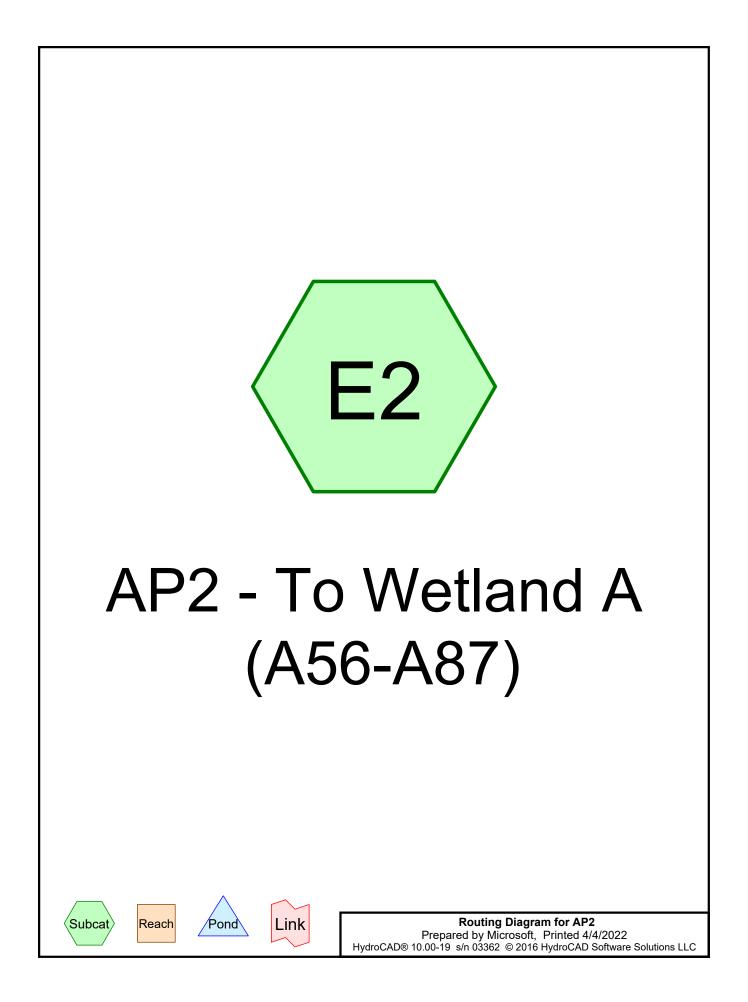
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

_	A	rea (sf)	CN E	Description		
	1	67,515	70 V	Voods, Go	od, HSG C	
	167,515 100.00% Pervious Area				ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.7	50	0.0200	0.07		Sheet Flow,
	2.4	220	0.0950	1.54		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
_	15.1	270	Total			

Runoff = 15.00 cfs @ 12.20 hrs, Volume= 61,293 cf, Depth> 4.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

_	A	rea (sf)	CN [Description		
	1	67,515	70 \	Noods, Go	od, HSG C	
	167,515 100.00% Pervious Area					a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.7	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
	2.4	220	0.0950	1.54		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
-	15.1	270	Total			



Runoff = 7.95 cfs @ 12.38 hrs, Volume= 45,298 cf, Depth> 0.75"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN D	escription		
	513,900		,	od, HSG C	
1	11,680	55 V	Voods, Go	od, HSG B	
7	25,580	68 V	Veighted A	verage	
7	25,580	1	00.00% Pe	ervious Are	а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
9.3	740	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.8	100	0.1700	2.06		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.8	60	0.0600	1.22		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
23.6	950	Total			

Runoff = 22.72 cfs @ 12.35 hrs, Volume= 115,158 cf, Depth> 1.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN D	escription		
	513,900		,	od, HSG C	
1	11,680	55 V	Voods, Go	od, HSG B	
7	25,580	68 V	Veighted A	verage	
7	25,580	1	00.00% Pe	ervious Are	а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
9.3	740	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.8	100	0.1700	2.06		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.8	60	0.0600	1.22		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
23.6	950	Total			

Runoff = 33.38 cfs @ 12.34 hrs, Volume= 166,010 cf, Depth> 2.75"

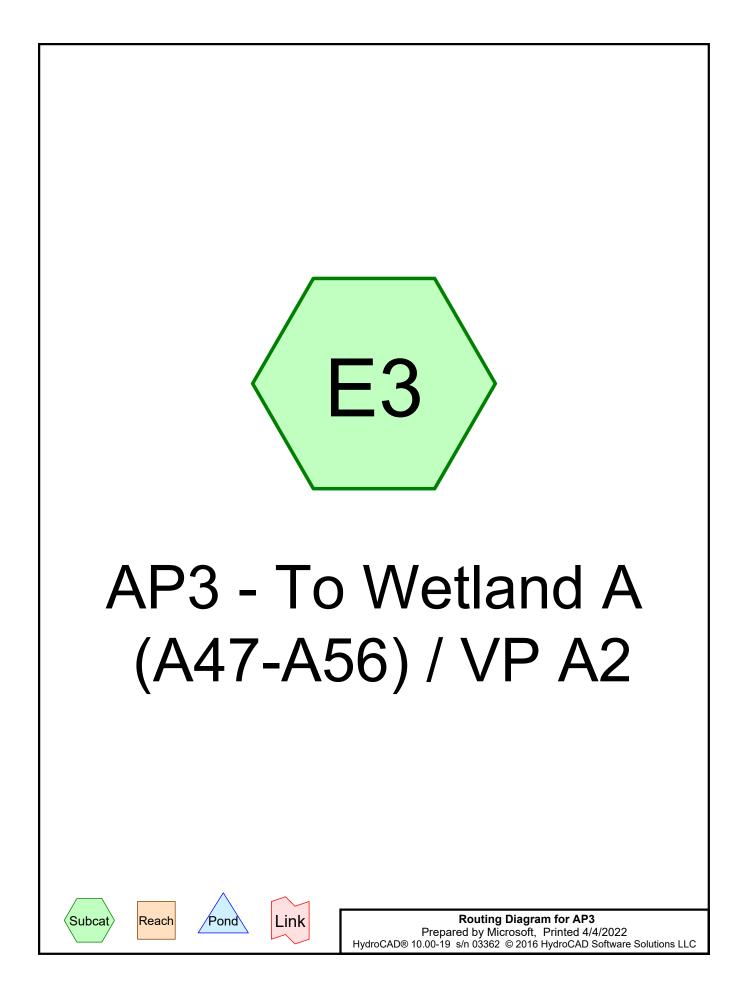
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Α	rea (sf)	CN D	escription		
6	13,900	70 V	loods, Go	od, HSG C	
1	11,680	55 V	Voods, Go	od, HSG B	
7	25,580	68 V	Veighted A	verage	
7	25,580	1	00.00% Pe	ervious Are	а
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
9.3	740	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.8	100	0.1700	2.06		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.8	60	0.0600	1.22		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
23.6	950	Total			

Runoff = 51.05 cfs @ 12.33 hrs, Volume= 251,343 cf, Depth> 4.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN D	escription		
	513,900		,	od, HSG C	
1	11,680	<u>55</u> V	Voods, Go	od, HSG B	
7	25,580		Veighted A		
7	25,580	1	00.00% Pe	ervious Are	а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
9.3	740	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.8	100	0.1700	2.06		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.8	60	0.0600	1.22		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
23.6	950	Total			



Runoff = 0.34 cfs @ 12.33 hrs, Volume= 2,780 cf, Depth> 0.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

_	A	rea (sf)	CN I	Description		
108,890 55 Woods, Good, HSG B				Woods, Go	od, HSG B	
_		5,640	77 \	Woods, Go	od, HSG D	
114,53056Weighted Average114,530100.00% Pervious Area						а
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	5.7	50	0.1500	0.15		Sheet Flow,
	2.1	280	0.2000	2.24		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	7.8	330	Total			

Runoff = 2.55 cfs @ 12.13 hrs, Volume= 10,163 cf, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN I	Description		
108,890 55 Woods, Good, HSG B				,	,	
_		5,640	77 \	Woods, Go	od, HSG D	
114,530 56 Weighted Average						
	114,530 100.00% Pervious Area				ervious Are	а
_	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
	5.7	50	0.1500	0.15		Sheet Flow,
	2.1	280	0.2000	2.24		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	7.8	330	Total			

Runoff = 4.48 cfs @ 12.12 hrs, Volume= 16,226 cf, Depth> 1.70"

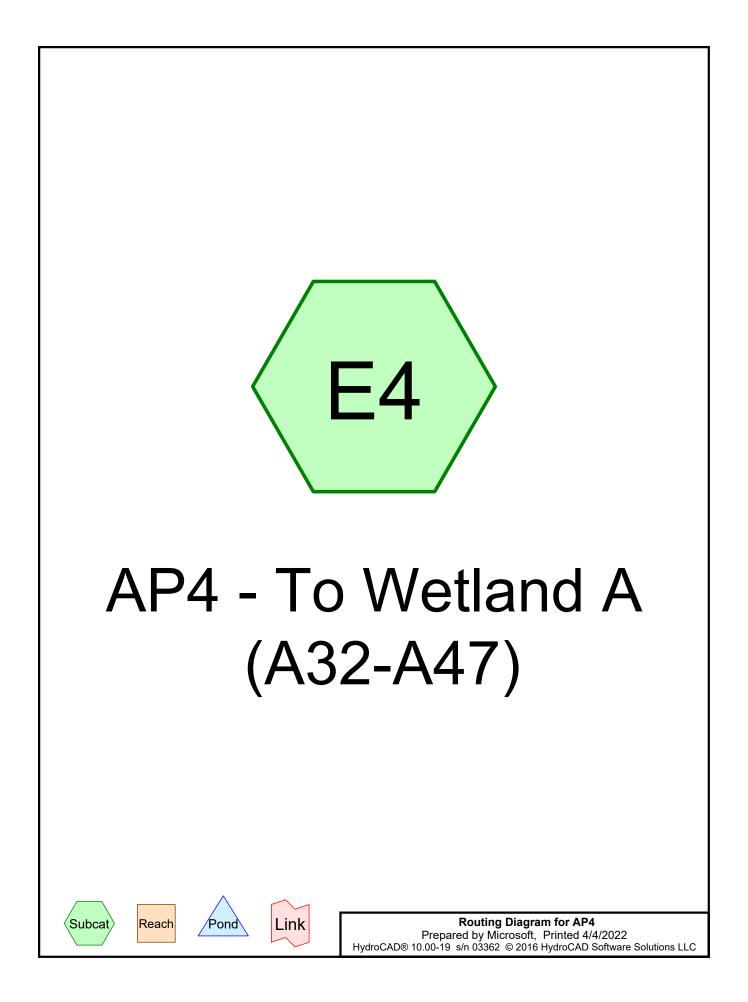
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

_	A	rea (sf)	CN I	Description		
108,890 55 Woods, Good, HSG B				Woods, Go	od, HSG B	
_		5,640	77 \	Woods, Go	od, HSG D	
114,53056Weighted Average114,530100.00% Pervious Area						a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	5.7	50	0.1500	0.15		Sheet Flow,
	2.1	280	0.2000	2.24		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	7.8	330	Total			

Runoff = 7.93 cfs @ 12.12 hrs, Volume= 27,093 cf, Depth> 2.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

_	A	rea (sf)	CN I	Description		
108,890 55 Woods, Good, HSG B				Woods, Go	od, HSG B	
_		5,640	77 \	Woods, Go	od, HSG D	
114,53056Weighted Average114,530100.00% Pervious Area						a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	5.7	50	0.1500	0.15		Sheet Flow,
	2.1	280	0.2000	2.24		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	7.8	330	Total			



Runoff = 3.07 cfs @ 12.15 hrs, Volume= 12,702 cf, Depth= 0.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

	A	rea (sf)	CN [Description		
	1	00,155	55 \	Voods, Go	od, HSG B	
		6,695			od, HSG C	
_	1	08,390	77 \	Voods, Go	od, HSG D	
215,240 67 Weighted Average			Veighted A	verage		
	2	15,240		00.00% Pe	ervious Are	а
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.1	50	0.3500	0.21		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	5.1	435	0.0800	1.41		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	9.2	485	Total			

Runoff = 9.20 cfs @ 12.14 hrs, Volume= 32,966 cf, Depth= 1.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

	A	rea (sf)	CN [Description		
	1	00,155	55 \	Voods, Go	od, HSG B	
		6,695	70 \	Voods, Go	od, HSG C	
	1	08,390	77 \	Voods, Go	od, HSG D	
215,240 67 Weighted Average				Veighted A	verage	
215,240 100.00% Pervious Area				00.00% Pe	ervious Are	а
	Тс	Length	Slope	Velocity	Capacity	Description
<u>(m</u>	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4	4.1	50	0.3500	0.21		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
Ę	5.1	435	0.0800	1.41		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
Q	9.2	485	Total			

Runoff = 13.66 cfs @ 12.13 hrs, Volume= 47,831 cf, Depth= 2.67"

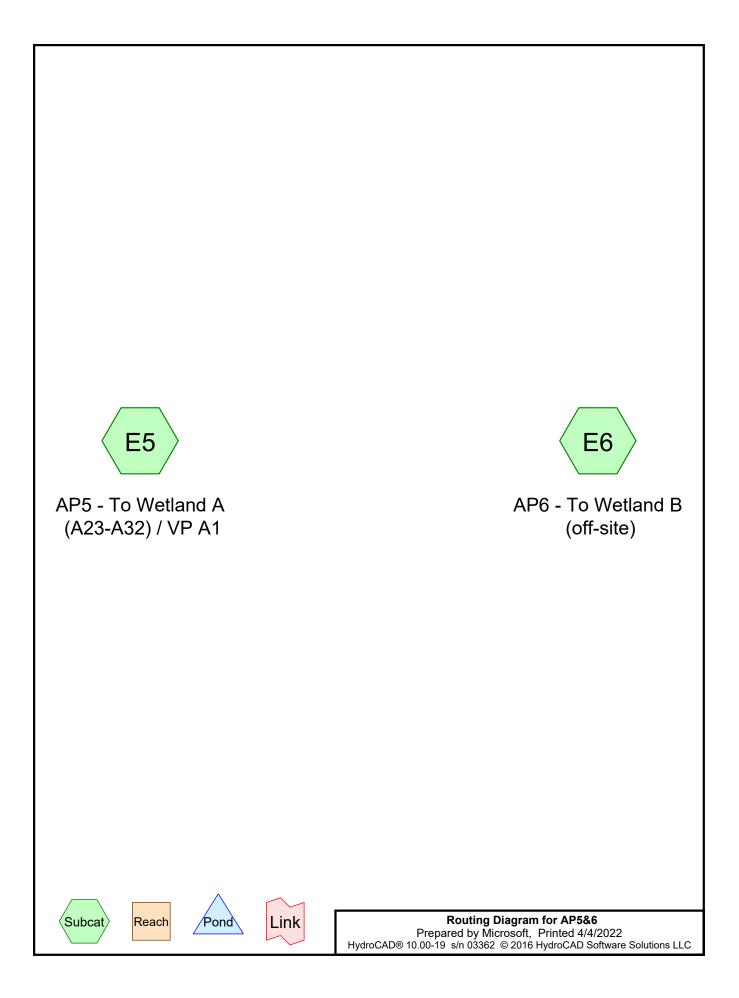
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

	A	rea (sf)	CN [Description		
	1	00,155	55 \	Voods, Go	od, HSG B	
		6,695			od, HSG C	
_	1	08,390	77 \	Voods, Go	od, HSG D	
215,240 67 Weighted Average			Veighted A	verage		
	2	15,240		00.00% Pe	ervious Are	а
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.1	50	0.3500	0.21		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	5.1	435	0.0800	1.41		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	9.2	485	Total			

Runoff = 21.07 cfs @ 12.13 hrs, Volume= 72,881 cf, Depth= 4.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	A	rea (sf)	CN [Description		
	1	00,155	55 \	Noods, Go	od, HSG B	
6,695 70 Woods, Good, HSG C						
	1	08,390	77 \	Noods, Go	od, HSG D	
215,240 67 Weighted Average				Neighted A	verage	
215,240 100.00% Pervious Area					ervious Are	a
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	4.1	50	0.3500	0.21		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	5.1	435	0.0800	1.41		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	9.2	485	Total			



Runoff = 2.87 cfs @ 12.27 hrs, Volume= 14,691 cf, Depth> 0.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN D	Description		
	80,545 55 Woods, Good, HSG B				
	121,865 70 Woods, Good, HSG C				
	47,880	77 V	Voods, Go	od, HSG D	
2	50,290		Veighted A		
2	50,290	1	00.00% Pe	ervious Area	а
-		01		A	
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.8	50	0.0300	0.08		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.9	115	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	30	0.3500	2.96		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.9	310	0.0700	1.32		Shallow Concentrated Flow,
3.9	310	0.0700	1.32		

Summary for Subcatchment E6: AP6 - To Wetland B (off-site)

Runoff = 1.57 cfs @ 12.15 hrs, Volume= 6,195 cf, Depth> 0.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

	Α	rea (sf)	CN [Description		
7,985 55 Woods, Good, HSG B						
		84,935	<u>70 \</u>	Voods, Go	od, HSG C	
		92,920	69 \	Veighted A	verage	
		92,920		100.00% Pe	ervious Are	a
	Тс	Length	Slope	Velocity	Capacity	Description
(m		(feet)	(ft/ft)	(ft/sec)	(cfs)	
5	5.5	50	0.1600	0.15		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
().3	45	0.2200	2.35		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
2	3.4	290	0.0800	1.41		Shallow Concentrated Flow,
		200	0.0000			Woodland Kv= 5.0 fps
	9.2	385	Total			

Runoff = 8.60 cfs @ 12.24 hrs, Volume= 38,172 cf, Depth> 1.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN D	escription		
	80,545	55 V	Voods, Goo	od, HSG B	
	21,865			od, HSG C	
	47,880	77 V	Voods, Goo	od, HSG D	
	50,290		Veighted A		
2	50,290	1	00.00% Pe	ervious Area	а
т.	1	0		0	
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.8	50	0.0300	0.08		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.9	115	0.0400	1.00		
		0.0+00	1.00		Shallow Concentrated Flow,
		0.0400	1.00		Woodland Kv= 5.0 fps
0.2	30	0.3500	2.96		•
0.2	30				Woodland Kv= 5.0 fps
0.2 3.9	30 310				Woodland Kv= 5.0 fps Shallow Concentrated Flow,
-		0.3500	2.96		Woodland Kv= 5.0 fps Shallow Concentrated Flow, Woodland Kv= 5.0 fps

Summary for Subcatchment E6: AP6 - To Wetland B (off-site)

Runoff = 4.36 cfs @ 12.13 hrs, Volume= 15,405 cf, Depth> 1.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN [Description		
		7,985		,	od, HSG B	
_		84,935			od, HSG C	
		92,920	69 N	Veighted A	verage	
		92,920		100.00% Pe	ervious Area	а
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
	5.5	50	0.1600	0.15		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	0.3	45	0.2200	2.35		Shallow Concentrated Flow,
						,
	34	290	0 0800	1 4 1		
	5.1	200	0.0000			•
-	0.2	385	Total			
_	3.4 9.2	290 385	0.2200 0.0800 Total	1.41		Woodland Kv= 5.0 fps Shallow Concentrated Flow, Woodland Kv= 5.0 fps

Runoff = 12.80 cfs @ 12.23 hrs, Volume= 55,402 cf, Depth> 2.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Α	rea (sf)	CN D	escription		
	80,545	55 V	Voods, Goo	od, HSG B	
	21,865			od, HSG C	
	47,880	77 V	Voods, Goo	od, HSG D	
2	50,290		Veighted A		
2	50,290	1	00.00% Pe	ervious Area	а
т.	1	0		0	Description
TC	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.8	50	0.0300	0.08		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.9	115	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	30	0.3500	2.96		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.9	310	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps

Summary for Subcatchment E6: AP6 - To Wetland B (off-site)

Runoff = 6.35 cfs @ 12.13 hrs, Volume= 22,053 cf, Depth> 2.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN E	escription		
	7,985		,	od, HSG B	
	84,935	70 V	Voods, Go	od, HSG C	
	92,920	69 V	Veighted A	verage	
	92,920			ervious Area	a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	· · · · · · · · · · · · · · · · · · ·
5.5	50	0.1600	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
0.3	45	0.2200	2.35		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.4	290	0.0800	1.41		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
9.2	385	Total			

Runoff = 19.79 cfs @ 12.23 hrs, Volume= 84,447 cf, Depth> 4.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

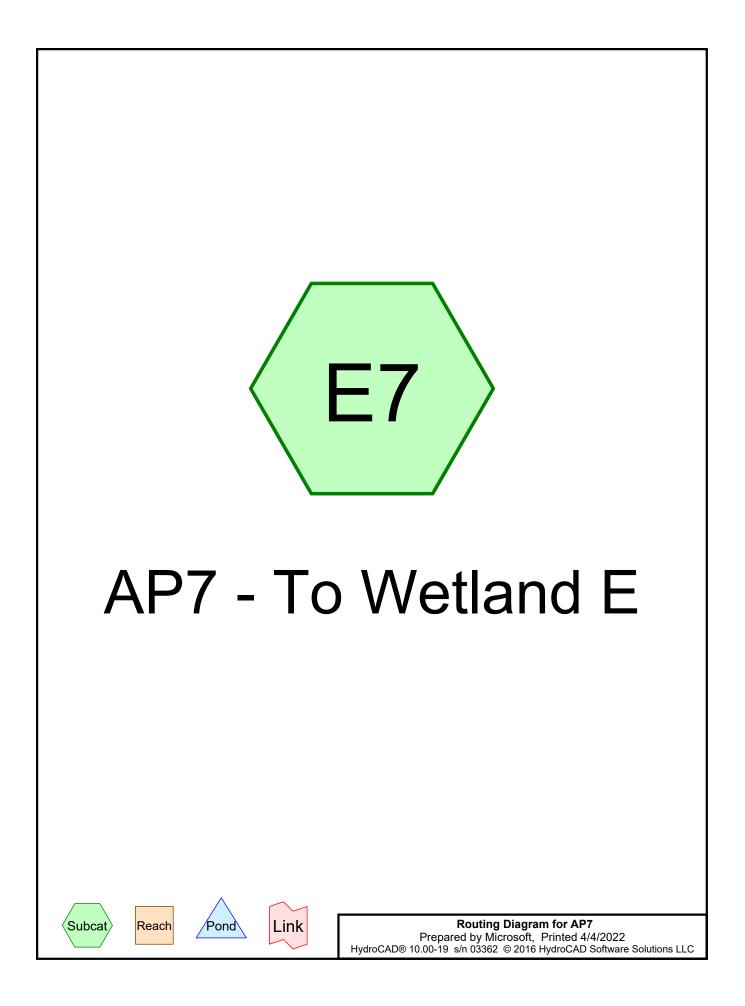
A	rea (sf)	CN D	escription		
	80,545	55 V	Voods, Go	od, HSG B	
1	21,865		,	od, HSG C	
	47,880	77 V	Voods, Go	od, HSG D	
2	50,290	67 V	Veighted A	verage	
2	50,290	1	00.00% Pe	ervious Area	a
_				_	
Тс	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.8	50	0.0300	0.08		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.9	115	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.2	30	0.3500	2.96		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.9	310	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
16.8	505	Total			

Summary for Subcatchment E6: AP6 - To Wetland B (off-site)

Runoff = 9.62 cfs @ 12.13 hrs, Volume= 33,159 cf, Depth> 4.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN E	Description		
	7,985		Voods, Go	,	
	84,935	70 V	Voods, Go	od, HSG C	
	92,920	69 V	Veighted A	verage	
	92,920	1	00.00% Pe	ervious Area	а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
5.5	50	0.1600	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
0.3	45	0.2200	2.35		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.4	290	0.0800	1.41		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
9.2	385	Total			



Runoff = 1.61 cfs @ 12.40 hrs, Volume= 10,728 cf, Depth> 0.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN D	escription		
	47,040		,	od, HSG B	
1	13,445	70 V	<u>Voods, Go</u>	od, HSG C	
2	260,485	62 Weighted Average			
2	260,485	100.00% Pervious Area			а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
8.0	675	0.0800	1.41		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.5	85	0.3000	2.74		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.3	30	0.1000	1.58		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
21.5	840	Total			

Runoff = 6.18 cfs @ 12.33 hrs, Volume= 31,700 cf, Depth> 1.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN D	escription		
	47,040		,	od, HSG B	
1	13,445	70 V	<u>Voods, Go</u>	od, HSG C	
2	260,485	62 Weighted Average			
2	260,485	100.00% Pervious Area			а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
8.0	675	0.0800	1.41		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.5	85	0.3000	2.74		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.3	30	0.1000	1.58		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
21.5	840	Total			

Runoff = 9.74 cfs @ 12.31 hrs, Volume= 47,805 cf, Depth> 2.20"

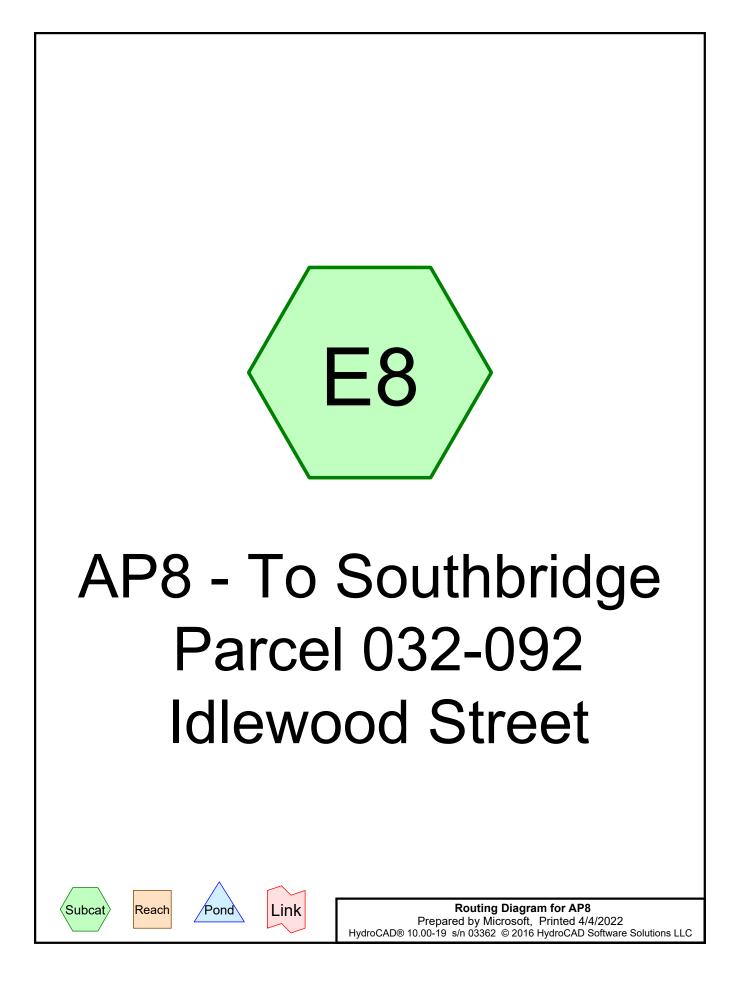
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN D	escription		
	47,040		,	od, HSG B	
1	13,445	70 V	<u>Voods, Go</u>	od, HSG C	
2	260,485	62 Weighted Average			
2	260,485	100.00% Pervious Area			а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
8.0	675	0.0800	1.41		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.5	85	0.3000	2.74		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.3	30	0.1000	1.58		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
21.5	840	Total			

Runoff = 15.87 cfs @ 12.30 hrs, Volume= 75,646 cf, Depth> 3.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	Α	rea (sf)	CN D	escription				
	1	47,040	55 V	Voods, Go	od, HSG B			
	1	13,445	70 V	Voods, Go	od, HSG C			
	2	60,485	62 V	Weighted Average				
	2	60,485	100.00% Pervious Area			а		
	Тс	Length	Slope	Velocity	Capacity	Description		
<u>(m</u>	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
12	2.7	50	0.0200	0.07		Sheet Flow,		
						Woods: Light underbrush n= 0.400 P2= 3.00"		
8	3.0	675	0.0800	1.41		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
().5	85	0.3000	2.74		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
().3	30	0.1000	1.58		Shallow Concentrated Flow,		
						Woodland Kv= 5.0 fps		
2	1.5	840	Total					



Summary for Subcatchment E8: AP8 - To Southbridge Parcel 032-092 Idlewood Street

Runoff = 0.40 cfs @ 12.17 hrs, Volume= 1,626 cf, Depth> 0.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

_	A	rea (sf)	CN I	Description						
		18,055	70 Woods, Good, HSG C							
_		3,635	74 >	74 >75% Grass cover, Good, HSG C						
		21,690	71 \	Neighted A	verage					
	21,690 100.00% Pervious Area					а				
	Тс	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	8.8	50	0.0500	0.09		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.00"				
	2.2	150	0.0500	1.12		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	11.0	200	Total							

Summary for Subcatchment E8: AP8 - To Southbridge Parcel 032-092 Idlewood Street

Runoff = 1.05 cfs @ 12.16 hrs, Volume= 3,884 cf, Depth> 2.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN I	Description						
		18,055	70 Woods, Good, HSG C							
_		3,635	74 >	74 >75% Grass cover, Good, HSG C						
		21,690	71 \	Neighted A	verage					
	21,690 100.00% Pervious Area					а				
	Тс	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	8.8	50	0.0500	0.09		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.00"				
	2.2	150	0.0500	1.12		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	11.0	200	Total							

Summary for Subcatchment E8: AP8 - To Southbridge Parcel 032-092 Idlewood Street

Runoff = 1.50 cfs @ 12.16 hrs, Volume= 5,490 cf, Depth> 3.04"

_	A	rea (sf)	CN I	Description						
		18,055								
_		3,635	74 >	>75% Grass cover, Good, HSG C						
		21,690	71 \	Neighted A	verage					
	21,690 100.00% Pervious Are					а				
	Тс	Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	8.8	50	0.0500	0.09		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.00"				
	2.2	150	0.0500	1.12		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	11.0	200	Total							

Summary for Subcatchment E8: AP8 - To Southbridge Parcel 032-092 Idlewood Street

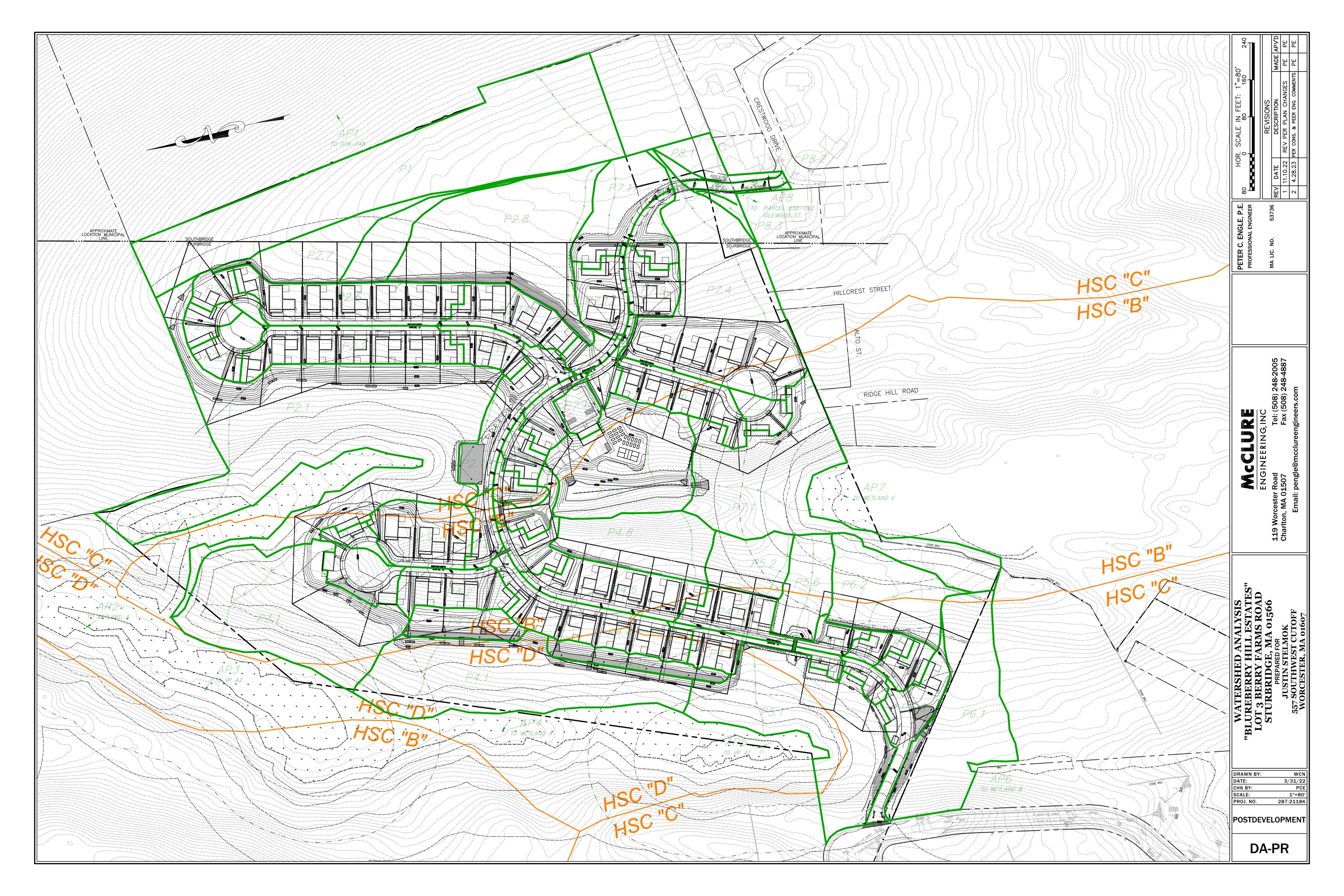
Runoff = 2.23 cfs @ 12.15 hrs, Volume= 8,149 cf, Depth> 4.51"

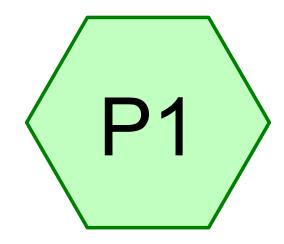
_	A	rea (sf)	CN [Description							
		18,055	70 \								
_		3,635	74 >	>75% Grass cover, Good, HSG C							
		21,690	71 \	Veighted A	verage						
21,690 100.00% Pervious Are					ervious Are	a					
	_		~		• •	— • • •					
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	8.8	50	0.0500	0.09		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.00"					
	2.2	150	0.0500	1.12		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	11.0	200	Total								

McClure Engineering, Inc. March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023

APPENDIX E

POST-DEVELOPMENT HYDROCAD DRAINAGE CALCULATIONS





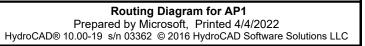
AP1 - To Southbridge Parcel 019-048 (0 Cournoyer Blvd)

Link

Pond

Subcat

Reach



Runoff = 2.56 cfs @ 12.23 hrs, Volume= 11,765 cf, Depth> 0.85"

_	A	rea (sf)	CN [Description		
166,550 70 Woods, Good, HSG C						
166,550 100.00% Pervious Area					ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.7	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
	2.4	220	0.0950	1.54		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
-	15.1	270	Total			

Runoff = 6.86 cfs @ 12.22 hrs, Volume= 28,675 cf, Depth> 2.07"

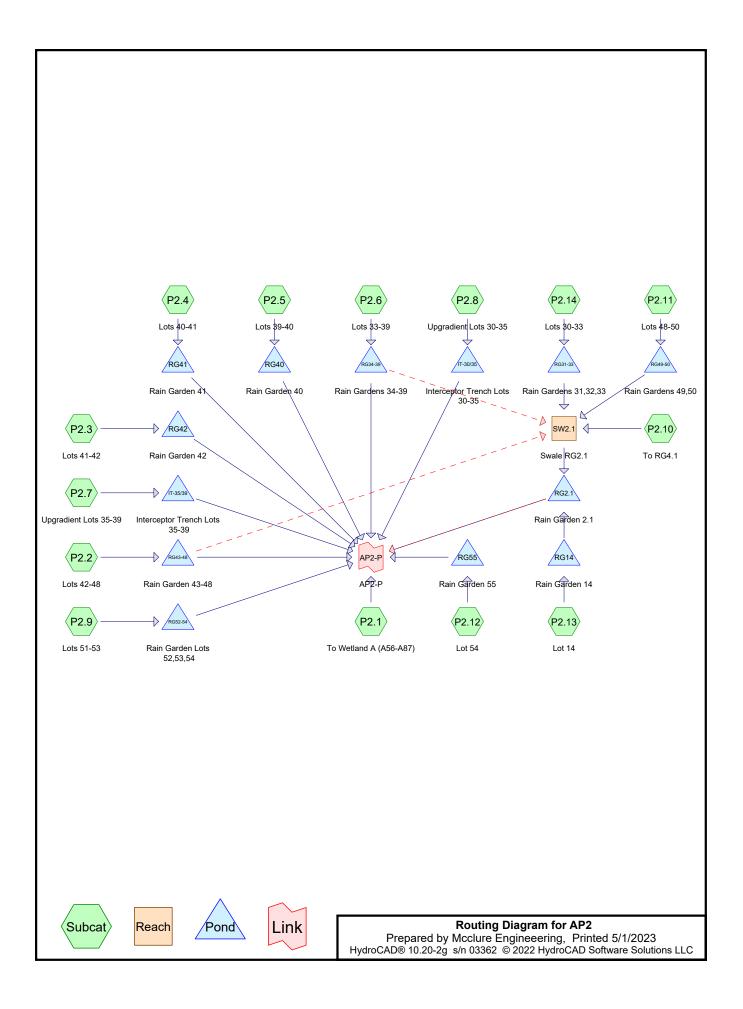
_	A	rea (sf)	CN E	Description		
	1	66,550				
	166,550 100.00% Pervious Area					a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	12.7	50	0.0200	0.07		Sheet Flow,
	2.4	220	0.0950	1.54		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	15.1	270	Total			

Runoff = 9.90 cfs @ 12.21 hrs, Volume= 40,790 cf, Depth> 2.94"

_	A	rea (sf)	CN [Description		
166,550 70 Woods, Good, HSG C						
166,550 100.00% Pervious Area					ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.7	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
	2.4	220	0.0950	1.54		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
-	15.1	270	Total			

Runoff = 14.91 cfs @ 12.20 hrs, Volume= 60,940 cf, Depth> 4.39"

A	rea (sf)	CN E	Description		
1	66,550				
1	66,550	1	00.00% Pe	ervious Are	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	50	0.0200	0.07		Sheet Flow,
2.4	220	0.0950	1.54		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
15.1	270	Total			



Summary for Subcatchment P2.1: To Wetland A (A56-A87)

Runoff = 3.65 cfs @ 12.15 hrs, Volume= 14,371 cf, Depth> 0.85" Routed to Link AP2-P : AP2-P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN [Description							
1	42,635	70 \	Woods, Good, HSG C							
	45,615	74 >	>75% Grass cover, Good, HSG C							
	10,360	55 \	Woods, Good, HSG B							
	3,340		>75% Grass cover, Good, HSG B							
	1,060	<u>98 l</u>	Jnconnecte	ed roofs, H	SG B					
203,010 70 Weighted Average										
2	01,950	ę	9.48% Per	rvious Area						
	1,060			ervious Are						
	1,060		00.00% U	nconnected	1					
_		~ .		.						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	Capacity (cfs)	Description					
	•		,		Sheet Flow,					
<u>(min)</u> 5.1	(feet) 50	(ft/ft) 0.2000	(ft/sec) 0.16		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"					
(min)	(feet)	(ft/ft)	(ft/sec)		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow,					
(min) 5.1 0.7	(feet) 50 90	(ft/ft) 0.2000 0.2000	(ft/sec) 0.16 2.24		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps					
<u>(min)</u> 5.1	(feet) 50	(ft/ft) 0.2000	(ft/sec) 0.16		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps Shallow Concentrated Flow,					
(min) 5.1 0.7	(feet) 50 90	(ft/ft) 0.2000 0.2000	(ft/sec) 0.16 2.24		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps					

Summary for Subcatchment P2.10: To RG4.1

Runoff = 1.01 cfs @ 12.11 hrs, Volume= 3,318 cf, Depth> 1.50" Routed to Reach SW2.1 : Swale RG2.1

	Area (sf)	CN	Description
*	12,070	90	Residential Lots, 65% imp, HSG C
	2,925	70	Woods, Good, HSG C
	11,575	74	>75% Grass cover, Good, HSG C
	26,570	81	Weighted Average
	18,725		70.47% Pervious Area
	7,846		29.53% Impervious Area

AP2					Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24"
Prepare	d by Mc	clure Eng	gineeering)	Printed 5/1/2023
HydroCA	D® 10.20-	2g_s/n 03	362 © 202	2 HydroCAD	O Software Solutions LLC Page 3
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.2	50	0.1200	0.13		Sheet Flow,
1.0	100	0.1200	1.73		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps

7.2 150 Total

Summary for Subcatchment P2.11: Lots 48-50

1,912 cf, Depth> 2.20"

Runoff = 0.61 cfs @ 12.07 hrs, Volume= Routed to Pond RG49-50 : Rain Gardens 49,50

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YearMass Rainfall=3.24"

	A	rea (sf)	CN	Description						
*		10,410	90 Residential Lots, 65% imp, HSG C							
		3,644	;	35.00% Pervious Area						
		6,767		65.00% Impervious Area						
	Тс	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	,	(cfs)	Description				
	5.0					Direct Entry,				

Summary for Subcatchment P2.12: Lot 54

Runoff = 0.32 cfs @ 12.07 hrs, Volume= 1,001 cf, Depth> 2.20" Routed to Pond RG55 : Rain Garden 55

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YearMass Rainfall=3.24"

	A	rea (sf)	CN [Description						
*		5,450	90 F	90 Residential Lots, 65% imp, HSG C						
		1,908	3	35.00% Pervious Area						
		3,543	6	65.00% Impervious Area						
	т.	1	<u>Olama</u>	\/_l;t	0	Description				
	Tc (min)	Length	Slope	,	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0					Direct Entry,				

Summary for Subcatchment P2.13: Lot 14

Runoff = 0.39 cfs @ 12.07 hrs, Volume= 1,212 cf, Depth> 2.12" Routed to Pond RG14 : Rain Garden 14

A	rea (sf)	CN	Description					
	5,915	90	1/8 acre lots	s, 65% imp	, HSG C			
	955	85	1/8 acre lots, 65% imp, HSG B					
	6,870	89	Weighted Average					
	2,405		35.00% Pervious Area					
	4,466		65.00% Imp	ervious Ar	ea			
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P2.14: Lots 30-33

Runoff	=	1.27 cfs @	12.07 hrs,	Volume=	3,950 cf,	Depth>	2.20"
Routed	l to Pond	d RG31-33 - R	Rain Garden	is 31,32,33			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YearMass Rainfall=3.24"

_	A	rea (sf)	CN [Description					
*		21,505	90 F) Residential Lots, 65% imp, HSG C					
		7,527	3	35.00% Pervious Area					
		13,978	6	65.00% Impervious Area					
	Та	Longth	Clana	Valacity	Conosity	Description			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	5.0	(1001)	(1411)	(14000)	(010)	Direct Entry,			
	0.0					,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			

Summary for Subcatchment P2.2: Lots 42-48

Runoff = 2.35 cfs @ 12.07 hrs, Volume= 7,325 cf, Depth> 2.20" Routed to Pond RG43-48 : Rain Garden 43-48

	A	rea (sf)	CN I	Description					
*		39,875	90 I	Residential Lots, 65% imp, HSG C					
		13,956 25,919		35.00% Per 55.00% Imp					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	5.0					Direct Entry,			

Summary for Subcatchment P2.3: Lots 41-42

Runoff = 0.75 cfs @ 12.07 hrs, Volume= 2,336 cf, Depth> 2.20" Routed to Pond RG42 : Rain Garden 42

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Area (sf)	CN Description						
12,715	90 1/8 acre lots, 65% imp, HSG C						
4,450	35.00% Pervious Area	35.00% Pervious Area					
8,265	65.00% Impervious Area						
Tc Length (min) (feet)							
5.0	Direct Entry,						

Summary for Subcatchment P2.4: Lots 40-41

Runoff = 0.44 cfs @ 12.07 hrs, Volume= 1,382 cf, Depth> 2.20" Routed to Pond RG41 : Rain Garden 41

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN E	Description					
	7,525	90 1	/8 acre lots	s, 65% imp	, HSG C			
	2,634 4,891	-	35.00% Pervious Area 65.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P2.5: Lots 39-40

Runoff = 0.56 cfs @ 12.07 hrs, Volume= Routed to Pond RG40 : Rain Garden 40

1,750 cf, Depth> 2.20"

Area (sf)	CN	Description
9,525	90	1/8 acre lots, 65% imp, HSG C
3,334		35.00% Pervious Area
6,191		65.00% Impervious Area

			ngineeering 03362 © 202) Software Solu	utions LLC	Printed 5/1/2023 Page
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		-
5.0					Direct Entry	y ,	
			Summary	/ for Sub	catchment	P2.6: Lots 33-39	
Runoff Route	= d to Pon		cfs @ 12.0 -39 : Rain G			8,319 cf, Depth> 2.20)"
			ethod, UH=S s Rainfall=3		ted-CN, Time	Span= 0.00-24.00 hrs,	dt= 0.05 hrs
	rea (sf)		Description				
	45,285	90			imp, HSG C		
	15,850 29,435		35.00% Per 65.00% Imp				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
5.0					Direct Entry	y ,	
		Sumr	mary for S	ubcatch	ment P2.7:	Upgradient Lots 35	-39
Runoff Route	= d to Pon		cfs @ 12.3 /39 : Intercej			3,211 cf, Depth> 0.8	5"
			ethod, UH=S s Rainfall=3		ted-CN, Time	Span= 0.00-24.00 hrs,	dt= 0.05 hrs
A	rea (sf)	CN	Description				
	4,550 40,950	74 70	>75% Gras Woods, Go		ood, HSG C		
	45,500	70					

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	12.7	50	0.0200	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"
	7.1	560	0.0700	1.32		Shallow Concentrated Flow,
	0.1	20	0.5000	4.95		Woodland Kv= 5.0 fps Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
-	40.0	000	- + +			

19.9 630 Total

AP2

Summary for Subcatchment P2.8: Upgradient Lots 30-35

Runoff = 1.42 cfs @ 12.25 hrs, Volume= 6,623 cf, Depth> 0.90" Routed to Pond IT-30/35 : Interceptor Trench Lots 30-35

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN E	Description		
	11,085			,	ood, HSG C
	77,375	70 V	<u>Voods, Go</u>	<u>od, HSG C</u>	
	88,460	71 V	Veighted A	verage	
	88,460	1	00.00% Pe	ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
3.5	280	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.1	30	0.5000	4.95		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
16.3	360	Total			

Summary for Subcatchment P2.9: Lots 51-53

Runoff = 1.19 cfs @ 12.08 hrs, Volume= 3,680 cf, Depth> 1.57" Routed to Pond RG52-54 : Rain Garden Lots 52,53,54

Area (sf)	CN	Description					
2,430	90	1/8 acre lots, 65% imp, HSG C					
20,825	85	1/8 acre lots, 65% imp, HSG B					
3,605	70	Woods, Good, HSG C					
1,280	55	Woods, Good, HSG B					
28,140	82	Weighted Average					
13,024		46.28% Pervious Area					
15,116		53.72% Impervious Area					
Tc Length							
(min) (feet) (ft/	/ft) (ft/sec) (cfs)					
5.0		Direct Entry,					

Summary for Reach SW2.1: Swale RG2.1

Inflow Area = 58,485 sf, 48.88% Impervious, Inflow Depth > 2.32" for 2YearMass event 1.98 cfs @ 12.16 hrs, Volume= Inflow 11.286 cf 1.97 cfs @ 12.16 hrs, Volume= 11,284 cf, Atten= 0%, Lag= 0.3 min Outflow = Routed to Pond RG2.1 : Rain Garden 2.1 Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Max. Velocity= 6.61 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.45 fps, Avg. Travel Time= 0.8 min Peak Storage= 36 cf @ 12.16 hrs Average Depth at Peak Storage= 0.21', Surface Width= 1.84' Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 46.26 cfs 1.00' x 1.00' deep channel, n= 0.025 Earth, clean & winding Side Slope Z-value= 2.0 '/' Top Width= 5.00' Length= 120.0' Slope= 0.1500 '/' Inlet Invert= 722.00', Outlet Invert= 704.00' Summary for Pond IT-30/35: Interceptor Trench Lots 30-35 Inflow Area = 88,460 sf, 0.00% Impervious, Inflow Depth > 0.90" for 2YearMass event Inflow 1.42 cfs @ 12.25 hrs, Volume= 6.623 cf = Outflow = 1.36 cfs @ 12.31 hrs, Volume= 6,565 cf, Atten= 4%, Lag= 3.6 min 1.36 cfs @ 12.31 hrs, Volume= = Primary 6.565 cf Routed to Link AP2-P : AP2-P

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.56' @ 12.31 hrs Surf.Area= 1,470 sf Storage= 421 cf

Plug-Flow detention time= 12.1 min calculated for 6,565 cf (99% of inflow) Center-of-Mass det. time= 7.3 min (885.7 - 878.4)

Volume	Invert	Avail.Stora	age	Storage Description
#1	732.00'	1,696	6 cf	3.00'W x 490.00'L x 3.00'H Prismatoid
#2	732.00'	17 [.]	1 cf	4,410 cf Overall - 171 cf Embedded = 4,239 cf x 40.0% Voids 8.0" Round Pipe Storage Inside #1
<i>\\\\</i>	102.00	17		L= 490.0'
		1,867	7 cf	Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Primary	732.00'	15.0'	' Round Culvert
			L= 25	50.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 732.00' / 724.00' S= 0.0320 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.35 cfs @ 12.31 hrs HW=732.56' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.35 cfs @ 2.54 fps)

Summary for Pond IT-35/39: Interceptor Trench Lots 35-39

 Inflow Area =
 45,500 sf, 0.00% Impervious, Inflow Depth > 0.85" for 2YearMass event

 Inflow =
 0.63 cfs @
 12.31 hrs, Volume=
 3,211 cf

 Outflow =
 0.58 cfs @
 12.40 hrs, Volume=
 3,173 cf, Atten= 7%, Lag= 5.4 min

 Primary =
 0.58 cfs @
 12.40 hrs, Volume=
 3,173 cf

 Routed to Link AP2-P : AP2-P
 3,173 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 734.41' @ 12.40 hrs Surf.Area= 1,200 sf Storage= 251 cf

Plug-Flow detention time= 15.6 min calculated for 3,173 cf (99% of inflow) Center-of-Mass det. time= 9.3 min (893.8 - 884.6)

Volume	Invert	Avail.Storage	Storage Description
#1	734.00'	1,384 cf	3.00'W x 400.00'L x 3.00'H Prismatoid
			3,600 cf Overall - 140 cf Embedded = 3,460 cf x 40.0% Voids
#2	734.00'	140 cf	8.0" Round Pipe Storage Inside #1
			L= 400.0'
		1,524 cf	Total Available Storage
			·
Device	Routing	Invert Outl	et Devices
#1	Primary	734.00' 10.0	" Round Culvert

#1	Primary	734.00°	10.0 ^{°°} Round Culvert
	-		L= 80.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 734.00' / 733.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=0.58 cfs @ 12.40 hrs HW=734.41' TW=0.00' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.58 cfs @ 2.18 fps)

Summary for Pond RG14: Rain Garden 14

 Inflow Area =
 6,870 sf, 65.00% Impervious, Inflow Depth > 2.12" for 2YearMass event

 Inflow =
 0.39 cfs @
 12.07 hrs, Volume=
 1,212 cf

 Outflow =
 0.25 cfs @
 12.18 hrs, Volume=
 1,201 cf, Atten= 37%, Lag= 6.2 min

 Primary =
 0.25 cfs @
 12.18 hrs, Volume=
 1,201 cf

 Routed to Pond RG2.1 : Rain Garden 2.1
 1
 1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 703.98' @ 12.18 hrs Surf.Area= 240 sf Storage= 218 cf

Plug-Flow detention time= 20.7 min calculated for 1,201 cf (99% of inflow) Center-of-Mass det. time= 15.4 min (824.7 - 809.3)

Postdevelopment

Type III 24-hr 2YearMass Rainfall=3.24"

Printed 5/1/2023 Page 10

Volume	Invert	Avail.Storage	Storage Description
<u>volume</u> #1	702.00'	47 cf	
	700.001	000 (L= 60.0'
#2	702.00'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	704.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
#4	704.50'	373 cf	60 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
<u></u>	704.00	720 cf	
Device	Routing	Invert Out	let Devices
#1	Primary	702.00' 6.0'	' Round Culvert
	•	L= 1	75.0' CPP, square edge headwall, Ke= 0.500
			t / Outlet Invert= 702.00' / 702.00' S= 0.0000 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		'Horiz. Orifice/Grate C= 0.600
#2	DEVICE		ited to weir flow at low heads
#2	Device 1		
#3		-	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	703.75' 4.0'	'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.24 cfs @ 12.18 hrs HW=703.97' TW=699.63' (Dynamic Tailwater)

1=Culvert (Passes 0.24 cfs of 0.70 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.61 fps)

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

-4=Orifice/Grate (Orifice Controls 0.10 cfs @ 1.60 fps)

Summary for Pond RG2.1: Rain Garden 2.1

65,355 sf	, 50.58% Impervious	, Inflow Depth > 2.29" for 2YearMass event
2.23 cfs @	12.17 hrs, Volume=	12,485 cf
0.65 cfs @	13.36 hrs, Volume=	12,485 cf, Atten= 71%, Lag= 71.5 min
0.48 cfs @	13.36 hrs, Volume=	10,942 cf
0.17 cfs @	13.35 hrs, Volume=	1,542 cf
AP2-P : AP2-	P	
0.00 cfs @	0.00 hrs, Volume=	0 cf
AP2-P : AP2-	P	
	2.23 cfs @ 0.65 cfs @ 0.48 cfs @ 0.17 cfs @ AP2-P : AP2- 0.00 cfs @	65,355 sf, 50.58% Impervious 2.23 cfs @ 12.17 hrs, Volume= 0.65 cfs @ 13.36 hrs, Volume= 0.48 cfs @ 13.36 hrs, Volume= 0.17 cfs @ 13.35 hrs, Volume= AP2-P : AP2-P 0.00 cfs @ 0.00 hrs, Volume= AP2-P : AP2-P

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 701.75' @ 13.35 hrs Surf.Area= 8,500 sf Storage= 5,147 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 153.5 min (976.9 - 823.4)

AP2

Prepared by Mcclure Engineeering

Postdevelopment

Type III 24-hr 2YearMass Rainfall=3.24"

Printed 5/1/2023 Page 11

<u>HydroCAD® 10.20-2g_s/n 03362</u>	© 2022 HydroCAD Software Solutions LLC	P

Invert	Avail.Sto	rage	ge Storage Description		
698.75'	6	67 cf	· · · · · · · · · · · · · · · · · · ·		
			L= 85.0'		
698.75'	5,07	73 cf			
701 751	E.	01 of	12,750 cf Overall - 67 cf Embedded = 12,683 cf x 40.0% Voids 50.00'W x 85.00'L x 0.25'H Mulch		
701.75	50		1,063 cf Overall x 50.0% Voids		
702 00'	9.62	23 cf	50.00'W x 85.00'L x 2.00'H Ponding Z=2.0		
102.00			Total Available Storage		
	10,20				
Routing	Invert	Outl	et Devices		
Primary	701.00'	12.0	" Round Culvert		
-		L= 1	0.0' CPP, square edge headwall, Ke= 0.500		
		Inlet / Outlet Invert= 701.00' / 699.65' S= 0.1350 '/' Cc= 0.900			
			0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf		
Device 1	703.00'	10.0" Horiz. Orifice/Grate X 2.00 C= 0.600			
During 4	704 001		ted to weir flow at low heads		
Device 1	701.00	2.0" Vert. Orifice/Grate X 2.00 C= 0.600			
Davias 1	701 75'	Limited to weir flow at low heads			
Device I	701.75	6.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads			
Secondary	703 75'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir			
coolidary	100.10		d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60		
			f. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64		
Discarded	698.75'		0 in/hr Exfiltration over Surface area		
-	698.75' 698.75' 701.75' 702.00' Routing Primary Device 1 Device 1 Device 1 Secondary	698.75' 6 698.75' 5,0'' 701.75' 5' 702.00' 9,6' 15,29 15,29 Routing Invert Primary 701.00' Device 1 703.00' Device 1 701.00' Device 1 701.75' Secondary 703.75'	698.75' 67 cf 698.75' 5,073 cf 701.75' 531 cf 702.00' 9,623 cf 15,294 cf Routing Invert Primary 701.00' 11 11 Primary 701.00' 10 L= 1 Inlet n= 0 Device 1 703.00' 10.0 Limi Device 1 701.75' 6.0" Limi Secondary 703.75' 10.0 Hea Coe Coe		

Discarded OutFlow Max=0.47 cfs @ 13.36 hrs HW=701.75' (Free Discharge) **G=Exfiltration** (Exfiltration Controls 0.47 cfs)

Primary OutFlow Max=0.17 cfs @ 13.35 hrs HW=701.75' TW=0.00' (Dynamic Tailwater) 1=Culvert (Passes 0.17 cfs of 1.87 cfs potential flow) 2=Orifice/Grate (Controls 0.00 cfs) 3=Orifice/Grate (Orifice Controls 0.17 cfs @ 3.94 fps) 4=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.19 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=698.75' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG31-33: Rain Gardens 31,32,33

Inflow Are	a =	21,505 sf,	65.00% Impervious,	Inflow Depth >	2.20"	for 2YearMass event			
Inflow	=	1.27 cfs @	12.07 hrs, Volume=	3,950 c	of				
Outflow	=	0.50 cfs @	12.31 hrs, Volume=	3,934 c	of, Atter	n= 61%, Lag= 14.0 min			
Primary	=	0.50 cfs @	12.31 hrs, Volume=	3,934 c	of				
Routed to Reach SW2.1 : Swale RG2.1									

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 730.84' @ 12.31 hrs Surf.Area= 720 sf Storage= 829 cf

Plug-Flow detention time= 18.5 min calculated for 3,934 cf (100% of inflow) Center-of-Mass det. time= 16.0 min (821.1 - 805.1)

AP2

Prepared by Mcclure Engineeering

AP2 Type III 24-hr 2YearMass Rainfall=3.24" Prepared by Mcclure Engineeering Printed 5/1/2023 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 12

Volume Invert		Avail.Sto	rage	Storage Description		
#1	728.25'	14	11 cf	12.0" Round Pipe Storage x 3 Inside #2		
#2	728.25'	80)7 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 3 2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids		
#3	731.25'	ę	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3		
				180 cf Overall x 50.0% Voids		
#4	731.50'	1,12	20 ct	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3		
		2,15	59 cf	Total Available Storage		
				C C		
Device	Routing	Invert	Outl	et Devices		
#1	Primary	728.25'	6.0"	Round Culvert X 3.00		
	·		L= 1	0.0' CPP, square edge headwall, Ke= 0.500		
				/ Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900		
			n= 0	0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	732.00'		Horiz. Orifice/Grate X 3.00 C= 0.600		
			Limited to weir flow at low heads			
#3	Device 1	728.25'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600			
			-	_imited to weir flow at low heads		
#4	Device 1	731.00'	3.0"	Vert. Orifice/Grate X 3.00 C= 0.600		
				ted to weir flow at low heads		

Primary OutFlow Max=0.50 cfs @ 12.31 hrs HW=730.83' TW=722.19' (Dynamic Tailwater)

-1=Culvert (Passes 0.50 cfs of 4.33 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.50 cfs @ 7.61 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond RG34-39: Rain Gardens 34-39

45,285 sf, 65.00% Impervious, Inflow Depth > 2.20" for 2YearMass event Inflow Area = Inflow 2.67 cfs @ 12.07 hrs, Volume= 8.319 cf = Outflow = 1.04 cfs @ 12.31 hrs, Volume= 8,285 cf, Atten= 61%, Lag= 13.9 min 0.70 cfs @ 12.31 hrs, Volume= Primary = 5,523 cf Routed to Link AP2-P : AP2-P 0.35 cfs @ 12.31 hrs, Volume= 2,762 cf Secondary = Routed to Reach SW2.1 : Swale RG2.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 731.03' @ 12.31 hrs Surf.Area= 1,440 sf Storage= 1,769 cf

Plug-Flow detention time= 18.8 min calculated for 8,268 cf (99% of inflow) Center-of-Mass det. time= 16.3 min (821.4 - 805.1) Postdevelopment "Type III 24-hr_2YearMass Rainfall=3.24

Printed 5/1/2023

HydroCAD® 10.20-2g s/n 0336	2 © 2022 HydroCAD Software Solutions LLC	Page 13

Volume	Invert	Avail.Storage		Storage Description	
#1	728.25'	28	33 cf	12.0" Round Pipe Storage x 6 Inside #2 L= 60.0'	
#2	728.25'	1,61	5 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 6 4,320 cf Overall - 283 cf Embedded = 4,037 cf x 40.0% Voids	
#3	731.25'	18	30 cf	4.00'W x 60.00'L x 0.25'H Mulch x 6 360 cf Overall x 50.0% Voids	
#4	731.50'	2,24	l0 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 6	
		4,31	8 cf	Total Available Storage	
Device	Routing	Invert	Outle	et Devices	
#1	Primary	728.25'		Round Culvert X 4.00	
			Inlet	0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900 013 Corrugated PE, smooth interior, Flow Area= 0.20 sf	
#2	Secondary	728.25'		Round Culvert X 2.00	
			L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#3	Device 1	732.00'	0		
#4	Device 2	732.00'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600 ed to weir flow at low heads	
#5	Device 1	728.25'	2.0"	Vert. Orifice/Grate X 4.00 C= 0.600 ed to weir flow at low heads	
#6	Device 2	728.25'	2.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ed to weir flow at low heads	
#7	Device 1	731.00'	3.0"	Vert. Orifice/Grate X 4.00 C= 0.600	
#8	Device 2	731.00'	3.0"	ed to weir flow at low heads Vert. Orifice/Grate X 2.00 C= 0.600 ed to weir flow at low heads	

Primary OutFlow Max=0.69 cfs @ 12.31 hrs HW=731.02' TW=0.00' (Dynamic Tailwater)

1=Culvert (Passes 0.69 cfs of 6.01 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-5=Orifice/Grate (Orifice Controls 0.69 cfs @ 7.90 fps)

-7=Orifice/Grate (Orifice Controls 0.01 cfs @ 0.53 fps)

Secondary OutFlow Max=0.35 cfs @ 12.31 hrs HW=731.02' TW=722.19' (Dynamic Tailwater) 2=Culvert (Passes 0.35 cfs of 3.00 cfs potential flow)

4=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Orifice Controls 0.34 cfs @ 7.90 fps)

-8=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.53 fps)

Summary for Pond RG40: Rain Garden 40

 Inflow Area =
 9,525 sf, 65.00% Impervious, Inflow Depth > 2.20" for 2YearMass event

 Inflow =
 0.56 cfs @
 12.07 hrs, Volume=
 1,750 cf

 Outflow =
 0.42 cfs @
 12.16 hrs, Volume=
 1,743 cf, Atten= 25%, Lag= 5.0 min

 Primary =
 0.42 cfs @
 12.16 hrs, Volume=
 1,743 cf

 Routed to Link AP2-P : AP2-P
 12.16 hrs, Volume=
 1,743 cf

AP2

Prepared by Mcclure Engineeering

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.52' @ 12.16 hrs Surf.Area= 240 sf Storage= 294 cf

Plug-Flow detention time= 16.2 min calculated for 1,740 cf (99% of inflow) Center-of-Mass det. time= 13.9 min (819.0 - 805.1)

Volume	Invert	Avail.Storage	Storage Description
#1	729.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	729.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	732.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	733.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	729.75'	8.0" Round Culvert
			L= 140.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 729.75' / 728.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	733.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	729.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	732.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.41 cfs @ 12.16 hrs HW=732.50' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 0.41 cfs of 1.82 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.17 cfs @ 7.86 fps)

-4=Orifice/Grate (Orifice Controls 0.24 cfs @ 2.78 fps)

Summary for Pond RG41: Rain Garden 41

Inflow Area =	7,525 sf, 65.00% Impervious,	Inflow Depth > 2.20" for 2YearMass event						
Inflow =	0.44 cfs @ 12.07 hrs, Volume=	1,382 cf						
Outflow =	0.30 cfs @ 12.16 hrs, Volume=	1,331 cf, Atten= 32%, Lag= 5.3 min						
Discarded =	0.04 cfs @ 12.15 hrs, Volume=	750 cf						
Primary =	0.26 cfs @ 12.17 hrs, Volume=	581 cf						
Routed to Link AP2-P : AP2-P								

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 731.51' @ 12.17 hrs Surf.Area= 723 sf Storage= 349 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 79.0 min (884.1 - 805.1)

Postdevelopment

Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

<u>e 15</u>

Prepared by Mcclure Engineeering	Printed 5/1/2
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	728.25'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	731.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	731.50'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage
Device	Routing	Invert Outl	let Devices
#1	Discarded	728.25' 2.41	0 in/hr Exfiltration over Surface area
#2	Primary	730.25' 6.0"	' Round Culvert
	•	L= 2	26.0' CPP, square edge headwall, Ke= 0.500
		Inlet	t / Outlet Invert= 730.25' / 730.00' S= 0.0096 '/' Cc= 0.900
		n= 0	0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	732.00' 6.0"	Horiz. Orifice/Grate C= 0.600
		Limi	ited to weir flow at low heads
#4	Device 2	730.25' 2.0''	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

731.00' 3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads #5 Device 2

Discarded OutFlow Max=0.04 cfs @ 12.15 hrs HW=731.50' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.26 cfs @ 12.17 hrs HW=731.49' TW=0.00' (Dynamic Tailwater)

-2=Culvert (Passes 0.26 cfs of 0.83 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.11 cfs @ 5.19 fps)

-5=Orifice/Grate (Orifice Controls 0.14 cfs @ 2.92 fps)

Summary for Pond RG42: Rain Garden 42

Inflow Area =	12,715 sf,	65.00% Impervious,	Inflow Depth > 2.20"	for 2YearMass event
Inflow =	0.75 cfs @ 1	12.07 hrs, Volume=	2,336 cf	
Outflow =	0.40 cfs @ 1	12.21 hrs, Volume=	1,848 cf, Atte	n= 47%, Lag= 8.3 min
Discarded =	0.02 cfs @ 1	12.15 hrs, Volume=	669 cf	-
Primary =	0.38 cfs @ 1	12.21 hrs, Volume=	1,179 cf	
Routed to Link	AP2-P : AP2-P	1		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 731.44' @ 12.21 hrs Surf.Area= 960 sf Storage= 748 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 46.5 min (851.6 - 805.1)

AP2

Postdevelopment

Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 16

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	212 cf	18.0" Round Pipe Storage x 2 Inside #2
			L= 60.0'
#2	728.25'	491 cf	8.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			1,440 cf Overall - 212 cf Embedded = 1,228 cf x 40.0% Voids
#3	731.25'	60 cf	8.00'W x 60.00'L x 0.25'H Mulch
			120 cf Overall x 50.0% Voids
#4	731.50'	621 cf	8.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		1,385 cf	Total Available Storage
Device	Routing	Invert Outl	let Devices

00100	rtouting	1110010	Salet Defield
#1	Discarded	728.25'	1.020 in/hr Exfiltration over Surface area
#2	Primary	730.25'	8.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 730.25' / 730.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Device 2	732.00'	8.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2	730.25'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	731.00'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.02 cfs @ 12.15 hrs HW=731.38' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.37 cfs @ 12.21 hrs HW=731.43' TW=0.00' (Dynamic Tailwater)

-2=Culvert (Passes 0.37 cfs of 1.54 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.24 cfs @ 4.95 fps)

-5=Orifice/Grate (Orifice Controls 0.13 cfs @ 2.66 fps)

Summary for Pond RG43-48: Rain Garden 43-48

Inflow Area =	39,875 sf	, 65.00% Impervious,	Inflow Depth > 2.20" for 2YearMass event
Inflow =	2.35 cfs @	12.07 hrs, Volume=	7,325 cf
Outflow =	1.27 cfs @	12.22 hrs, Volume=	5,907 cf, Atten= 46%, Lag= 8.6 min
Discarded =	0.09 cfs @	12.23 hrs, Volume=	2,453 cf
Primary =	0.99 cfs @	12.22 hrs, Volume=	2,878 cf
Routed to Link	AP2-P : AP2-	·P	
Secondary =	0.20 cfs @	12.22 hrs, Volume=	576 cf
Routed to Read	ch SW2.1 : Sv	vale RG2.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.79' @ 12.21 hrs Surf.Area= 3,600 sf Storage= 2,361 cf

Plug-Flow detention time= 128.5 min calculated for 5,895 cf (80% of inflow) Center-of-Mass det. time= 54.8 min (859.9 - 805.1)

AP2

Postdevelopment AP2 Type III 24-hr 2YearMass Rainfall=3.24" Prepared by Mcclure Engineeering Printed 5/1/2023 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 17 Avail.Storage Storage Description Volume Invert #1 729.75' 12.0" Round Pipe Storage x 6 Inside #2 283 cf L = 60.0'#2 5.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 6 729.75' 2,047 cf 5.400 cf Overall - 283 cf Embedded = 5,117 cf x 40.0% Voids #3 732.75' 225 cf 5.00'W x 60.00'L x 0.25'H Mulch x 6 450 cf Overall x 50.0% Voids #4 733.00' 2,612 cf 5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 6 5,167 cf Total Available Storage Device Routing Invert Outlet Devices Discarded #1 729.75' 1.020 in/hr Exfiltration over Surface area #2 731.75' 6.0" Round Culvert X 5.00 Primary L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 731.75' / 731.65' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf #3 Secondary 731.75' 6.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 731.75' / 731.65' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf #4 Device 2 6.0" Horiz. Orifice/Grate X 5.00 C= 0.600 733.50' Limited to weir flow at low heads 6.0" Horiz. Orifice/Grate C= 0.600 #5 Device 3 733.50' Limited to weir flow at low heads Device 2 #6 731.75' 2.0" Vert. Orifice/Grate X 5.00 C= 0.600 Limited to weir flow at low heads #7 2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads Device 3 731.75 #8 Device 2 732.50' 3.0" Vert. Orifice/Grate X 5.00 C= 0.600 Limited to weir flow at low heads #9 Device 3 732.50' 3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.08 cfs @ 12.23 hrs HW=732.77' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=0.97 cfs @ 12.22 hrs HW=732.78' TW=0.00' (Dynamic Tailwater)

-**2=Culvert** (Passes 0.97 cfs of 4.12 cfs potential flow)

-4=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Orifice Controls 0.51 cfs @ 4.68 fps)

-8=Orifice/Grate (Orifice Controls 0.46 cfs @ 1.87 fps)

Secondary OutFlow Max=0.19 cfs @ 12.22 hrs HW=732.78' TW=722.20' (Dynamic Tailwater) -3=Culvert (Passes 0.19 cfs of 0.82 cfs potential flow)

5=Orifice/Grate (Controls 0.00 cfs)

-7=Orifice/Grate (Orifice Controls 0.10 cfs @ 4.68 fps)

-9=Orifice/Grate (Orifice Controls 0.09 cfs @ 1.87 fps)

Summary for Pond RG49-50: Rain Gardens 49,50

Inflow Area =	10,410 sf, 65.00% Impervious,	Inflow Depth > 2.20" for 2YearMass event	
Inflow =	0.61 cfs @ 12.07 hrs, Volume=	1,912 cf	
Outflow =	0.18 cfs @ 12.40 hrs, Volume=	1,494 cf, Atten= 70%, Lag= 19.7 min	
Discarded =	0.01 cfs @ 10.50 hrs, Volume=	797 cf	
Primary =	0.17 cfs @ 12.40 hrs, Volume=	697 cf	
Routed to Reach SW2.1 : Swale RG2.1			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 730.99' @ 12.40 hrs Surf.Area= 600 sf Storage= 715 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 81.0 min (886.1 - 805.1)

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
#2	728.25'	682 cf	L= 60.0' 5.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 2
			1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids
#3	731.25'	75 cf	5.00'W x 60.00'L x 0.25'H Mulch x 2
			150 cf Overall x 50.0% Voids
#4	731.50'	871 ct	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	728.25'	1.020 in/hr Exfiltration over Surface area
#2	Primary	730.25'	6.0" Round Culvert X 2.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 730.25' / 730.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	732.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	730.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 2	731.00'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 10.50 hrs HW=728.29' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.17 cfs @ 12.40 hrs HW=730.99' TW=722.18' (Dynamic Tailwater) 2=Culvert (Passes 0.17 cfs of 1.22 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -4=Orifice/Grate (Orifice Controls 0.17 cfs @ 3.91 fps) -5=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond RG52-54: Rain Garden Lots 52,53,54

Inflow Are	a =	28,140 sf, 53.72% Impervious,	Inflow Depth > 1.57" for 2YearMass event
Inflow	=	1.19 cfs @ 12.08 hrs, Volume=	3,680 cf
Outflow	=	0.50 cfs @ 12.31 hrs, Volume=	3,662 cf, Atten= 58%, Lag= 14.1 min
Primary	=	0.50 cfs @ 12.31 hrs, Volume=	3,662 cf
Routed	l to Link	AP2-P : AP2-P	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 689.07' @ 12.32 hrs Surf.Area= 720 sf Storage= 752 cf

Plug-Flow detention time= 18.0 min calculated for 3,654 cf (99% of inflow) Center-of-Mass det. time= 15.1 min (849.4 - 834.3)

Volume	Invert	Avail.Storage	Storage Description
#1	686.75'	141 cf	12.0" Round Pipe Storage x 3 Inside #2
			L= 60.0'
#2	686.75'	807 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel × 3
			2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids
#3	689.75'	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
			180 cf Overall x 50.0% Voids
#4	690.00'	1,120 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3
		2,159 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	686.75'	6.0" Round Culvert X 3.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 686.75' / 686.70' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	690.50'	6.0" Horiz. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	686.75'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	689.00'	4.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.50 cfs @ 12.31 hrs HW=689.06' TW=0.00' (Dynamic Tailwater)

-**1=Culvert** (Passes 0.50 cfs of 4.07 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.47 cfs @ 7.19 fps)

-4=Orifice/Grate (Orifice Controls 0.03 cfs @ 0.84 fps)

Summary for Pond RG55: Rain Garden 55

 Inflow Area =
 5,450 sf, 65.00% Impervious, Inflow Depth > 2.20" for 2YearMass event

 Inflow =
 0.32 cfs @
 12.07 hrs, Volume=
 1,001 cf

 Outflow =
 0.13 cfs @
 12.31 hrs, Volume=
 996 cf, Atten= 61%, Lag= 14.2 min

 Primary =
 0.13 cfs @
 12.31 hrs, Volume=
 996 cf

 Routed to Link AP2-P : AP2-P
 AP2-P
 12.31 hrs, Volume=

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Peak Elev= 687.25' @ 12.31 hrs Surf.Area= 240 sf Storage= 208 cf

Plug-Flow detention time= 18.9 min calculated for 996 cf (100% of inflow) Center-of-Mass det. time= 15.9 min (821.1 - 805.1)

Volume	Invert	Avail.Storage	Storage Description
#1	685.75'	106 cf	18.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	685.75'	246 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	688.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	689.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		755 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	685.75'	6.0" Round Culvert
	•		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 685.75' / 685.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	689.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	685.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	688.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.12 cfs @ 12.31 hrs HW=687.25' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 0.12 cfs of 1.06 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.12 cfs @ 5.73 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP2-P: AP2-P

Inflow Area	a =	550,840 sf, 23.14% Impervious,	Inflow Depth > 0.92"	for 2YearMass event
Inflow	=	7.92 cfs @ 12.20 hrs, Volume=	42,215 cf	
Primary	=	7.92 cfs @ 12.20 hrs, Volume=	42,215 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Subcatchment P2.1: To Wetland A (A56-A87)

Runoff = 9.73 cfs @ 12.14 hrs, Volume= 35,011 cf, Depth> 2.07" Routed to Link AP2-P : AP2-P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN E	Description		
1	42,635	70 V	Voods, Go	od, HSG C	
	45,615	74 >	75% Gras	s cover, Go	ood, HSG C
	10,360	55 V	Voods, Go	od, HSG B	
	3,340				ood, HSG B
	1,060	98 L	Inconnecte	ed roofs, H	SG B
2	203,010	70 V	Veighted A	verage	
2	201,950	9	9.48% Per	vious Area	
	1,060			ervious Are	
	1,060	1	00.00% U	nconnected	1
-				A	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.1	50	0.2000	0.16		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
0.7	90	0.2000	2.24		Shallow Concentrated Flow,
• -					Woodland Kv= 5.0 fps
3.7	290	0.0700	1.32		Shallow Concentrated Flow,
9.5					Woodland Kv= 5.0 fps

Summary for Subcatchment P2.10: To RG4.1

Runoff = 2.05 cfs @ 12.11 hrs, Volume= 6,701 cf, Depth> 3.03" Routed to Reach SW2.1 : Swale RG2.1

	Area (sf)	CN	Description
*	12,070	90	Residential Lots, 65% imp, HSG C
	2,925	70	Woods, Good, HSG C
	11,575	74	>75% Grass cover, Good, HSG C
	26,570	81	Weighted Average
	18,725		70.47% Pervious Area
	7,846		29.53% Impervious Area

			gineeering 362 © 202		Postdevelopment <i>Type III 24-hr 10YearMass Rainfall=5.05"</i> Printed 5/1/2023 Software Solutions LLC Page 22
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.2	50	0.1200	0.13		Sheet Flow,
1.0	100	0.1200	1.73		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
7.0	450	T ()			

7.2 150 Total

Summary for Subcatchment P2.11: Lots 48-50

Runoff = 1.07 cfs @ 12.07 hrs, Volume= Routed to Pond RG49-50 : Rain Gardens 49,50

3,403 cf, Depth> 3.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"

	A	rea (sf)	CN I	Description							
*		10,410	90 I	Residential	Lots, 65%	b imp, HSG C					
		3,644	(35.00% Pervious Area							
		6,767	6	65.00% Impervious Area							
	-		<u></u>		A						
	TC	Length	Slope	,	Capacity						
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry,					

Summary for Subcatchment P2.12: Lot 54

Runoff = 0.56 cfs @ 12.07 hrs, Volume= 1,781 cf, Depth> 3.92" Routed to Pond RG55 : Rain Garden 55

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN I	Description							
*		5,450	90 I	Residential	Lots, 65%	imp, HSG C					
		1,908	3	35.00% Pervious Area							
		3,543	6	65.00% Impervious Area							
	-		0		o						
	Tc	Length	Slope	,	Capacity	•					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry,					

Summary for Subcatchment P2.13: Lot 14

Runoff = 0.69 cfs @ 12.07 hrs, Volume= 2,186 cf, Depth> 3.82" Routed to Pond RG14 : Rain Garden 14

A	rea (sf)	CN	Description				
	5,915	90	1/8 acre lot	s, 65% imp	o, HSG C		
	955	85	1/8 acre lots	s, 65% imp	o, HSG B		
	6,870	89	Weighted A	verage			
	2,405		35.00% Pervious Area				
	4,466		65.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
5.0					Direct Entry,		

Summary for Subcatchment P2.14: Lots 30-33

Runoff	=	2.20 cfs @	12.07 hrs,	Volume=	7,029 cf,	Depth>	3.92"
Routed	I to Pond	I RG31-33 : F	Rain Garden	s 31,32,33		-	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN E	Description						
*		21,505	90 F	Residential	Lots, 65%	imp, HSG C				
		7,527	3	35.00% Pervious Area						
		13,978	6	65.00% Impervious Area						
	Тс	Longth	Slope	Velocity	Capacity	Description				
	(min)	Length (feet)	(ft/ft)	(ft/sec)	(cfs)	Description				
_	5.0			· · · ·		Direct Entry,				
	5.0					Direct Entry,				

Summary for Subcatchment P2.2: Lots 42-48

Runoff = 4.08 cfs @ 12.07 hrs, Volume= 13,034 cf, Depth> 3.92" Routed to Pond RG43-48 : Rain Garden 43-48

	A	rea (sf)	CN I	Description				
*		39,875	90 I	Residential Lots, 65% imp, HSG C				
		13,956 25,919			vious Area pervious Are			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	5.0					Direct Entry,		

Summary for Subcatchment P2.3: Lots 41-42

Runoff = 1.30 cfs @ 12.07 hrs, Volume= 4,156 cf, Depth> 3.92" Routed to Pond RG42 : Rain Garden 42

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Are	ea (sf)	CN E	CN Description					
1	2,715	90 1	90 1/8 acre lots, 65% imp, HSG C					
	4,450	35.00% Pervious Area						
	8,265	6	65.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			
			.	for Cub	estalment D2 4: Late 40 44			

Summary for Subcatchment P2.4: Lots 40-41

Runoff = 0.77 cfs @ 12.07 hrs, Volume= 2,460 cf, Depth> 3.92" Routed to Pond RG41 : Rain Garden 41

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN E	Description					
	7,525	90 1	1/8 acre lots, 65% imp, HSG C					
	2,634 4,891	-	35.00% Pervious Area 65.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P2.5: Lots 39-40

Runoff = 0.98 cfs @ 12.07 hrs, Volume= 3,113 Routed to Pond RG40 : Rain Garden 40

3,113 cf, Depth> 3.92"

Area (sf)	CN	Description
9,525	90	1/8 acre lots, 65% imp, HSG C
3,334		35.00% Pervious Area
6,191		65.00% Impervious Area

AP2 7	ype III 24-hr 10YearMass Rainfall=5.05"						
Prepared by Mcclure Engineeering	Printed 5/1/2023						
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solu	tions LLC Page 25						
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
5.0 Direct Entry	,						
Summary for Subcatchment I	P2.6: Lots 33-39						
Runoff = 4.64 cfs @ 12.07 hrs, Volume= Routed to Pond RG34-39 : Rain Gardens 34-39	14,802 cf, Depth> 3.92"						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Type III 24-hr 10YearMass Rainfall=5.05"	Span= 0.00-24.00 hrs, dt= 0.05 hrs						
Area (sf) CN Description							
* 45,285 90 Residential Lots, 65% imp, HSG C							
15,850 35.00% Pervious Area							
29,435 65.00% Impervious Area							
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)							
5.0 Direct Entry	,						
Summary for Subcatchment P2.7: Upgradient Lots 35-39							
Runoff = 1.67 cfs @ 12.29 hrs, Volume= 7,827 cf, Depth> 2.06" Routed to Pond IT-35/39 : Interceptor Trench Lots 35-39							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"							
Area (sf) CN Description							
4,550 74 >75% Grass cover, Good, HSG C							

		4,550 40,950							
45,500 70 Weighted Average 45,500 100.00% Pervious Area									
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	12.7	50	0.0200	0.07		Sheet Flow,			
	7.1	560	0.0700	1.32		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps			
	0.1	20	0.5000	4.95		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
	19.9	630	Total						

Summary for Subcatchment P2.8: Upgradient Lots 30-35

Runoff = 3.67 cfs @ 12.24 hrs, Volume= 15,826 cf, Depth> 2.15" Routed to Pond IT-30/35 : Interceptor Trench Lots 30-35

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Α	rea (sf)	CN E	Description					
	11,085	74 >75% Grass cover, Good, HSG C						
	77,375	70 V	<u>Voods, Go</u>	<u>od, HSG C</u>				
	88,460 71 Weighted Average							
	88,460	1	00.00% Pe	ervious Are	а			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
12.7	50	0.0200	0.07		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
3.5	280	0.0700	1.32		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.1	30	0.5000	4.95		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
16.3	360	Total						

Summary for Subcatchment P2.9: Lots 51-53

Runoff = 2.36 cfs @ 12.08 hrs, Volume= 7,321 cf, Depth> 3.12" Routed to Pond RG52-54 : Rain Garden Lots 52,53,54

Area	a (sf)	CN	Description					
2	2,430	90	1/8 acre lots	s, 65% imp	p, HSG C			
20),825	85	1/8 acre lots	s, 65% imp	p, HSG B			
3	3,605	70	Woods, Goo	od, HSG C				
1	,280	55	Woods, Goo	od, HSG B	3			
28	3,140	82	Weighted Average					
13	3,024		46.28% Pervious Area					
15	5,116		53.72% Imp	ervious Are	rea			
Tc L	ength	Slope		Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
5.0					Direct Entry,			

Summary for Reach SW2.1: Swale RG2.1

Inflow Area = 58,485 sf, 48.88% Impervious, Inflow Depth > 4.51" for 10YearMass event 4.87 cfs @ 12.12 hrs, Volume= Inflow 22.003 cf 4.89 cfs @ 12.12 hrs, Volume= 22,000 cf, Atten= 0%, Lag= 0.0 min Outflow = Routed to Pond RG2.1 : Rain Garden 2.1 Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Max. Velocity= 8.51 fps, Min. Travel Time= 0.2 min Avg. Velocity = 2.91 fps, Avg. Travel Time= 0.7 min Peak Storage= 69 cf @ 12.12 hrs Average Depth at Peak Storage= 0.34', Surface Width= 2.36' Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 46.26 cfs 1.00' x 1.00' deep channel, n= 0.025 Earth, clean & winding Side Slope Z-value= 2.0 '/' Top Width= 5.00' Length= 120.0' Slope= 0.1500 '/' Inlet Invert= 722.00', Outlet Invert= 704.00' Summary for Pond IT-30/35: Interceptor Trench Lots 30-35 Inflow Area = 88,460 sf, 0.00% Impervious, Inflow Depth > 2.15" for 10YearMass event Inflow 3.67 cfs @ 12.24 hrs, Volume= 15,826 cf = 3.61 cfs @ 12.27 hrs, Volume= Outflow = 15,743 cf, Atten= 2%, Lag= 2.0 min 3.61 cfs @ 12.27 hrs, Volume= = Primary 15,743 cf Routed to Link AP2-P : AP2-P Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 733.01' @ 12.27 hrs Surf.Area= 1,470 sf Storage= 694 cf Plug-Flow detention time= 8.3 min calculated for 15,743 cf (99% of inflow) Center-of-Mass det. time= 5.2 min (857.3 - 852.0)

Volume	Invert	Avail.Storag	ge Storage Description
#1	732.00'	1,696	cf 3.00'W x 490.00'L x 3.00'H Prismatoid 4,410 cf Overall - 171 cf Embedded = 4,239 cf x 40.0% Voids
#2	732.00'	171	cf 8.0" Round Pipe Storage Inside #1 L= 490.0'
		1,867	cf Total Available Storage
Device	Routing	Invert C	Dutlet Devices
#1	Primary		5.0" Round Culvert = 250.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 732.00' / 724.00' S= 0.0320' /' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.57 cfs @ 12.27 hrs HW=733.00' TW=0.00' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.57 cfs @ 3.40 fps)

Summary for Pond IT-35/39: Interceptor Trench Lots 35-39

 Inflow Area =
 45,500 sf, 0.00% Impervious, Inflow Depth > 2.06" for 10YearMass event

 Inflow =
 1.67 cfs @
 12.29 hrs, Volume=
 7,827 cf

 Outflow =
 1.61 cfs @
 12.35 hrs, Volume=
 7,774 cf, Atten= 4%, Lag= 3.5 min

 Primary =
 1.61 cfs @
 12.35 hrs, Volume=
 7,774 cf

 Routed to Link AP2-P : AP2-P
 AP2-P
 7,774 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 734.79' @ 12.35 hrs Surf.Area= 1,200 sf Storage= 461 cf

Plug-Flow detention time= 10.6 min calculated for 7,758 cf (99% of inflow) Center-of-Mass det. time= 6.8 min (864.1 - 857.4)

Volume	Invert	Avail.Storage	Storage Description
#1	734.00'	1,384 cf	3.00'W x 400.00'L x 3.00'H Prismatoid
			3,600 cf Overall - 140 cf Embedded = 3,460 cf x 40.0% Voids
#2	734.00'	140 cf	8.0" Round Pipe Storage Inside #1
			L= 400.0'
		1,524 cf	Total Available Storage
Device	Routing	Invert Outl	et Devices
<u>D 0 1100</u>	rtouting		

#1	Primary	734.00'	10.0" Round Culvert
			L= 80.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 734.00' / 733.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=1.61 cfs @ 12.35 hrs HW=734.79' TW=0.00' (Dynamic Tailwater) -1=Culvert (Inlet Controls 1.61 cfs @ 3.02 fps)

Summary for Pond RG14: Rain Garden 14

 Inflow Area =
 6,870 sf, 65.00% Impervious, Inflow Depth > 3.82" for 10YearMass event

 Inflow =
 0.69 cfs @
 12.07 hrs, Volume=
 2,186 cf

 Outflow =
 0.50 cfs @
 12.15 hrs, Volume=
 2,173 cf, Atten= 28%, Lag= 4.8 min

 Primary =
 0.50 cfs @
 12.15 hrs, Volume=
 2,173 cf

 Routed to Pond RG2.1 : Rain Garden 2.1
 2.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 704.54' @ 12.15 hrs Surf.Area= 730 sf Storage= 312 cf

Plug-Flow detention time= 16.9 min calculated for 2,173 cf (99% of inflow) Center-of-Mass det. time= 13.3 min (806.0 - 792.7)

Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 29

Volume	Invert	Avail.Storage	Storage Description
#1	702.00'	47 cf	
#2	702.00'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	704.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
#4	704.50'	373 cf	60 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
#4	704.30		
		720 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	702.00' 6.0 '	' Round Culvert
		L= 7	75.0' CPP, square edge headwall, Ke= 0.500
			t / Outlet Invert= 702.00' / 702.00' S= 0.0000 '/' Cc= 0.900
		n= (0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		'Horiz. Orifice/Grate C= 0.600
		Lim	ited to weir flow at low heads
#3	Device 1	702.00' 2.0 '	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.50 cfs @ 12.15 hrs HW=704.54' TW=701.25' (Dynamic Tailwater)

1=Culvert (Passes 0.50 cfs of 0.83 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs) -3=Orifice/Grate (Orifice Controls 0.16 cfs @ 7.54 fps)

-4=Orifice/Grate (Orifice Controls 0.33 cfs @ 3.80 fps)

Summary for Pond RG2.1: Rain Garden 2.1

Inflow Area	a =	65,355 sf	, 50.58% Ir	npervious,	Inflow Depth >	4.44"	for 1	0YearN	Aass event
Inflow	=	5.39 cfs @	12.12 hrs,	Volume=	24,173 c	f			
Outflow	=	2.20 cfs @	12.56 hrs,	Volume=	23,340 c	f, Atter	า= 59%	∕, Lag⊧	= 26.0 min
Discarded	=	0.72 cfs @	12.56 hrs,	Volume=	15,934 c	f			
Primary	=	1.48 cfs @	12.56 hrs,	Volume=	7,405 c	f			
Routed	to Link	AP2-P : AP2-	Р						
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	0 c	f			
Routed	to Link	AP2-P : AP2-	Р						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 702.43' @ 12.56 hrs Surf.Area= 12,983 sf Storage= 7,529 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 96.2 min (903.0 - 806.8)

AP2

Type III 24-hr 10YearMass Rainfall=5.05"

	ed by Mcclure			Printed 5/1/2023 HydroCAD Software Solutions LLC Page 3
Volume	Invert	Avail.Stor	age	Storage Description
#1	698.75'	6	7 cf	12.0" Round Pipe Storage Inside #2 L= 85.0'
#2	698.75'	5,07	3 cf	50.00'W x 85.00'L x 3.00'H Soil Media and Gravel 12,750 cf Overall - 67 cf Embedded = 12,683 cf x 40.0% Voids
#3	701.75'	53	1 cf	
#4	702.00'	9,62	3 cf	
		,		Total Available Storage
Device	Routing	Invert	Outl	let Devices
#1	Primary	701.00'	L= 1 Inlet)" Round Culvert 10.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 701.00' / 699.65' S= 0.1350 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	703.00'	10.0)" Horiz. Orifice/Grate X 2.00 C= 0.600 ited to weir flow at low heads
#3	Device 1	701.00'	2.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ited to weir flow at low heads
#4	Device 1	701.75'	6.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ited to weir flow at low heads
#5	Secondary	703.75'	10.0 Hea	O' long x 10.0' breadth Broad-Crested Rectangular Weir ad (feet) 0.20 0.40 0.60 0.80 1.00 1.40 1.60 of. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#6	Discarded	698.75'		0 in/hr Exfiltration over Surface area
	ed OutFlow M filtration (Exf			2.56 hrs HW=702.42' (Free Discharge) 0.72 cfs)

Primary OutFlow Max=1.47 cfs @ 12.56 hrs HW=702.42' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Passes 1.47 cfs of 3.64 cfs potential flow) -2=Orifice/Grate (Controls 0.00 cfs) -3=Orifice/Grate (Orifice Controls 0.24 cfs @ 5.58 fps) **4=Orifice/Grate** (Orifice Controls 1.23 cfs @ 3.14 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=698.75' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG31-33: Rain Gardens 31,32,33

Inflow Are	a =	21,505 sf, 65.00% Impervious	, Inflow Depth > 3.92" for 10YearMass event				
Inflow	=	2.20 cfs @ 12.07 hrs, Volume=	7,029 cf				
Outflow	=	1.18 cfs @ 12.20 hrs, Volume=	7,008 cf, Atten= 46%, Lag= 7.9 min				
Primary	=	1.18 cfs @ 12.20 hrs, Volume=	7,008 cf				
Routed to Reach SW2.1 : Swale RG2.1							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 731.83' @ 12.20 hrs Surf.Area= 2,417 sf Storage= 1,317 cf

Plug-Flow detention time= 17.1 min calculated for 7,008 cf (100% of inflow) Center-of-Mass det. time= 15.1 min (804.2 - 789.1)

AP2

AP2 Type III 24-hr 10YearMass Rainfall=5.05" Prepared by Mcclure Engineeering Printed 5/1/2023 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 31

Volume	Invert	Avail.Sto	rage	Storage Description
#1	728.25'	14	11 cf	12.0" Round Pipe Storage x 3 Inside #2
#2	728.25'	80)7 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 3 2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids
#3	731.25'	ę	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
				180 cf Overall x 50.0% Voids
#4	731.50'	1,12	20 ct	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3
		2,15	59 cf	Total Available Storage
				C C
Device	Routing	Invert	Outl	et Devices
#1	Primary	728.25'	6.0"	Round Culvert X 3.00
	·		L= 1	0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900
			n= 0	0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	732.00'		Horiz. Orifice/Grate X 3.00 C= 0.600
			Limi	ted to weir flow at low heads
#3	Device 1	728.25'	2.0"	Vert. Orifice/Grate X 3.00 C= 0.600
			-	ted to weir flow at low heads
#4	Device 1	731.00'	3.0"	Vert. Orifice/Grate X 3.00 C= 0.600
				ted to weir flow at low heads

Primary OutFlow Max=1.18 cfs @ 12.20 hrs HW=731.83' TW=722.32' (Dynamic Tailwater)

-**1=Culvert** (Passes 1.18 cfs of 5.17 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.59 cfs @ 9.00 fps)

-4=Orifice/Grate (Orifice Controls 0.59 cfs @ 4.03 fps)

Summary for Pond RG34-39: Rain Gardens 34-39

45,285 sf, 65.00% Impervious, Inflow Depth > 3.92" for 10YearMass event Inflow Area = Inflow 4.64 cfs @ 12.07 hrs, Volume= 14.802 cf = Outflow = 2.44 cfs @ 12.21 hrs, Volume= 14,758 cf, Atten= 47%, Lag= 8.2 min 1.63 cfs @ 12.21 hrs, Volume= Primary = 9,839 cf Routed to Link AP2-P : AP2-P 0.81 cfs @ 12.21 hrs, Volume= 4,919 cf Secondary = Routed to Reach SW2.1 : Swale RG2.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 731.90' @ 12.21 hrs Surf.Area= 4,953 sf Storage= 2,783 cf

Plug-Flow detention time= 17.1 min calculated for 14,727 cf (99% of inflow) Center-of-Mass det. time= 15.2 min (804.3 - 789.1)

	ed by Mcclure			Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023
HydroCA	D® 10.20-2g s/	<u>n 03362 © 2</u>	2022 H	lydroCAD Software Solutions LLC Page 32
Volume	Invert	Avail.Stor	rage	Storage Description
#1	728.25'	28	33 cf	12.0" Round Pipe Storage x 6 Inside #2 L= 60.0'
#2	728.25'	1,61	15 cf	
#3	731.25'	18	30 cf	4.00'W x 60.00'L x 0.25'H Mulch x 6
#4	731.50'	2,24	40 cf	360 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 6
				Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Primary	728.25'		Round Culvert X 4.00
#2	Secondary	728.25'	Inlet n= 0 6.0'' L= 1 Inlet	 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900 .013 Corrugated PE, smooth interior, Flow Area= 0.20 sf Round Culvert X 2.00 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900 .013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 1	732.00'	6.0"	Horiz. Orifice/Grate X 4.00 C= 0.600 ted to weir flow at low heads
#4	Device 2	732.00'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads
#5	Device 1	728.25'	2.0"	Vert. Orifice/Grate X 4.00 C= 0.600 ted to weir flow at low heads
#6	Device 2	728.25'	2.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads
#7	Device 1	731.00'	3.0"	Vert. Orifice/Grate X 4.00 C= 0.600 ted to weir flow at low heads
#8	Device 2	731.00'	3.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads
Primary OutFlow Max=1.62 cfs @ 12.21 hrs HW=731.90' TW=0.00' (Dynamic Tailwater) 1=Culvert (Passes 1.62 cfs of 6.97 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs)				

-**3=Orifice/Grate** (Controls 0.00 cfs) -**5=Orifice/Grate** (Orifice Controls 0.79 cfs @ 9.09 fps)

-7=Orifice/Grate (Orifice Controls 0.83 cfs @ 4.23 fps)

Secondary OutFlow Max=0.81 cfs @ 12.21 hrs HW=731.90' TW=722.32' (Dynamic Tailwater) -**2=Culvert** (Passes 0.81 cfs of 3.49 cfs potential flow) **4=Orifice/Grate** (Controls 0.00 cfs)

-6=Orifice/Grate (Orifice Controls 0.40 cfs @ 9.09 fps)

-8=Orifice/Grate (Orifice Controls 0.42 cfs @ 4.23 fps)

Summary for Pond RG40: Rain Garden 40

 9,525 sf, 65.00% impervises,

 0.98 cfs @
 12.07 hrs, Volume=

 3,113 cf

 0.65 cfs @
 12.16 hrs, Volume=

 3,105 cf, Atten= 33%, Lag= 5.4 min

 3,105 cf

 Inflow Area = 9,525 sf, 65.00% Impervious, Inflow Depth > 3.92" for 10YearMass event Inflow = Outflow = Primary = Routed to Link AP2-P : AP2-P

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 733.35' @ 12.16 hrs Surf.Area= 811 sf Storage= 446 cf

Plug-Flow detention time= 14.5 min calculated for 3,105 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (801.9 - 789.1)

Volume	Invert	Avail.Storage	Storage Description
#1	729.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	729.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	732.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	733.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage
			-

Device	Routing	Invert	Outlet Devices
#1	Primary	729.75'	8.0" Round Culvert
	-		L= 140.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 729.75' / 728.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	733.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	729.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	732.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.65 cfs @ 12.16 hrs HW=733.34' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 0.65 cfs of 2.01 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.20 cfs @ 9.01 fps)

-4=Orifice/Grate (Orifice Controls 0.45 cfs @ 5.21 fps)

Summary for Pond RG41: Rain Garden 41

Inflow Area =	7,525 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event
Inflow =	0.77 cfs @ 12.07 hrs, Volume=	2,460 cf
Outflow =	0.49 cfs @ 12.17 hrs, Volume=	2,307 cf, Atten= 36%, Lag= 5.9 min
Discarded =	0.05 cfs @ 12.17 hrs, Volume=	894 cf
Primary =	0.44 cfs @ 12.17 hrs, Volume=	1,412 cf
Routed to Link	AP2-P : AP2-P	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.07' @ 12.17 hrs Surf.Area= 870 sf Storage= 524 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 35.8 min (824.9 - 789.1)

Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 34

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	47 cf	
#2	728.25'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	731.25'	30 cf	
#4	731.50'	373 cf	
		720 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Discarded	728.25' 2.4 '	10 in/hr Exfiltration over Surface area
#2	Primary	730.25' 6.0 '	" Round Culvert
		L= :	26.0' CPP, square edge headwall, Ke= 0.500
		Inle	t / Outlet Invert= 730.25' / 730.00' S= 0.0096 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	732.00' 6.0 '	"Horiz. Orifice/Grate C= 0.600
		Lim	ited to weir flow at low heads
#4	Device 2	730.25' 2.0 '	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	731.00' 3.0 '	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.05 cfs @ 12.17 hrs HW=732.05' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.43 cfs @ 12.17 hrs HW=732.05' TW=0.00' (Dynamic Tailwater)

-2=Culvert (Passes 0.43 cfs of 1.04 cfs potential flow)

-3=Orifice/Grate (Weir Controls 0.07 cfs @ 0.77 fps)

-4=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.32 fps)

-5=Orifice/Grate (Orifice Controls 0.23 cfs @ 4.64 fps)

Summary for Pond RG42: Rain Garden 42

Inflow Area =	12,715 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event
Inflow =	1.30 cfs @ 12.07 hrs, Volume=	4,156 cf
Outflow =	0.86 cfs @ 12.17 hrs, Volume=	3,640 cf, Atten= 34%, Lag= 5.9 min
Discarded =	0.04 cfs @ 12.17 hrs, Volume=	804 cf
Primary =	0.82 cfs @ 12.17 hrs, Volume=	2,836 cf
Routed to Link	AP2-P : AP2-P	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.12' @ 12.17 hrs Surf.Area= 1,615 sf Storage= 1,115 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 31.8 min (820.9 - 789.1)

AP2

Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 35

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	212 cf	18.0" Round Pipe Storage x 2 Inside #2
			L= 60.0'
#2	728.25'	491 cf	8.00'W x 60.00'L x 3.00'H Soil Media and Gravel
	704 05	00 5	1,440 cf Overall - 212 cf Embedded = 1,228 cf x 40.0% Voids
#3	731.25'	60 cf	8.00'W x 60.00'L x 0.25'H Mulch
#1	724 501	601 of	120 cf Overall x 50.0% Voids
#4	731.50'	02101	8.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		1,385 cf	Total Available Storage
Davias	Douting	Invert Out	at Daviaga
Device	Routing	Invert Outl	et Devices
#1	Discorded	700 05' 4 00	0 in/hr Exfiltration over Surface area

#1	Discarded	728.25'	1.020 in/hr Exfiltration over Surface area
#2	Primary	730.25'	8.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 730.25' / 730.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Device 2	732.00'	8.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2	730.25'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	731.00'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.17 hrs HW=732.11' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.78 cfs @ 12.17 hrs HW=732.11' TW=0.00' (Dynamic Tailwater)

2=Culvert (Passes 0.78 cfs of 2.07 cfs potential flow)

-3=Orifice/Grate (Weir Controls 0.24 cfs @ 1.07 fps)

-4=Orifice/Grate (Orifice Controls 0.31 cfs @ 6.34 fps)

-5=Orifice/Grate (Orifice Controls 0.23 cfs @ 4.77 fps)

Summary for Pond RG43-48: Rain Garden 43-48

Inflow Area =	39,875 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event
Inflow =	4.08 cfs @ 12.07 hrs, Volume=	13,034 cf
Outflow =	2.21 cfs @ 12.20 hrs, Volume=	11,422 cf, Atten= 46%, Lag= 7.8 min
Discarded =	0.14 cfs @ 12.20 hrs, Volume=	2,928 cf
Primary =	1.72 cfs @ 12.20 hrs, Volume=	7,079 cf
Routed to Link	AP2-P : AP2-P	
Secondary =	0.34 cfs @ 12.20 hrs, Volume=	1,416 cf
Routed to Read	ch SW2.1 : Swale RG2.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 733.42' @ 12.20 hrs Surf.Area= 6,080 sf Storage= 3,463 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 31.3 min (820.4 - 789.1)

AP2

AP2	ed by Mcclure	Engineeer	ina	Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023
				ydroCAD Software Solutions LLC Page 36
<u></u>	<u> </u>			
Volume	Invert	Avail.Stor	rage	Storage Description
#1	729.75'	28	33 cf	12.0" Round Pipe Storage x 6 Inside #2 L= 60.0'
#2	729.75'	2,04	7 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 6 5,400 cf Overall - 283 cf Embedded = 5,117 cf x 40.0% Voids
#3	732.75'	22	25 cf	5.00'W x 60.00'L x 0.25'H Mulch x 6
				450 cf Overall x 50.0% Voids
#4	733.00'			5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 6
		5,16	67 cf	Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Discarded	729.75') in/hr Exfiltration over Surface area
#2	Primary	731.75'		Round Culvert X 5.00
			L= 1().0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 731.75' / 731.65' S= 0.0100 '/' Cc= 0.900
				013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Secondary	731.75'		Round Culvert
	coordary			0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 731.75' / 731.65' S= 0.0100 '/' Cc= 0.900
				013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#4	Device 2	733.50'		Horiz. Orifice/Grate X 5.00 C= 0.600
	During			ed to weir flow at low heads
#5	Device 3	733.50'		Horiz. Orifice/Grate C= 0.600 ed to weir flow at low heads
#6	Device 2	731.75'		Vert. Orifice/Grate X 5.00 C= 0.600
#0	Device 2	101.10	-	ed to weir flow at low heads
#7	Device 3	731.75'		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#8	Device 2	732.50'		Vert. Orifice/Grate X 5.00 C= 0.600 ed to weir flow at low heads
#9	Device 3	732.50'		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.14 cfs @ 12.20 hrs HW=733.42' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=1.72 cfs @ 12.20 hrs HW=733.42' TW=0.00' (Dynamic Tailwater)

-2=Culvert (Passes 1.72 cfs of 5.64 cfs potential flow)

4=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Orifice Controls 0.66 cfs @ 6.07 fps)

-8=Orifice/Grate (Orifice Controls 1.06 cfs @ 4.30 fps)

Secondary OutFlow Max=0.34 cfs @ 12.20 hrs HW=733.42' TW=722.32' (Dynamic Tailwater) -3=Culvert (Passes 0.34 cfs of 1.13 cfs potential flow)

5=Orifice/Grate (Controls 0.00 cfs)

-7=Orifice/Grate (Orifice Controls 0.13 cfs @ 6.07 fps)

-9=Orifice/Grate (Orifice Controls 0.21 cfs @ 4.30 fps)

Summary for Pond RG49-50: Rain Gardens 49,50

Inflow Area =	10,410 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event
Inflow =	1.07 cfs @ 12.07 hrs, Volume=	3,403 cf
Outflow =	0.64 cfs @ 12.18 hrs, Volume=	2,887 cf, Atten= 40%, Lag= 6.4 min
Discarded =	0.04 cfs @ 12.18 hrs, Volume=	927 cf
Primary =	0.60 cfs @ 12.18 hrs, Volume=	1,960 cf
Routed to Read	h SW2.1 : Swale RG2.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 731.68' @ 12.18 hrs Surf.Area= 1,895 sf Storage= 969 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 36.9 min (825.9 - 789.1)

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
#2	728.25'	682 cf	L= 60.0' 5.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 2
<i>π</i> ∠	120.20	002 01	1,800 cf Overall - 94 cf Embedded = $1,706$ cf x 40.0% Voids
#3	731.25'	75 cf	5.00'W x 60.00'L x 0.25'H Mulch x 2
		074 6	150 cf Overall x 50.0% Voids
#4	731.50'	8/1 ct	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	728.25'	1.020 in/hr Exfiltration over Surface area
#2	Primary	730.25'	6.0" Round Culvert X 2.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 730.25' / 730.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	732.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	730.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 2	731.00'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.18 hrs HW=731.68' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.59 cfs @ 12.18 hrs HW=731.68' TW=722.33' (Dynamic Tailwater) 2=Culvert (Passes 0.59 cfs of 2.05 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -4=Orifice/Grate (Orifice Controls 0.24 cfs @ 5.58 fps) -5=Orifice/Grate (Orifice Controls 0.35 cfs @ 3.58 fps)

Summary for Pond RG52-54: Rain Garden Lots 52,53,54

Inflow Are	a =	28,140 sf, 53.72% Impervious, Inflow Depth > 3.12" for 10Year	Mass event
Inflow	=	2.36 cfs @ 12.08 hrs, Volume= 7,321 cf	
Outflow	=	1.76 cfs @ 12.15 hrs, Volume= 7,297 cf, Atten= 25%, Lag	g= 4.5 min
Primary	=	1.76 cfs @ 12.15 hrs, Volume= 7,297 cf	
Routed	l to Link	P2-P : AP2-P	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 690.06' @ 12.15 hrs Surf.Area= 2,209 sf Storage= 1,086 cf

Plug-Flow detention time= 14.9 min calculated for 7,281 cf (99% of inflow) Center-of-Mass det. time= 12.9 min (827.4 - 814.5)

Volume	Invert	Avail.Storage	Storage Description
#1	686.75'	141 cf	12.0" Round Pipe Storage x 3 Inside #2
			L= 60.0'
#2	686.75'	807 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel × 3
			2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids
#3	689.75'	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
			180 cf Overall x 50.0% Voids
#4	690.00'	1,120 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3
		2,159 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	686.75'	6.0" Round Culvert X 3.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 686.75' / 686.70' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	690.50'	6.0" Horiz. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	686.75'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	689.00'	4.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=1.76 cfs @ 12.15 hrs HW=690.06' TW=0.00' (Dynamic Tailwater)

-**1=Culvert** (Passes 1.76 cfs of 4.96 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.57 cfs @ 8.65 fps)

-4=Orifice/Grate (Orifice Controls 1.19 cfs @ 4.56 fps)

Summary for Pond RG55: Rain Garden 55

 Inflow Area =
 5,450 sf, 65.00% Impervious, Inflow Depth > 3.92" for 10YearMass event

 Inflow =
 0.56 cfs @
 12.07 hrs, Volume=
 1,781 cf

 Outflow =
 0.41 cfs @
 12.16 hrs, Volume=
 1,775 cf, Atten= 27%, Lag= 5.5 min

 Primary =
 0.41 cfs @
 12.16 hrs, Volume=
 1,775 cf

 Routed to Link AP2-P : AP2-P
 12.16 hrs, Volume=
 1,775 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Peak Elev= 688.48' @ 12.16 hrs Surf.Area= 240 sf Storage= 326 cf

Plug-Flow detention time= 18.2 min calculated for 1,771 cf (99% of inflow) Center-of-Mass det. time= 15.9 min (805.0 - 789.1)

Volume	Invert	Avail.Storage	Storage Description
#1	685.75'	106 cf	18.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	685.75'	246 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	688.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	689.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		755 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	685.75'	6.0" Round Culvert
	•		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 685.75' / 685.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	689.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	685.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	688.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.39 cfs @ 12.16 hrs HW=688.45' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 0.39 cfs of 1.48 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.17 cfs @ 7.78 fps)

-4=Orifice/Grate (Orifice Controls 0.22 cfs @ 2.54 fps)

Summary for Link AP2-P: AP2-P

Inflow Are	a =	550,840 sf, 23.14% Impervious,	Inflow Depth > 2.16"	for 10YearMass event
Inflow	=	20.93 cfs @ 12.17 hrs, Volume=	99,276 cf	
Primary	=	20.93 cfs @ 12.17 hrs, Volume=	99,276 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Subcatchment P2.1: To Wetland A (A56-A87)

Runoff = 14.02 cfs @ 12.14 hrs, Volume= 49,797 cf, Depth> 2.94" Routed to Link AP2-P : AP2-P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN E	Description						
1	42,635	70 V	70 Woods, Good, HSG C						
	45,615	74 >	75% Gras	s cover, Go	bod, HSG C				
	10,360	55 V	Voods, Go	od, HSG B					
	3,340				ood, HSG B				
	1,060	98 L	Inconnecte	ed roofs, HS	SG B				
2	03,010	70 V	Veighted A	verage					
2	01,950	g	9.48% Pei	vious Area					
	1,060			ervious Are					
	1,060	1	00.00% U	nconnected	1				
т	1 11.	0	\/.l!t.	0	Description				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)								
		<u>(ft/ft)</u>	(ft/sec)	(cfs)					
5.1	50	0.2000	0.16	(015)	Sheet Flow,				
		0.2000	0.16	(015)	Woods: Light underbrush n= 0.400 P2= 3.00"				
5.1 0.7	50 90			(CIS)	Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow,				
0.7	90	0.2000	0.16 2.24	(015)	Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps				
		0.2000	0.16	(015)	Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps Shallow Concentrated Flow,				
0.7	90	0.2000	0.16 2.24	(015)	Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps				

Summary for Subcatchment P2.10: To RG4.1

Runoff = 2.72 cfs @ 12.10 hrs, Volume= 8,954 cf, Depth> 4.04" Routed to Reach SW2.1 : Swale RG2.1

	Area (sf)	CN	Description
*	12,070	90	Residential Lots, 65% imp, HSG C
	2,925	70	Woods, Good, HSG C
	11,575	74	>75% Grass cover, Good, HSG C
	26,570	81	Weighted Average
	18,725		70.47% Pervious Area
	7,846		29.53% Impervious Area

AP2					Postdevelopment Type III 24-hr 25YearMass Rainfall=6.18"
Prepare	d by Mco	clure Eng	gineeering	1	Printed 5/1/2023
HydroCA	<u>) 10.20-</u>	2g_s/n 03	362 © 202	2 HydroCAD	Software Solutions LLC Page 41
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
(min) 6.2	(feet) 50	(ft/ft) 0.1200	(ft/sec) 0.13	(cfs)	Sheet Flow,

7.2 150 Total

Summary for Subcatchment P2.11: Lots 48-50

Woodland Kv= 5.0 fps

4,354 cf, Depth> 5.02"

Runoff = 1.35 cfs @ 12.07 hrs, Volume= Routed to Pond RG49-50 : Rain Gardens 49,50

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"

_	A	rea (sf)	CN I	Description						
*		10,410	90	90 Residential Lots, 65% imp, HSG C						
		3,644	;	35.00% Pervious Area						
		6,767	(65.00% Impervious Area						
	_		~		• •	— • • • •				
	TC	Length	Slope		Capacity	1				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0					Direct Entry,				

Summary for Subcatchment P2.12: Lot 54

Runoff = 0.70 cfs @ 12.07 hrs, Volume= 2,279 cf, Depth> 5.02" Routed to Pond RG55 : Rain Garden 55

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"

	A	rea (sf)	CN I	Description					
*		5,450	90	90 Residential Lots, 65% imp, HSG C					
		1,908		35.00% Pervious Area					
		3,543		65.00% Imp	pervious Ar	rea			
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	5.0					Direct Entry,			

Summary for Subcatchment P2.13: Lot 14

Runoff = 0.87 cfs @ 12.07 hrs, Volume= 2,809 cf, Depth> 4.91" Routed to Pond RG14 : Rain Garden 14

Α	rea (sf)	CN	Description				
	5,915	90	1/8 acre lots	s, 65% imp	, HSG C		
	955	85	1/8 acre lots	s, 65% imp	, HSG B		
	6,870	89	Neighted A	verage			
	2,405		35.00% Pervious Area				
	4,466		65.00% Imp	pervious Ar	ea		
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description		
5.0			, <i>r</i>		Direct Entry,		

Summary for Subcatchment P2.14: Lots 30-33

Runoff	=	2.78 cfs @	12.07 hrs,	Volume=	8,994 cf,	Depth>	5.02"
Routed	to Pond	I RG31-33 : F	≀ain Garden	s 31,32,33		-	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"

_	A	rea (sf)	CN I	Description					
*		21,505	90 I	90 Residential Lots, 65% imp, HSG C					
		7,527	3	35.00% Pervious Area					
		13,978	6	65.00% Impervious Area					
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description			
_	5.0					Direct Entry,			

Summary for Subcatchment P2.2: Lots 42-48

Runoff = 5.16 cfs @ 12.07 hrs, Volume= 16,676 cf, Depth> 5.02" Routed to Pond RG43-48 : Rain Garden 43-48

	A	rea (sf)	CN [Description				
*		39,875	90 F	Residential Lots, 65% imp, HSG C				
		13,956	-		vious Area			
		25,919	6	65.00% Imp	pervious Ar	ea		
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
	5.0					Direct Entry,		

Summary for Subcatchment P2.3: Lots 41-42

Runoff = 1.64 cfs @ 12.07 hrs, Volume= 5,318 cf, Depth> 5.02" Routed to Pond RG42 : Rain Garden 42

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Area (s	f) CN	CN Description					
12,71	5 90	90 1/8 acre lots, 65% imp, HSG C					
4,45	0	35.00% Pervious Area					
8,26	5	65.00% Impervious Area					
Tc Leng (min) (fe	,	ope t/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		
		~		. f O			

Summary for Subcatchment P2.4: Lots 40-41

Runoff = 0.97 cfs @ 12.07 hrs, Volume= 3,147 cf, Depth> 5.02" Routed to Pond RG41 : Rain Garden 41

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"

ı (sf) (CN D	Description					
,525	90 1	1/8 acre lots, 65% imp, HSG C					
,	35.00% Pervious Area						
,031	05.00 % Impervious Area						
ength	Slope	Velocity	Capacity	Description			
(feet)	(ft/ft)	(ft/sec)	(cfs)				
				Direct Entry,			
	<u>(sf)</u> , <u>525</u> ,634 ,891 ength (feet)	, <u>525 90 1</u> ,634 3 ,891 6 ength Slope	525 90 1/8 acre lots 634 35.00% Per 891 65.00% Imp ength Slope Velocity	525901/8 acre lots, 65% imp,63435.00% Pervious Area,89165.00% Impervious AreaengthSlopeVelocityCapacity			

Summary for Subcatchment P2.5: Lots 39-40

Runoff = 1.23 cfs @ 12.07 hrs, Volume= 3,9 Routed to Pond RG40 : Rain Garden 40

3,984 cf, Depth> 5.02"

Area (sf)	CN	Description
9,525	90	1/8 acre lots, 65% imp, HSG C
3,334		35.00% Pervious Area
6,191		65.00% Impervious Area

AP2 Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Postdevelopment <i>Type III 24-hr 25YearMass Rainfall=6.18</i> " Printed 5/1/2023 olutions LLC Page 44						
Tc Length Slope Velocity Capacity Descriptio (min) (feet) (ft/ft) (ft/sec) (cfs)	n						
5.0 Direct En	try,						
Summary for Subcatchmer	nt P2.6: Lots 33-39						
Runoff = 5.86 cfs @ 12.07 hrs, Volume= Routed to Pond RG34-39 : Rain Gardens 34-39	18,939 cf, Depth> 5.02"						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Tin Type III 24-hr 25YearMass Rainfall=6.18"	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"						
Area (sf) CN Description							
* 45,285 90 Residential Lots, 65% imp, HSG C)						
15,850 35.00% Pervious Area 29,435 65.00% Impervious Area							
Tc Length Slope Velocity Capacity Descriptio (min) (feet) (ft/ft) (ft/sec) (cfs)	n						
5.0 Direct En	try,						
Summary for Subcatchment P2.7	: Upgradient Lots 35-39						
Runoff = 2.41 cfs @ 12.28 hrs, Volume= Routed to Pond IT-35/39 : Interceptor Trench Lots 35-39	11,134 cf, Depth> 2.94"						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"							
Area (sf) CN Description							
4,550 74 >75% Grass cover, Good, HSG C 40,950 70 Woods, Good, HSG C							
45,50070Weighted Average45,500100.00% Pervious Area							
To Longth Clone Malacity Consolity Deceminitie	_						

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	(11111)	(ieel)	(1011)	(11/360)	(015)	
	12.7	50	0.0200	0.07		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	7.1	560	0.0700	1.32		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
	~ 4		~ ~ ~ ~ ~ ~	4.05		
	0.1	20	0.5000	4.95		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	40.0	000	T . 4 . 1			

19.9 630 Total

Summary for Subcatchment P2.8: Upgradient Lots 30-35

Runoff = 5.25 cfs @ 12.23 hrs, Volume= 22,370 cf, Depth> 3.03" Routed to Pond IT-30/35 : Interceptor Trench Lots 30-35

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN E	Description		
	11,085			,	bod, HSG C
	77,375	70 V	Voods, Go	<u>od, HSG C</u>	
	88,460	71 V	Veighted A	verage	
	88,460	1	00.00% Pe	ervious Are	а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12.7	50	0.0200	0.07		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
3.5	280	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.1	30	0.5000	4.95		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
16.3	360	Total			

Summary for Subcatchment P2.9: Lots 51-53

Runoff = 3.13 cfs @ 12.07 hrs, Volume= 9,733 cf, Depth> 4.15" Routed to Pond RG52-54 : Rain Garden Lots 52,53,54

Area	(sf) CN	Description					
2,4	130 90	1/8 acre lots, 65% imp, HSG C					
20,8	825 85	1/8 acre lots, 65% imp, HSG B					
3,6	605 70	Woods, Good, HSG C					
1,2	280 55	Woods, Good, HSG B					
28,1	140 82	Weighted Average					
13,0)24	46.28% Pervious Area					
15,1	116	53.72% Impervious Area					
Tc Lei	ngth Slo						
<u>(min)</u> (f	eet) (ft	/ft) (ft/sec) (cfs)					
5.0		Direct Entry,					

Summary for Reach SW2.1: Swale RG2.1

Inflow Area = 58,485 sf, 48.88% Impervious, Inflow Depth > 5.96" for 25YearMass event Inflow = 6.87 cfs @ 12.15 hrs, Volume= 29,023 cf Outflow = 6.83 cfs @ 12.15 hrs, Volume= 29,019 cf, Atten= 1%, Lag= 0.2 min Routed to Pond RG2.1 : Rain Garden 2.1 Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Max. Velocity= 9.37 fps, Min. Travel Time= 0.2 min Avg. Velocity = 3.15 fps, Avg. Travel Time= 0.6 min

Peak Storage= 87 cf @ 12.15 hrs Average Depth at Peak Storage= 0.40', Surface Width= 2.61' Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 46.26 cfs

1.00' x 1.00' deep channel, n= 0.025 Earth, clean & winding Side Slope Z-value= 2.0 '/' Top Width= 5.00' Length= 120.0' Slope= 0.1500 '/' Inlet Invert= 722.00', Outlet Invert= 704.00'

Summary for Pond IT-30/35: Interceptor Trench Lots 30-35

Inflow Are	a =	88,460 sf,	0.00% Impervious,	Inflow Depth > 3.03" for 25YearMass event		
Inflow	=	5.25 cfs @	12.23 hrs, Volume=	22,370 cf		
Outflow	=	5.08 cfs @	12.27 hrs, Volume=	22,274 cf, Atten= 3%, Lag= 2.7 min		
Primary	=	5.08 cfs @	12.27 hrs, Volume=	22,274 cf		
Routed to Link AP2-P : AP2-P						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 733.36' @ 12.27 hrs Surf.Area= 1,470 sf Storage= 905 cf

Plug-Flow detention time= 7.2 min calculated for 22,228 cf (99% of inflow) Center-of-Mass det. time= 4.7 min (846.7 - 842.0)

Volume	Invert	Avail.Storage	Storage Description
#1	732.00'	1,696 c	5 3.00'W x 490.00'L x 3.00'H Prismatoid
			4,410 cf Overall - 171 cf Embedded = 4,239 cf x 40.0% Voids
#2	732.00'	171 c	f 8.0" Round Pipe Storage Inside #1
			L= 490.0'
		1,867 c	f Total Available Storage
Device	Routing	Invert Ou	itlet Devices
#1	Primary	732.00' 15	.0" Round Culvert
	2	L=	250.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 732.00' / 724.00' S= 0.0320 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=5.03 cfs @ 12.27 hrs HW=733.35' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 5.03 cfs @ 4.10 fps)

Summary for Pond IT-35/39: Interceptor Trench Lots 35-39

 Inflow Area =
 45,500 sf, 0.00% Impervious, Inflow Depth > 2.94" for 25YearMass event

 Inflow =
 2.41 cfs @
 12.28 hrs, Volume=
 11,134 cf

 Outflow =
 2.26 cfs @
 12.36 hrs, Volume=
 11,073 cf, Atten= 6%, Lag= 4.6 min

 Primary =
 2.26 cfs @
 12.36 hrs, Volume=
 11,073 cf

 Routed to Link AP2-P : AP2-P
 11,073 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 735.16' @ 12.36 hrs Surf.Area= 1,200 sf Storage= 640 cf

Plug-Flow detention time= 9.3 min calculated for 11,050 cf (99% of inflow) Center-of-Mass det. time= 6.2 min (853.3 - 847.2)

Volume	Invert	Avail.Storage	Storage Description
#1	734.00'	1,384 cf	3.00'W x 400.00'L x 3.00'H Prismatoid
			3,600 cf Overall - 140 cf Embedded = 3,460 cf x 40.0% Voids
#2	734.00'	140 cf	8.0" Round Pipe Storage Inside #1
			L= 400.0'
		1,524 cf	Total Available Storage
Device	Routing	Invert Outl	et Devices
#1	Primary	734 00' 10 0	" Round Culvert

#1	Primary	734.00'	10.0" Round Culvert
			L= 80.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 734.00' / 733.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=2.26 cfs @ 12.36 hrs HW=735.16' TW=0.00' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.26 cfs @ 4.14 fps)

Summary for Pond RG14: Rain Garden 14

 Inflow Area =
 6,870 sf, 65.00% Impervious, Inflow Depth > 4.91" for 25YearMass event

 Inflow =
 0.87 cfs @
 12.07 hrs, Volume=
 2,809 cf

 Outflow =
 0.56 cfs @
 12.17 hrs, Volume=
 2,795 cf, Atten= 36%, Lag= 5.6 min

 Primary =
 0.56 cfs @
 12.17 hrs, Volume=
 2,795 cf

 Routed to Pond RG2.1 : Rain Garden 2.1
 2.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 704.76' @ 12.17 hrs Surf.Area= 789 sf Storage= 396 cf

Plug-Flow detention time= 16.7 min calculated for 2,795 cf (100% of inflow) Center-of-Mass det. time= 13.6 min (799.5 - 785.9)

Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023 Page 48

Volume	Invert	Avail Storago	Storage Description
		Avail.Storage	
#1	702.00'	47 cf	
			L= 60.0'
#2	702.00'	269 cf	
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	704.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	704.50'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage
			5
Device	Routing	Invert Out	let Devices
#1	Primary	702.00' 6.0'	' Round Culvert
	j	= 1	75.0' CPP, square edge headwall, Ke= 0.500
			t / Outlet Invert= 702.00' / 702.00' S= 0.0000 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		'Horiz. Orifice/Grate C= 0.600
<i>"</i> –	Borneo		ited to weir flow at low heads
#3	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	-	Vert. Orlice/Grate C= 0.600 Limited to weir flow at low heads
$\pi - 1$	Device I	103.13 4.0	Vert. Office/Grate O- 0.000 Linnied to wen now at 10W fields

Primary OutFlow Max=0.55 cfs @ 12.17 hrs HW=704.75' TW=702.23' (Dynamic Tailwater)

1=Culvert (Passes 0.55 cfs of 0.87 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.17 cfs @ 7.65 fps)

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

-4=Orifice/Grate (Orifice Controls 0.38 cfs @ 4.41 fps)

Summary for Pond RG2.1: Rain Garden 2.1

Inflow Area =	65,355 sf, 50.58% Impervious,	Inflow Depth > 5.84" for 25YearMass event
Inflow =	7.39 cfs @ 12.15 hrs, Volume=	31,815 cf
Outflow =	2.87 cfs @ 12.60 hrs, Volume=	30,160 cf, Atten= 61%, Lag= 26.7 min
Discarded =	0.74 cfs @ 12.60 hrs, Volume=	17,724 cf
Primary =	2.13 cfs @ 12.60 hrs, Volume=	12,436 cf
Routed to L	ink AP2-P : AP2-P	
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf
Routed to L	ink AP2-P : AP2-P	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 702.95' @ 12.60 hrs Surf.Area= 13,275 sf Storage= 9,939 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 73.4 min (874.6 - 801.2)

AP2 Prepared by Mcclure Engineeering

Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023 Page 49

TIYUIUCA	De 10.20-29 5		2022 1	TydrocAD Software Solutions LEC Fage 49	
Volume	Invert	Avail.Sto	rage	Storage Description	
#1	698.75'	67 cf		12.0" Round Pipe Storage Inside #2	
#2	698.75'	5,073 cf			
#3	701.75'	531 cf			
#4	702.00'	9,62	23 cf	1,063 cf Overall x 50.0% Voids 50.00'W x 85.00'L x 2.00'H Ponding Z=2.0	
		15,29	94 cf	Total Available Storage	
Device	Routing	Invert	Outl	et Devices	
#1	Primary	701.00'	-	" Round Culvert	
			Inlet	0.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 701.00' / 699.65' S= 0.1350 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	
#2	Device 1	703.00'	10.0	"Horiz. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads	
#3	Device 1	701.00'	2.0"	Vert. Orifice/Grate X 2.00 C= 0.600	
#4	Device 1	701.75'		ted to weir flow at low heads ' Vert. Orifice/Grate X 2.00 C= 0.600	
<i>1</i> 7-1	Device	101.10		ted to weir flow at low heads	
#5	Secondary	703.75'	703.75' 10.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64		
#6	Discarded	698.75'		0 in/hr Exfiltration over Surface area	
Discarded OutFlow Max=0.74 cfs @ 12.60 hrs. HW=702.95' (Free Discharge)					

Discarded OutFlow Max=0.74 cfs @ 12.60 hrs HW=702.95' (Free Discharge) **G=Exfiltration** (Exfiltration Controls 0.74 cfs)

Primary OutFlow Max=2.13 cfs @ 12.60 hrs HW=702.95' TW=0.00' (Dynamic Tailwater) 1=Culvert (Passes 2.13 cfs of 4.55 cfs potential flow) 2=Orifice/Grate (Controls 0.00 cfs) 3=Orifice/Grate (Orifice Controls 0.29 cfs @ 6.57 fps) 4=Orifice/Grate (Orifice Controls 1.84 cfs @ 4.68 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=698.75' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG31-33: Rain Gardens 31,32,33

Inflow Are	a =	21,505 sf, 65.00% Impervious	s, Inflow Depth > 5.02" for 25YearMass event				
Inflow	=	2.78 cfs @ 12.07 hrs, Volume=	= 8,994 cf				
Outflow	=	1.84 cfs @ 12.17 hrs, Volume=	= 8,970 cf, Atten= 34%, Lag= 5.8 min				
Primary	=	1.84 cfs @ 12.17 hrs, Volume=	= 8,970 cf				
Routed to Reach SW2.1 : Swale RG2.1							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.11' @ 12.17 hrs Surf.Area= 2,648 sf Storage= 1,627 cf

Plug-Flow detention time= 16.9 min calculated for 8,970 cf (100% of inflow) Center-of-Mass det. time= 15.2 min (797.7 - 782.5)

AP2

Prepared by Mcclure Engineeering

HvdroCAD® 10.20-2g s/n 03362 © 2022 HvdroCAD Software Solutions LLC

AP2 Type III 24-hr 25YearMass Rainfall=6.18" Prepared by Mcclure Engineeering Printed 5/1/2023 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 50

Volume	Invert	Avail.Sto	rage	Storage Description	
#1	728.25'	141 cf		12.0" Round Pipe Storage x 3 Inside #2	
#2	728.25'	807 cf		L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 3 2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids	
#3	731.25'	ę	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3	
				180 cf Overall x 50.0% Voids	
#4	731.50'	1,12	20 ct	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3	
		2,15	59 cf	Total Available Storage	
				C C	
Device	Routing	Invert	Outl	et Devices	
#1	Primary	728.25'	6.0"	Round Culvert X 3.00	
	·		L= 1	0.0' CPP, square edge headwall, Ke= 0.500	
				/ Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900	
			n= 0	0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf	
#2	Device 1	732.00'		Horiz. Orifice/Grate X 3.00 C= 0.600	
			Limited to weir flow at low heads		
#3	Device 1	728.25'			
			Limited to weir flow at low heads		
#4	Device 1	731.00'	3.0"	Vert. Orifice/Grate X 3.00 C= 0.600	
				ted to weir flow at low heads	

Primary OutFlow Max=1.77 cfs @ 12.17 hrs HW=732.10' TW=722.40' (Dynamic Tailwater)

-1=Culvert (Passes 1.77 cfs of 5.38 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.46 cfs @ 1.01 fps)

-3=Orifice/Grate (Orifice Controls 0.61 cfs @ 9.34 fps)

-4=Orifice/Grate (Orifice Controls 0.70 cfs @ 4.75 fps)

Summary for Pond RG34-39: Rain Gardens 34-39

45,285 sf, 65.00% Impervious, Inflow Depth > 5.02" for 25YearMass event Inflow Area = Inflow 5.86 cfs @ 12.07 hrs, Volume= 18,939 cf = Outflow = 4.31 cfs @ 12.16 hrs, Volume= 18,889 cf, Atten= 26%, Lag= 5.3 min 2.87 cfs @ 12.16 hrs, Volume= Primary = 12,593 cf Routed to Link AP2-P : AP2-P 1.44 cfs @ 12.16 hrs, Volume= 6,296 cf Secondary = Routed to Reach SW2.1 : Swale RG2.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.14' @ 12.16 hrs Surf.Area= 5,348 sf Storage= 3,331 cf

Plug-Flow detention time= 16.8 min calculated for 18,889 cf (100% of inflow) Center-of-Mass det. time= 15.1 min (797.6 - 782.5)

	Prepared by Mcclure Engineeering Printed 5/1/2023					
	10@ 10.20-29 3/			-		
Volume	Invert	Avail.Sto		Storage Description		
#1	728.25'	28	33 cf	12.0" Round Pipe Storage x 6 Inside #2 L= 60.0'		
#2	728.25'	1,61	15 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 6 4,320 cf Overall - 283 cf Embedded = 4,037 cf x 40.0% Voids		
#3	731.25'	18	30 cf			
#4	731.50'	2,24	40 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 6		
				Total Available Storage		
		.,-		· · · · · · · · · · · · · · · · · · ·		
Device	Routing	Invert	Outl	et Devices		
#1	Primary	728.25'	6.0"	Round Culvert X 4.00		
				0.0' CPP, square edge headwall, Ke= 0.500		
				/ Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900		
	0			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Secondary	728.25'		Round Culvert X 2.00 0.0' CPP, square edge headwall, Ke= 0.500		
				/ Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900		
				0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#3	Device 1	732.00'		Horiz. Orifice/Grate X 4.00 C= 0.600		
				ted to weir flow at low heads		
#4	Device 2	732.00'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600		
				ted to weir flow at low heads		
#5	Device 1	728.25'		Vert. Orifice/Grate X 4.00 C= 0.600		
		700 051		ted to weir flow at low heads		
#6	Device 2	728.25'		Vert. Orifice/Grate X 2.00 C= 0.600		
#7	Device 1	731.00'		ted to weir flow at low heads Vert. Orifice/Grate X 4.00 C= 0.600		
#1	Device	731.00		ted to weir flow at low heads		
#8	Device 2	731.00'		Vert. Orifice/Grate X 2.00 C= 0.600		
	2011002	101.00		ted to weir flow at low heads		
Primary	Primary OutFlow Max=2.76 cfs @ 12.16 hrs HW=732.13' TW=0.00' (Dynamic Tailwater)					

Primary OutFlow Max=2.76 cfs @ 12.16 hrs HW=732.13' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 2.76 cfs of 7.21 cfs potential flow)

-3=Orifice/Grate (Weir Controls 0.99 cfs @ 1.19 fps)

-5=Orifice/Grate (Orifice Controls 0.82 cfs @ 9.38 fps)

-7=Orifice/Grate (Orifice Controls 0.95 cfs @ 4.83 fps)

Secondary OutFlow Max=1.38 cfs @ 12.16 hrs HW=732.13' TW=722.40' (Dynamic Tailwater) 2=Culvert (Passes 1.38 cfs of 3.60 cfs potential flow) 4=Orifice/Grate (Weir Controls 0.49 cfs @ 1.19 fps)

-6=Orifice/Grate (Orifice Controls 0.41 cfs @ 9.38 fps)

-8=Orifice/Grate (Orifice Controls 0.47 cfs @ 4.83 fps)

Summary for Pond RG40: Rain Garden 40

 Inflow Area =
 9,525 sf, 65.00% Impervious, Inflow Depth > 5.02" for 25YearMass event

 Inflow =
 1.23 cfs @
 12.07 hrs, Volume=
 3,984 cf

 Outflow =
 0.92 cfs @
 12.16 hrs, Volume=
 3,974 cf, Atten= 25%, Lag= 5.0 min

 Primary =
 0.92 cfs @
 12.16 hrs, Volume=
 3,974 cf

 Routed to Link AP2-P : AP2-P
 AP2-P
 3,974 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 733.62' @ 12.16 hrs Surf.Area= 885 sf Storage= 546 cf

Plug-Flow detention time= 14.4 min calculated for 3,974 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (795.3 - 782.5)

Volume	Invert	Avail.Storage	Storage Description
#1	729.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	729.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	732.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	733.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage
			-

Device	Routing	Invert	Outlet Devices
#1	Primary	729.75'	8.0" Round Culvert
			L= 140.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 729.75' / 728.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	733.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	729.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	732.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.91 cfs @ 12.16 hrs HW=733.61' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 0.91 cfs of 2.07 cfs potential flow)

-2=Orifice/Grate (Weir Controls 0.20 cfs @ 1.10 fps)

-3=Orifice/Grate (Orifice Controls 0.20 cfs @ 9.36 fps)

-4=Orifice/Grate (Orifice Controls 0.51 cfs @ 5.79 fps)

Summary for Pond RG41: Rain Garden 41

Inflow Area =	7,525 sf, 65.00% Impervious,	Inflow Depth > 5.02" for 25YearMass event				
Inflow =	0.97 cfs @ 12.07 hrs, Volume=	3,147 cf				
Outflow =	0.79 cfs @ 12.14 hrs, Volume=	2,955 cf, Atten= 19%, Lag= 4.1 min				
Discarded =	0.05 cfs @ 12.14 hrs, Volume=	955 cf				
Primary =	0.74 cfs @ 12.14 hrs, Volume=	2,001 cf				
Routed to Link AP2-P : AP2-P						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.17' @ 12.14 hrs Surf.Area= 899 sf Storage= 566 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 25.7 min (808.2 - 782.5)

Type III 24-hr 25YearMass Rainfall=6.18" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 53

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	47 cf	
#2	728.25'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	731.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch 60 cf Overall x 50.0% Voids
#4	731.50'	373 cf	
		720 cf	Total Available Storage
Device	Routing	Invert Out	tlet Devices
#1	Discarded	728.25' 2.4	10 in/hr Exfiltration over Surface area
#2	Primary	730.25' 6.0	" Round Culvert
	-	L=	26.0' CPP, square edge headwall, Ke= 0.500
		Inle	et / Outlet Invert= 730.25' / 730.00' S= 0.0096 '/' Cc= 0.900
		n=	0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	732.00' 6.0	"Horiz. Orifice/Grate C= 0.600
		Lim	nited to weir flow at low heads
#4	Device 2	730.25' 2.0	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	731.00' 3.0	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.05 cfs @ 12.14 hrs HW=732.16' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.72 cfs @ 12.14 hrs HW=732.16' TW=0.00' (Dynamic Tailwater)

2=Culvert (Passes 0.72 cfs of 1.08 cfs potential flow)

3=Orifice/Grate (Weir Controls 0.34 cfs @ 1.32 fps)

-4=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.51 fps)

-5=Orifice/Grate (Orifice Controls 0.24 cfs @ 4.91 fps)

Summary for Pond RG42: Rain Garden 42

Inflow Area =	12,715 sf, 65.00% Impervious,	Inflow Depth > 5.02" for 25YearMass event		
Inflow =	1.64 cfs @ 12.07 hrs, Volume=	5,318 cf		
Outflow =	1.34 cfs @ 12.14 hrs, Volume=	4,798 cf, Atten= 18%, Lag= 3.9 min		
Discarded =	0.04 cfs @ 12.14 hrs, Volume=	856 cf		
Primary =	1.30 cfs @ 12.14 hrs, Volume=	3,942 cf		
Routed to Link AP2-P : AP2-P				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.23' @ 12.14 hrs Surf.Area= 1,646 sf Storage= 1,186 cf

Plug-Flow detention time= 76.7 min calculated for 4,788 cf (90% of inflow) Center-of-Mass det. time= 29.6 min (812.1 - 782.5)

AP2

Type III 24-hr 25YearMass Rainfall=6.18" Printed 5/1/2023

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page	
	e 54

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	212 cf	18.0" Round Pipe Storage x 2 Inside #2
		40.4	L= 60.0'
#2	728.25'	491 cf	8.00'W x 60.00'L x 3.00'H Soil Media and Gravel 1,440 cf Overall - 212 cf Embedded = 1,228 cf x 40.0% Voids
#3	731.25'	60 cf	8.00'W x 60.00'L x 0.25'H Mulch
			120 cf Overall x 50.0% Voids
#4	731.50'	621 cf	8.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		1,385 cf	Total Available Storage
Device	Routing	Invert Outl	et Devices
#1	Discarded	728.25' 1.02	0 in/hr Exfiltration over Surface area

#1	Discarded	728.25	1.020 in/hr Exfiltration over Surface area
#2	Primary	730.25'	8.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 730.25' / 730.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Device 2	732.00'	8.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2	730.25'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	731.00'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.14 hrs HW=732.22' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=1.27 cfs @ 12.14 hrs HW=732.22' TW=0.00' (Dynamic Tailwater)

-**2=Culvert** (Passes 1.27 cfs of 2.15 cfs potential flow)

-3=Orifice/Grate (Weir Controls 0.70 cfs @ 1.53 fps)

-4=Orifice/Grate (Orifice Controls 0.32 cfs @ 6.54 fps)

-5=Orifice/Grate (Orifice Controls 0.25 cfs @ 5.04 fps)

Summary for Pond RG43-48: Rain Garden 43-48

Inflow Area =	39,875 sf, 65.00% Impervious,	Inflow Depth > 5.02" for 25YearMass event
Inflow =	5.16 cfs @ 12.07 hrs, Volume=	16,676 cf
Outflow =	3.55 cfs @ 12.17 hrs, Volume=	15,048 cf, Atten= 31%, Lag= 5.6 min
Discarded =	0.15 cfs @ 12.17 hrs, Volume=	3,138 cf
Primary =	2.84 cfs @ 12.17 hrs, Volume=	9,925 cf
Routed to Link	AP2-P : AP2-P	
Secondary =	0.57 cfs @ 12.17 hrs, Volume=	1,985 cf
Routed to Read	ch SW2.1 : Swale RG2.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 733.61' @ 12.17 hrs Surf.Area= 6,394 sf Storage= 3,961 cf

Plug-Flow detention time= 77.4 min calculated for 15,048 cf (90% of inflow) Center-of-Mass det. time= 29.7 min (812.1 - 782.5)

AP2

Prepared by Mcclure Engineeering

AP2			Postdevelopment "Type III 24-hr 25YearMass Rainfall=6.18
Prepare	ed by Mcclure	Engineeerii	ng Printed 5/1/2023
HydroCA	D® 10.20-2g s/	/ <u>n 03362 © 20</u>	022 HydroCAD Software Solutions LLC Page 55
Volume	Invert	Avail.Stora	age Storage Description
#1	729.75'	283	3 cf 12.0" Round Pipe Storage x 6 Inside #2 L= 60.0'
#2	729.75'	2,047	7 cf 5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 6 5,400 cf Overall - 283 cf Embedded = 5,117 cf x 40.0% Voids
#3	732.75'	22	5 cf 5.00'W x 60.00'L x 0.25'H Mulch x 6 450 cf Overall x 50.0% Voids
#4	733.00'	2,612	2 cf 5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 6
			7 cf Total Available Storage
Device	Routing	Invert	Outlet Devices
#1	Discarded	729.75'	1.020 in/hr Exfiltration over Surface area
#2	Primary		6.0" Round Culvert X 5.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 731.75' / 731.65' S= 0.0100 '/' Cc= 0.900
#3	Secondary		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf 6.0" Round Culvert
#3	Secondary		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 731.75' / 731.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#4	Device 2		6.0" Horiz. Orifice/Grate X 5.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 3	733.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#6	Device 2		2.0" Vert. Orifice/Grate X 5.00 C= 0.600 Limited to weir flow at low heads
#7	Device 3		2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#8	Device 2		3.0" Vert. Orifice/Grate X 5.00 C= 0.600
			Limited to weir flow at low heads
#9	Device 3	732.50'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.15 cfs @ 12.17 hrs HW=733.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.15 cfs)

Primary OutFlow Max=2.72 cfs @ 12.17 hrs HW=733.60' TW=0.00' (Dynamic Tailwater)

-2=Culvert (Passes 2.72 cfs of 5.98 cfs potential flow)

4=Orifice/Grate (Weir Controls 0.85 cfs @ 1.05 fps)

-6=Orifice/Grate (Orifice Controls 0.70 cfs @ 6.41 fps)

-8=Orifice/Grate (Orifice Controls 1.17 cfs @ 4.76 fps)

Secondary OutFlow Max=0.54 cfs @ 12.17 hrs HW=733.60' TW=722.40' (Dynamic Tailwater) -3=Culvert (Passes 0.54 cfs of 1.20 cfs potential flow)

5=Orifice/Grate (Weir Controls 0.17 cfs @ 1.05 fps)

-7=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.41 fps)

-9=Orifice/Grate (Orifice Controls 0.23 cfs @ 4.76 fps)

Summary for Pond RG49-50: Rain Gardens 49,50

Inflow Area =	10,410 sf, 65.00% Impervious,	Inflow Depth > 5.02" for 25YearMass event		
Inflow =	1.35 cfs @ 12.07 hrs, Volume=	4,354 cf		
Outflow =	0.73 cfs @ 12.20 hrs, Volume=	3,818 cf, Atten= 46%, Lag= 7.7 min		
Discarded =	0.05 cfs @ 12.20 hrs, Volume=	999 cf		
Primary =	0.69 cfs @ 12.20 hrs, Volume=	2,818 cf		
Routed to Reach SW2.1 : Swale RG2.1				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 731.92' @ 12.20 hrs Surf.Area= 2,023 sf Storage= 1,149 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 31.9 min (814.4 - 782.5)

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
	700 051	000 (L=60.0'
#2	728.25'	682 ct	5.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 2
#2	721 25'	75 of	
#3	131.25	7501	
#4	731.50'	871 cf	
			Total Available Storage
#3 #4	731.25'	75 cf 871 cf	1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids 5.00'W x 60.00'L x 0.25'H Mulch x 2 150 cf Overall x 50.0% Voids 5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2

Device	Routing	Invert	Outlet Devices
#1	Discarded	728.25'	1.020 in/hr Exfiltration over Surface area
#2	Primary	730.25'	6.0" Round Culvert X 2.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 730.25' / 730.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	732.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	730.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 2	731.00'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.05 cfs @ 12.20 hrs HW=731.92' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.69 cfs @ 12.20 hrs HW=731.92' TW=722.38' (Dynamic Tailwater) 2=Culvert (Passes 0.69 cfs of 2.25 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -4=Orifice/Grate (Orifice Controls 0.26 cfs @ 6.06 fps) 5=Orifice/Grate (Orifice Controls 0.42 cfs @ 4.20 fps)

-5=Orifice/Grate (Orifice Controls 0.42 cfs @ 4.29 fps)

Summary for Pond RG52-54: Rain Garden Lots 52,53,54

Inflow Are	a =	28,140 sf, 53.72% Impervious, In	flow Depth > 4.15" for 25YearMass event
Inflow	=	3.13 cfs @ 12.07 hrs, Volume=	9,733 cf
Outflow	=	2.02 cfs @ 12.17 hrs, Volume=	9,705 cf, Atten= 35%, Lag= 5.8 min
Primary	=	2.02 cfs @ 12.17 hrs, Volume=	9,705 cf
Routed	l to Link	AP2-P : AP2-P	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 690.44' @ 12.17 hrs Surf.Area= 2,506 sf Storage= 1,430 cf

Plug-Flow detention time= 14.6 min calculated for 9,705 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (819.3 - 806.5)

Volume	Invert	Avail.Storage	Storage Description
#1	686.75'	141 cf	12.0" Round Pipe Storage x 3 Inside #2
			L= 60.0'
#2	686.75'	807 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel × 3
			2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids
#3	689.75'	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
			180 cf Overall x 50.0% Voids
#4	690.00'	1,120 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3
		2,159 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	686.75'	6.0" Round Culvert X 3.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 686.75' / 686.70' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	690.50'	6.0" Horiz. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	686.75'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	689.00'	4.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=2.01 cfs @ 12.17 hrs HW=690.42' TW=0.00' (Dynamic Tailwater)

-**1=Culvert** (Passes 2.01 cfs of 5.25 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.60 cfs @ 9.12 fps)

-4=Orifice/Grate (Orifice Controls 1.41 cfs @ 5.40 fps)

Summary for Pond RG55: Rain Garden 55

 Inflow Area =
 5,450 sf, 65.00% Impervious, Inflow Depth > 5.02" for 25YearMass event

 Inflow =
 0.70 cfs @
 12.07 hrs, Volume=
 2,279 cf

 Outflow =
 0.54 cfs @
 12.14 hrs, Volume=
 2,272 cf, Atten= 24%, Lag= 4.3 min

 Primary =
 0.54 cfs @
 12.14 hrs, Volume=
 2,272 cf

 Routed to Link AP2-P : AP2-P
 AP2-P
 2,272 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Peak Elev= 688.88' @ 12.14 hrs Surf.Area= 480 sf Storage= 367 cf

Plug-Flow detention time= 16.9 min calculated for 2,267 cf (99% of inflow) Center-of-Mass det. time= 14.9 min (797.4 - 782.5)

Volume	Invert	Avail.Storage	Storage Description
#1	685.75'	106 cf	18.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	685.75'	246 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	688.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	689.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		755 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	685.75'	6.0" Round Culvert
	2		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 685.75' / 685.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	689.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	685.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	688.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.53 cfs @ 12.14 hrs HW=688.86' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 0.53 cfs of 1.60 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.18 cfs @ 8.38 fps)

-4=Orifice/Grate (Orifice Controls 0.35 cfs @ 4.02 fps)

Summary for Link AP2-P: AP2-P

Inflow Are	a =	550,840 sf, 23.14% Impervious,	Inflow Depth > 3.05"	for 25YearMass event
Inflow	=	31.68 cfs @ 12.16 hrs, Volume=	139,992 cf	
Primary	=	31.68 cfs @ 12.16 hrs, Volume=	139,992 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Subcatchment P2.1: To Wetland A (A56-A87)

Runoff = 21.04 cfs @ 12.14 hrs, Volume= 74,387 cf, Depth> 4.40" Routed to Link AP2-P : AP2-P

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN [Description						
1	42,635	70 \	Woods, Good, HSG C						
	45,615	74 >	•75% Gras	s cover, Go	ood, HSG C				
	10,360	55 \	Voods, Go	od, HSG B					
	3,340				ood, HSG B				
	1,060	<u>98 l</u>	Jnconnecte	ed roofs, H	SG B				
2	03,010	70 \	Veighted A	verage					
2	01,950	ę	9.48% Per	rvious Area					
	1,060			ervious Are					
	1,060		00.00% U	nconnected	1				
_		~ .		.					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	Capacity (cfs)	Description				
	•		,		Sheet Flow,				
<u>(min)</u> 5.1	(feet) 50	(ft/ft) 0.2000	(ft/sec) 0.16		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"				
(min)	(feet)	(ft/ft)	(ft/sec)		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow,				
(min) 5.1 0.7	(feet) 50 90	(ft/ft) 0.2000 0.2000	(ft/sec) 0.16 2.24		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps				
<u>(min)</u> 5.1	(feet) 50	(ft/ft) 0.2000	(ft/sec) 0.16		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps Shallow Concentrated Flow,				
(min) 5.1 0.7	(feet) 50 90	(ft/ft) 0.2000 0.2000	(ft/sec) 0.16 2.24		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps				

Summary for Subcatchment P2.10: To RG4.1

Runoff = 3.77 cfs @ 12.10 hrs, Volume= 12,556 cf, Depth> 5.67" Routed to Reach SW2.1 : Swale RG2.1

	Area (sf)	CN	Description				
*	12,070	90	Residential Lots, 65% imp, HSG C				
	2,925	70	Woods, Good, HSG C				
	11,575	74	>75% Grass cover, Good, HSG C				
	26,570	Weighted Average					
	18,725		70.47% Pervious Area				
	7,846		29.53% Impervious Area				

AP2					Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"
Prepare	ed by Mc	clure Eng	gineeering]	Printed 5/1/2023
HydroCA	D® 10.20-	2g_s/n 03	362 © 202	2 HydroCAD	O Software Solutions LLC Page 60
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.2	50	0.1200	0.13	(010)	Sheet Flow,
1.0	100	0.1200	1.73		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps

7.2 150 Total

Summary for Subcatchment P2.11: Lots 48-50

5,841 cf, Depth> 6.73"

Runoff = 1.78 cfs @ 12.07 hrs, Volume= Routed to Pond RG49-50 : Rain Gardens 49,50

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	A	rea (sf)	CN [Description						
*		10,410	90 F	90 Residential Lots, 65% imp, HSG C						
		3,644	3	35.00% Pervious Area						
		6,767	6	65.00% Impervious Area						
	Тс	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•				
	5.0					Direct Entry,				
						-				

Summary for Subcatchment P2.12: Lot 54

Runoff = 0.93 cfs @ 12.07 hrs, Volume= 3,058 cf, Depth> 6.73" Routed to Pond RG55 : Rain Garden 55

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

_	A	rea (sf)	CN I	Description							
*		5,450	90 I	Residential	Lots, 65%	imp, HSG C					
		1,908	3	35.00% Pervious Area							
		3,543	6	65.00% Impervious Area							
	-		0		o						
	Tc	Length	Slope	,	Capacity	•					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry,					

Summary for Subcatchment P2.13: Lot 14

Runoff = 1.16 cfs @ 12.07 hrs, Volume= 3,787 cf, Depth> 6.61" Routed to Pond RG14 : Rain Garden 14

A	rea (sf)	CN I	Description				
	5,915	90 [·]	1/8 acre lots	s, 65% imp	o, HSG C		
	955	85 ⁻	1/8 acre lots	s, 65% imp	, HSG B		
	6,870	89 \	Weighted Average				
	2,405		35.00% Pervious Area				
	4,466	(65.00% Imp	ervious Are	ea		
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
/	(ieet)	(1011)	(11/360)	(013)			
5.0					Direct Entry,		

Summary for Subcatchment P2.14: Lots 30-33

Runoff	=	3.67 cfs @	12.07 hrs,	Volume=	12,066 cf,	Depth>	6.73"
Routed	to Pond	RG31-33 : R	ain Garden	s 31,32,33			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	Area (sf)	CN I	Description						
*	21,505	90 I	Residential	Lots, 65%	imp, HSG C				
	7,527	÷	35.00% Pervious Area						
	13,978	(65.00% Impervious Area						
_		~		• •	-				
	c Length	Slope	,	Capacity	Description				
(mii	n) (feet)	(ft/ft)	(ft/sec)	(cfs)					
5	0				Direct Entry,				
					•				

Summary for Subcatchment P2.2: Lots 42-48

6.81 cfs @ 12.07 hrs, Volume= 22,374 cf, Depth> 6.73" Runoff = Routed to Pond RG43-48 : Rain Garden 43-48

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	A	rea (sf)	CN [Description						
*		39,875	90 F	Residential	Lots, 65%	imp, HSG C				
		13,956								
		25,919	6	65.00% Impervious Area						
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	5.0					Direct Entry,				

AP2

Summary for Subcatchment P2.3: Lots 41-42

Runoff = 2.17 cfs @ 12.07 hrs, Volume= 7,134 cf, Depth> 6.73" Routed to Pond RG42 : Rain Garden 42

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Area (sf)	CN Description					
12,715	90 1/8 acre lots, 65% imp, HSG C					
4,450	35.00% Pervious Area					
8,265	65.00% Impervious Area					
Tc Length (min) (feet)						
5.0	Direct Entry,					
Our many for Outboatchmant D0 4. Late 40.44						

Summary for Subcatchment P2.4: Lots 40-41

Runoff = 1.28 cfs @ 12.07 hrs, Volume= 4,222 cf, Depth> 6.73" Routed to Pond RG41 : Rain Garden 41

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN E	Description			
	7,525	90 1	1/8 acre lots, 65% imp, HSG C			
	2,634 4,891	-	35.00% Pervious Area 65.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
5.0					Direct Entry,	

Summary for Subcatchment P2.5: Lots 39-40

Runoff = 1.63 cfs @ 12.07 hrs, Volume= Routed to Pond RG40 : Rain Garden 40 5,344 cf, Depth> 6.73"

Area (sf)	CN	Description		
9,525	90	1/8 acre lots, 65% imp, HSG C		
3,334		35.00% Pervious Area		
6,191		65.00% Impervious Area		

AP2	al las s Mara				T_{j}	ype III 24-h	r 100YearMass Rainfall=7.93"
Prepared by Mcclure EngineeeringPrinted 5/1/2023HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLCPage 63							
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		· · ·
5.0					Direct Entr	у,	
	Summary for Subcatchment P2.6: Lots 33-39						
Runoff Route	Runoff = 7.73 cfs @ 12.07 hrs, Volume= 25,409 cf, Depth> 6.73" Routed to Pond RG34-39 : Rain Gardens 34-39						
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"							
A	rea (sf)	CN	Description				
*	45,285	90			imp, HSG C		
	15,850 29,435		35.00% Pei 65.00% Imp				
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
5.0							
Summary for Subcatchment P2.7: Upgradient Lots 35-39							
Runoff = 3.63 cfs @ 12.28 hrs, Volume= 16,636 cf, Depth> 4.39" Routed to Pond IT-35/39 : Interceptor Trench Lots 35-39							
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"							
A	rea (sf)	CN	Description				
	4,550	74			ood, HSG C		
	40,950	70	Woods, Go				
	45,500 45,500	70	Weighted A 100.00% Pe		а		

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	12.7	50	0.0200	0.07		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	7.1	560	0.0700	1.32		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	20	0.5000	4.95		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
_	19.9	630	Total			

Summary for Subcatchment P2.8: Upgradient Lots 30-35

Runoff = 7.86 cfs @ 12.22 hrs, Volume= 33,209 cf, Depth> 4.50" Routed to Pond IT-30/35 : Interceptor Trench Lots 30-35

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN E	Description						
	11,085		4 >75% Grass cover, Good, HSG C						
	77,375	70 V	Voods, Go	<u>od, HSG C</u>					
	88,460	71 V	Veighted A	verage					
	88,460	1	00.00% Pe	ervious Are	а				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
12.7	50	0.0200	0.07		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.00"				
3.5	280	0.0700	1.32		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
0.1	30	0.5000	4.95		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
16.3	360	Total							

Summary for Subcatchment P2.9: Lots 51-53

Runoff = 4.31 cfs @ 12.07 hrs, Volume= 13,577 cf, Depth> 5.79" Routed to Pond RG52-54 : Rain Garden Lots 52,53,54

Area (sf)	CN	Description
2,430	90	1/8 acre lots, 65% imp, HSG C
20,825	85	1/8 acre lots, 65% imp, HSG B
3,605	70	Woods, Good, HSG C
1,280	55	Woods, Good, HSG B
28,140	82	Weighted Average
13,024		46.28% Pervious Area
15,116		53.72% Impervious Area
Tc Length	Slop	
(min) (feet)	(ft/	(ft) (ft/sec) (cfs)
5.0		Direct Entry,

Summary for Reach SW2.1: Swale RG2.1

Inflow Area = 58,485 sf, 48.88% Impervious, Inflow Depth > 8.24" for 100YearMass event Inflow 10.45 cfs @ 12.13 hrs, Volume= 40.149 cf 10.52 cfs @ 12.13 hrs, Volume= 40,144 cf, Atten= 0%, Lag= 0.5 min Outflow = Routed to Pond RG2.1 : Rain Garden 2.1 Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Max. Velocity= 10.50 fps, Min. Travel Time= 0.2 min Avg. Velocity = 3.46 fps, Avg. Travel Time= 0.6 min Peak Storage= 120 cf @ 12.13 hrs Average Depth at Peak Storage= 0.50', Surface Width= 3.00' Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 46.26 cfs 1.00' x 1.00' deep channel, n= 0.025 Earth, clean & winding Side Slope Z-value= 2.0 '/' Top Width= 5.00' Length= 120.0' Slope= 0.1500 '/' Inlet Invert= 722.00', Outlet Invert= 704.00' Summary for Pond IT-30/35: Interceptor Trench Lots 30-35 Inflow Area = 88,460 sf, 0.00% Impervious, Inflow Depth > 4.50" for 100YearMass event Inflow 7.86 cfs @ 12.22 hrs, Volume= 33,209 cf = Outflow = 7.38 cfs @ 12.29 hrs, Volume= 33,096 cf, Atten= 6%, Lag= 3.7 min = 7.38 cfs @ 12.29 hrs, Volume= Primary 33,096 cf Routed to Link AP2-P : AP2-P Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 734.18' @ 12.29 hrs Surf.Area= 1,470 sf Storage= 1,387 cf

Plug-Flow detention time= 6.2 min calculated for 33,027 cf (99% of inflow) Center-of-Mass det. time= 4.2 min (835.0 - 830.8)

Volume	Invert	Avail.Storag	ge Storage Description
#1	732.00'	1,696	cf 3.00'W x 490.00'L x 3.00'H Prismatoid
			4,410 cf Overall - 171 cf Embedded = 4,239 cf x 40.0% Voids
#2	732.00'	171	cf 8.0" Round Pipe Storage Inside #1
			L= 490.0'
		1,867	cf Total Available Storage
Device	Routing	Invert C	Dutlet Devices
#1	Primary	732.00' 1	5.0" Round Culvert
	2	L	.= 250.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 732.00' / 724.00' S= 0.0320 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=7.34 cfs @ 12.29 hrs HW=734.17' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 7.34 cfs @ 5.98 fps)

Summary for Pond IT-35/39: Interceptor Trench Lots 35-39

 Inflow Area =
 45,500 sf, 0.00% Impervious, Inflow Depth > 4.39" for 100YearMass event

 Inflow =
 3.63 cfs @
 12.28 hrs, Volume=
 16,636 cf

 Outflow =
 3.10 cfs @
 12.40 hrs, Volume=
 16,563 cf, Atten= 14%, Lag= 7.5 min

 Primary =
 3.10 cfs @
 12.40 hrs, Volume=
 16,563 cf

 Routed to Link AP2-P : AP2-P
 16,563 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 736.20' @ 12.40 hrs Surf.Area= 1,200 sf Storage= 1,139 cf

Plug-Flow detention time= 8.5 min calculated for 16,529 cf (99% of inflow) Center-of-Mass det. time= 5.9 min (841.7 - 835.7)

Volume	Invert	Avail.Storage	Storage Description
#1	734.00'	1,384 cf	3.00'W x 400.00'L x 3.00'H Prismatoid
			3,600 cf Overall - 140 cf Embedded = 3,460 cf x 40.0% Voids
#2	734.00'	140 cf	8.0" Round Pipe Storage Inside #1
			L= 400.0'
		1,524 cf	Total Available Storage
			·
Device	Routing	Invert Outl	et Devices
#1	Primary	734 00' 10 0	" Round Culvert

#1	Primary	734.00'	10.0" Round Culvert
			L= 80.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 734.00' / 733.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf

Primary OutFlow Max=3.10 cfs @ 12.40 hrs HW=736.20' TW=0.00' (Dynamic Tailwater) -1=Culvert (Barrel Controls 3.10 cfs @ 5.69 fps)

Summary for Pond RG14: Rain Garden 14

 Inflow Area =
 6,870 sf, 65.00% Impervious, Inflow Depth > 6.61" for 100YearMass event

 Inflow =
 1.16 cfs @
 12.07 hrs, Volume=
 3,787 cf

 Outflow =
 0.74 cfs @
 12.17 hrs, Volume=
 3,771 cf, Atten= 36%, Lag= 5.7 min

 Primary =
 0.74 cfs @
 12.17 hrs, Volume=
 3,771 cf

 Routed to Pond RG2.1 : Rain Garden 2.1
 3,771 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 705.09' @ 12.17 hrs Surf.Area= 878 sf Storage= 535 cf

Plug-Flow detention time= 16.3 min calculated for 3,763 cf (99% of inflow) Center-of-Mass det. time= 13.6 min (791.7 - 778.1)

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

	0	0		
-lydroCAD® 10.20-2g	s/n 03362	© 2022 H	ydroCAD Software Solutions LLC	Page 67

Volume	Invert	Avail.Storage	Storage Description
#1	702.00'	47 cf	12.0" Round Pipe Storage Inside #2 L= 60.0'
#2	702.00'	269 cf	
#3	704.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
#4	704.50'	373 cf	60 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	L= 7 Inle	' Round Culvert 75.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 702.00' / 702.00' S= 0.0000 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	705.00' 6.0'	'Horiz. Orifice/Grate C= 0.600 ited to weir flow at low heads
#3 #4	Device 1 Device 1	702.00' 2.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.72 cfs @ 12.17 hrs HW=705.08' TW=703.01' (Dynamic Tailwater)

1=Culvert (Passes 0.72 cfs of 0.83 cfs potential flow)

-2=Orifice/Grate (Weir Controls 0.11 cfs @ 0.91 fps)

-3=Orifice/Grate (Orifice Controls 0.15 cfs @ 6.92 fps)

-4=Orifice/Grate (Orifice Controls 0.45 cfs @ 5.19 fps)

Summary for Pond RG2.1: Rain Garden 2.1

Inflow Area =	65,355 sf, 50.58% Impervious,	Inflow Depth > 8.06" for 100YearMass event
Inflow =	11.23 cfs @ 12.14 hrs, Volume=	43,915 cf
Outflow =	5.91 cfs @ 12.33 hrs, Volume=	41,265 cf, Atten= 47%, Lag= 11.8 min
Discarded =	0.75 cfs @ 12.33 hrs, Volume=	19,820 cf
Primary =	5.16 cfs @ 12.33 hrs, Volume=	21,446 cf
Routed to Link	(AP2-P : AP2-P	
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf
Routed to Link	(AP2-P : AP2-P	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 703.36' @ 12.33 hrs Surf.Area= 13,513 sf Storage= 11,958 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 52.4 min (846.6 - 794.2)

AP2

Prepared by Mcclure Engineeering

Postdevelopment

Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

	eu by Micciule			Finited 5/1/2023
HydroC/	AD® 10.20-2g s/	/n 03362 © 2	2022	HydroCAD Software Solutions LLC Page 68
Volume	Invert	Avail.Sto	rage	Storage Description
#1	698.75'	(67 cf	12.0" Round Pipe Storage Inside #2
				L= 85.0'
#2	698.75'	5,0	73 cf	
	704 75	-		12,750 cf Overall - 67 cf Embedded = 12,683 cf x 40.0% Voids
#3	701.75'	5	31 cf	
#4	702.00'	0.6	23 cf	1,063 cf Overall x 50.0% Voids 50.00'W x 85.00'L x 2.00'H Ponding Z=2.0
#4	702.00			
		15,23	94 CI	Total Available Storage
Device	Routing	Invert	Outl	let Devices
#1	Primary	701.00'	12.0	0" Round Culvert
	-		L= 1	10.0' CPP, square edge headwall, Ke= 0.500
				t / Outlet Invert= 701.00' / 699.65' S= 0.1350 '/' Cc= 0.900
				0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	703.00'		D" Horiz. Orifice/Grate X 2.00 C= 0.600
	During	704 001		ited to weir flow at low heads
#3	Device 1	701.00'	-	"Vert. Orifice/Grate X 2.00 C= 0.600
#4	Device 1	701.75'		ited to weir flow at low heads ' Vert. Orifice/Grate X 2.00 C= 0.600
#4	Device I	701.75		ited to weir flow at low heads
#5	Secondary	703.75'		0' long x 10.0' breadth Broad-Crested Rectangular Weir
110	eeeenaary	100.10		ad (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
				ef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#6	Discarded	698.75'		10 in/hr Exfiltration over Surface area
Dieser		lov-0 75 of	• @ 1	12.22 bra = 410/(-702.26') (Erec Discharge)
A	filtration (Exf		-	12.33 hrs HW=703.36' (Free Discharge)

6=Exfiltration (Exfiltration Controls 0.75 cfs)

Primary OutFlow Max=5.15 cfs @ 12.33 hrs HW=703.36' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.15 cfs @ 6.56 fps) 2=Orifice/Grate (Passes < 3.14 cfs potential flow) 3=Orifice/Grate (Passes < 0.32 cfs potential flow) 4=Orifice/Grate (Passes < 2.20 cfs potential flow)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=698.75' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG31-33: Rain Gardens 31,32,33

Inflow Are	a =	21,505 sf	, 65.00% Impervious	Inflow Depth >	6.73"	for 100YearMass event
Inflow	=	3.67 cfs @	12.07 hrs, Volume=	12,066 c	f	
Outflow	=	2.90 cfs @	12.14 hrs, Volume=	12,038 c	f, Atter	n= 21%, Lag= 3.9 min
Primary	=	2.90 cfs @	12.14 hrs, Volume=	12,038 c	f	
Routed to Reach SW2.1 : Swale RG2.1						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.28' @ 12.14 hrs Surf.Area= 2,791 sf Storage= 1,847 cf

Plug-Flow detention time= 16.1 min calculated for 12,013 cf (100% of inflow) Center-of-Mass det. time= 14.6 min (789.5 - 774.9)

AP2 Prepared by Mcclure Engineeering

 AP2
 Type III 24-hr
 100YearMass Rainfall=7.93"

 Prepared by Mcclure Engineeering
 Printed 5/1/2023

 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC
 Page 69

Volume	Invert	Avail.Stor	rage	Storage Description		
#1	728.25'	141 cf		12.0" Round Pipe Storage x 3 Inside #2		
#2	728.25'	807 cf		L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 3 2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids		
#3	731.25'	ç	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3		
				180 cf Overall x 50.0% Voids		
#4	731.50'	1,12	20 ct	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3		
		2,15	59 cf	Total Available Storage		
Device	Routing	Invert	Outle	et Devices		
#1	Primary	728.25'	6.0"	Round Culvert X 3.00		
	-		L= 10	0.0' CPP, square edge headwall, Ke= 0.500		
				/ Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900		
			n= 0.	.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	732.00'		Horiz. Orifice/Grate X 3.00 C= 0.600		
			Limit	ed to weir flow at low heads		
#3	Device 1	728.25'	2.0"	Vert. Orifice/Grate X 3.00 C= 0.600		
			Limit	ed to weir flow at low heads		
#4	Device 1	731.00'	3.0"	Vert. Orifice/Grate X 3.00 C= 0.600		
			Limit	ed to weir flow at low heads		

Primary OutFlow Max=2.87 cfs @ 12.14 hrs HW=732.27' TW=722.50' (Dynamic Tailwater)

-1=Culvert (Passes 2.87 cfs of 5.51 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.48 cfs @ 2.52 fps)

-3=Orifice/Grate (Orifice Controls 0.63 cfs @ 9.56 fps)

-4=Orifice/Grate (Orifice Controls 0.76 cfs @ 5.16 fps)

Summary for Pond RG34-39: Rain Gardens 34-39

45,285 sf, 65.00% Impervious, Inflow Depth > 6.73" for 100YearMass event Inflow Area = Inflow 7.73 cfs @ 12.07 hrs, Volume= 25,409 cf = Outflow = 6.05 cfs @ 12.14 hrs, Volume= 25,351 cf, Atten= 22%, Lag= 4.0 min 4.03 cfs @ 12.14 hrs, Volume= Primary = 16,901 cf Routed to Link AP2-P : AP2-P 2.02 cfs @ 12.14 hrs, Volume= 8,450 cf Secondary = Routed to Reach SW2.1 : Swale RG2.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.32' @ 12.14 hrs Surf.Area= 5,652 sf Storage= 3,805 cf

Plug-Flow detention time= 16.1 min calculated for 25,298 cf (100% of inflow) Center-of-Mass det. time= 14.6 min (789.4 - 774.9)

Postdevelopment

Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

HydroCAD® 10.20-2g	s/n 03362	© 2022 HydroCAD	Software Solutions LLC	Page 70

Volume	Invert	Avail.Sto	orage Storage Description		
#1	728.25'	28	83 cf 12.0" Round Pipe Storage x 6 Inside #2 L= 60.0'		
#2	728.25'	1,61	15 cf 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 6 4,320 cf Overall - 283 cf Embedded = 4,037 cf x 40.0% Voids		
#3	731.25'	18	80 cf 4.00'W x 60.00'L x 0.25'H Mulch x 6 360 cf Overall x 50.0% Voids		
#4	731.50'	2,24	40 cf 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 6		
		4,32	18 cf Total Available Storage		
Device	Routing	Invert	Outlet Devices		
#1	Primary	728.25'	6.0" Round Culvert X 4.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Secondary	728.25'	6.0" Round Culvert X 2.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 728.25' / 728.15' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#3	Device 1	732.00'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads		
#4	Device 2	732.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads		
#5	Device 1	728.25'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads		
#6	Device 2	728.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads		
#7	Device 1	731.00'	3.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads		
#8	Device 2	731.00'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads		

Primary OutFlow Max=3.99 cfs @ 12.14 hrs HW=732.32' TW=0.00' (Dynamic Tailwater)

- -1=Culvert (Passes 3.99 cfs of 7.39 cfs potential flow)
- -3=Orifice/Grate (Orifice Controls 2.12 cfs @ 2.70 fps)
- 5=Orifice/Grate (Orifice Controls 0.84 cfs @ 9.61 fps)
- **7=Orifice/Grate** (Orifice Controls 1.03 cfs @ 5.25 fps)

Secondary OutFlow Max=2.00 cfs @ 12.14 hrs HW=732.32' TW=722.50' (Dynamic Tailwater) -2=Culvert (Passes 2.00 cfs of 3.69 cfs potential flow) -4=Orifice/Grate (Orifice Controls 1.06 cfs @ 2.70 fps)

6=Orifice/Grate (Orifice Controls 0.42 cfs @ 9.61 fps)

-8=Orifice/Grate (Orifice Controls 0.52 cfs @ 5.25 fps)

Summary for Pond RG40: Rain Garden 40

 Inflow Area =
 9,525 sf, 65.00% Impervious, Inflow Depth > 6.73" for 100YearMass event

 Inflow =
 1.63 cfs @
 12.07 hrs, Volume=
 5,344 cf

 Outflow =
 1.30 cfs @
 12.13 hrs, Volume=
 5,333 cf, Atten= 20%, Lag= 3.8 min

 Primary =
 1.30 cfs @
 12.13 hrs, Volume=
 5,333 cf

 Routed to Link AP2-P : AP2-P
 5,333 cf

Prepared by Mcclure Engineeering

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 733.83' @ 12.13 hrs Surf.Area= 944 sf Storage= 637 cf

Plug-Flow detention time= 14.1 min calculated for 5,322 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (787.7 - 774.9)

Volume	Invert	Avail.Storage	Storage Description
#1	729.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	729.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	732.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	733.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage
			-

Device	Routing	Invert	Outlet Devices
#1	Primary	729.75'	8.0" Round Culvert
	-		L= 140.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 729.75' / 728.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	733.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	729.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	732.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.28 cfs @ 12.13 hrs HW=733.82' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 1.28 cfs of 2.11 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.53 cfs @ 2.71 fps)

-3=Orifice/Grate (Orifice Controls 0.21 cfs @ 9.61 fps)

-4=Orifice/Grate (Orifice Controls 0.54 cfs @ 6.18 fps)

Summary for Pond RG41: Rain Garden 41

Inflow Area =	7,525 sf, 65.00% Impervious,	Inflow Depth > 6.73" for 100YearMass event			
Inflow =	1.28 cfs @ 12.07 hrs, Volume=	4,222 cf			
Outflow =	1.02 cfs @ 12.13 hrs, Volume=	4,004 cf, Atten= 21%, Lag= 3.8 min			
Discarded =	0.05 cfs @ 12.13 hrs, Volume=	1,032 cf			
Primary =	0.97 cfs @ 12.13 hrs, Volume=	2,972 cf			
Routed to Link AP2-P : AP2-P					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.35' @ 12.13 hrs Surf.Area= 948 sf Storage= 644 cf

Plug-Flow detention time= 48.6 min calculated for 3,996 cf (95% of inflow) Center-of-Mass det. time= 20.0 min (794.8 - 774.9)

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

HydroCAD® 10.20-2g s/n 03362	© 2022 HydroCAD Software Solutions LLC	Page 72

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	47 cf	
#2	728.25'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	731.25'	30 cf	
#4	731.50'	373 cf	
		720 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Discarded	728.25' 2.4 1	I0 in/hr Exfiltration over Surface area
#2	Primary	730.25' 6.0'	' Round Culvert
		L= 2	26.0' CPP, square edge headwall, Ke= 0.500
			t / Outlet Invert= 730.25' / 730.00' S= 0.0096 '/' Cc= 0.900
		n= (0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	732.00' 6.0'	'Horiz. Orifice/Grate C= 0.600
		Lim	ited to weir flow at low heads
#4	Device 2	730.25' 2.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	731.00' 3.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.05 cfs @ 12.13 hrs HW=732.34' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.96 cfs @ 12.13 hrs HW=732.34' TW=0.00' (Dynamic Tailwater)

2=Culvert (Passes 0.96 cfs of 1.13 cfs potential flow)

3=Orifice/Grate (Orifice Controls 0.55 cfs @ 2.79 fps)

-4=Orifice/Grate (Orifice Controls 0.15 cfs @ 6.81 fps)

-5=Orifice/Grate (Orifice Controls 0.26 cfs @ 5.30 fps)

Summary for Pond RG42: Rain Garden 42

Inflow Area =	12,715 sf, 65.00% Impervious,	Inflow Depth > 6.73" for 100YearMass event		
Inflow =	2.17 cfs @ 12.07 hrs, Volume=	7,134 cf		
Outflow =	1.73 cfs @ 12.13 hrs, Volume=	6,611 cf, Atten= 20%, Lag= 3.8 min		
Discarded =	0.04 cfs @ 12.13 hrs, Volume=	919 cf		
Primary =	1.69 cfs @ 12.13 hrs, Volume=	5,691 cf		
Routed to Link AP2-P : AP2-P				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.41' @ 12.13 hrs Surf.Area= 1,701 sf Storage= 1,318 cf

Plug-Flow detention time= 66.1 min calculated for 6,611 cf (93% of inflow) Center-of-Mass det. time= 27.3 min (802.2 - 774.9)

AP2

Prepared by Mcclure Engineeering

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

AFZ	1 ype in 2+-in	100164111435114111411–1.55
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software	Solutions LLC	Page 73
		•

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	212 cf	18.0" Round Pipe Storage x 2 Inside #2
#0		404 -5	L= 60.0'
#2	728.25'	491 CT	8.00'W x 60.00'L x 3.00'H Soil Media and Gravel 1,440 cf Overall - 212 cf Embedded = 1,228 cf x 40.0% Voids
#3	731.25'	60 cf	8.00'W x 60.00'L x 0.25'H Mulch
110	101.20	00 01	120 cf Overall x 50.0% Voids
#4	731.50'	621 cf	8.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		1,385 cf	Total Available Storage
			-

Device	Routing	Invert	Outlet Devices
#1	Discarded	728.25'	1.020 in/hr Exfiltration over Surface area
#2	Primary	730.25'	8.0" Round Culvert
	-		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 730.25' / 730.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Device 2	732.00'	8.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2	730.25'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	731.00'	3.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.13 hrs HW=732.40' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=1.67 cfs @ 12.13 hrs HW=732.40' TW=0.00' (Dynamic Tailwater)

2=Culvert (Passes 1.67 cfs of 2.27 cfs potential flow)

3=Orifice/Grate (Orifice Controls 1.06 cfs @ 3.05 fps)

-4=Orifice/Grate (Orifice Controls 0.34 cfs @ 6.85 fps)

-5=Orifice/Grate (Orifice Controls 0.27 cfs @ 5.44 fps)

Summary for Pond RG43-48: Rain Garden 43-48

Inflow Area =	39,875 sf	65.00% Impervious,	Inflow Depth > 6.73" for 100YearMass event
Inflow =	6.81 cfs @	12.07 hrs, Volume=	22,374 cf
Outflow =	5.36 cfs @	12.14 hrs, Volume=	20,731 cf,Atten= 21%,Lag= 3.9 min
Discarded =	0.16 cfs @	12.14 hrs, Volume=	3,375 cf
Primary =	4.34 cfs @	12.14 hrs, Volume=	14,464 cf
Routed to Link	AP2-P : AP2-	Р	
Secondary =	0.87 cfs @	12.14 hrs, Volume=	2,893 cf
Routed to Read	ch SW2.1 : Sv	vale RG2.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 733.75' @ 12.14 hrs Surf.Area= 6,622 sf Storage= 4,353 cf

Plug-Flow detention time= 65.9 min calculated for 20,688 cf (92% of inflow) Center-of-Mass det. time= 27.7 min (802.6 - 774.9)

AP2

	ed by Mcclure D® 10.20-2g_s			Postdevelopment <i>Type III 24-hr 100YearMass Rainfall=7.93"</i> Printed 5/1/2023 lydroCAD Software Solutions LLC Page 74	
Volume	Invert	Avail.Stor	rage	Storage Description	
#1	729.75'		33 cf	12.0" Round Pipe Storage x 6 Inside #2	
#2	729.75'	2,04	17 cf	L= 60.0' F cf 5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 6 5,400 cf Overall - 283 cf Embedded = 5,117 cf x 40.0% Voids	
#3	732.75'	22	25 cf	5.00'W x 60.00'L x 0.25'H Mulch x 6 450 cf Overall x 50.0% Voids	
#4	733.00'	2,61	12 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 6	
		5,16	67 cf	Total Available Storage	
Device	Routing	Invert	Outl	et Devices	
#1	Discarded	729.75'	-	0 in/hr Exfiltration over Surface area	
#2	Primary	731.75'	L= 1 Inlet	Round Culvert X 5.00 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 731.75' / 731.65' S= 0.0100 '/' Cc= 0.900	
#3	Secondary	731.75'	6.0'' L= 1 Inlet	.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf Round Culvert 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 731.75' / 731.65' S= 0.0100 '/' Cc= 0.900 .013 Corrugated PE, smooth interior, Flow Area= 0.20 sf	
#4	Device 2	733.50'	6.0"	Horiz. Orifice/Grate X 5.00 C= 0.600 ted to weir flow at low heads	
#5	Device 3	733.50'	6.0"	Horiz. Orifice/Grate C= 0.600 ted to weir flow at low heads	
#6	Device 2	731.75'	2.0"	Vert. Orifice/Grate X 5.00 C= 0.600 ted to weir flow at low heads	
#7	Device 3	731.75'		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads	
#8	Device 2	732.50'	3.0"	Vert. Orifice/Grate X 5.00 C= 0.600 ted to weir flow at low heads	
#9	Device 3	732.50'	3.0"	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads	

Discarded OutFlow Max=0.16 cfs @ 12.14 hrs HW=733.74' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.16 cfs)

Primary OutFlow Max=4.29 cfs @ 12.14 hrs HW=733.74' TW=0.00' (Dynamic Tailwater)

- -2=Culvert (Passes 4.29 cfs of 6.24 cfs potential flow)
 - 4=Orifice/Grate (Orifice Controls 2.32 cfs @ 2.36 fps)
 - -6=Orifice/Grate (Orifice Controls 0.73 cfs @ 6.65 fps)
 - -8=Orifice/Grate (Orifice Controls 1.25 cfs @ 5.09 fps)

Secondary OutFlow Max=0.86 cfs @ 12.14 hrs HW=733.74' TW=722.50' (Dynamic Tailwater) -3=Culvert (Passes 0.86 cfs of 1.25 cfs potential flow)

- 5=Orifice/Grate (Orifice Controls 0.46 cfs @ 2.36 fps)
 - -7=Orifice/Grate (Orifice Controls 0.15 cfs @ 6.65 fps)

-9=Orifice/Grate (Orifice Controls 0.25 cfs @ 5.09 fps)

4

Summary for Pond RG49-50: Rain Gardens 49,50

Inflow Area = 10,410 sf, 65.00% Impervious, Inflow Depth > 6.73" for 100YearMass event 1.78 cfs @ 12.07 hrs, Volume= Inflow = 5.841 cf 1.28 cfs @ 12.16 hrs, Volume= 5,298 cf, Atten= 28%, Lag= 5.4 min Outflow = Discarded = 0.05 cfs @ 12.16 hrs, Volume= 1,086 cf Primary = 1.23 cfs @ 12.16 hrs, Volume= 4,212 cf Routed to Reach SW2.1 : Swale RG2.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 732.13' @ 12.16 hrs Surf.Area= 2,140 sf Storage= 1,334 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 30.0 min (804.9 - 774.9)

Volume	Invert	Avail.Storage	Storage Description
#1	728.25'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
			L= 60.0'
#2	728.25'	682 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2
			1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids
#3	731.25'	75 cf	5.00'W x 60.00'L x 0.25'H Mulch x 2
			150 cf Overall x 50.0% Voids
#4	731.50'	871 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	728.25'	1.020 in/hr Exfiltration over Surface area
#2	Primary	730.25'	6.0" Round Culvert X 2.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 730.25' / 730.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	732.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	730.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 2	731.00'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.05 cfs @ 12.16 hrs HW=732.12' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=1.18 cfs @ 12.16 hrs HW=732.12' TW=722.49' (Dynamic Tailwater) 2=Culvert (Passes 1.18 cfs of 2.41 cfs potential flow) -3=Orifice/Grate (Weir Controls 0.43 cfs @ 1.13 fps) -4=Orifice/Grate (Orifice Controls 0.28 cfs @ 6.44 fps) 5=Orifice/Grate (Orifice Controls 0.47 cfs @ 4.80 fps)

Summary for Pond RG52-54: Rain Garden Lots 52,53,54

 Inflow Area =
 28,140 sf, 53.72% Impervious, Inflow Depth > 5.79" for 100YearMass event

 Inflow =
 4.31 cfs @
 12.07 hrs, Volume=
 13,577 cf

 Outflow =
 3.47 cfs @
 12.14 hrs, Volume=
 13,545 cf, Atten= 20%, Lag= 4.1 min

 Primary =
 3.47 cfs @
 12.14 hrs, Volume=
 13,545 cf

 Routed to Link AP2-P : AP2-P
 13,545 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Peak Elev= 690.70' @ 12.14 hrs Surf.Area= 2,723 sf Storage= 1,740 cf

Plug-Flow detention time= 14.3 min calculated for 13,545 cf (100% of inflow) Center-of-Mass det. time= 12.7 min (809.9 - 797.1)

Volume	Invert	Avail.Storage	Storage Description
#1	686.75'	141 cf	12.0" Round Pipe Storage x 3 Inside #2
			L= 60.0'
#2	686.75'	807 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel × 3
			2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids
#3	689.75'	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
			180 cf Overall x 50.0% Voids
#4	690.00'	1,120 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3
		2,159 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	686.75'	6.0" Round Culvert X 3.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 686.75' / 686.70' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	690.50'	6.0" Horiz. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	686.75'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	689.00'	4.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=3.43 cfs @ 12.14 hrs HW=690.69' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 3.43 cfs of 5.45 cfs potential flow)

1-2=Orifice/Grate (Orifice Controls 1.25 cfs @ 2.12 fps)

-3=Orifice/Grate (Orifice Controls 0.62 cfs @ 9.46 fps)

-4=Orifice/Grate (Orifice Controls 1.56 cfs @ 5.95 fps)

Summary for Pond RG55: Rain Garden 55

 Inflow Area =
 5,450 sf, 65.00% Impervious, Inflow Depth > 6.73" for 100YearMass event

 Inflow =
 0.93 cfs @
 12.07 hrs, Volume=
 3,058 cf

 Outflow =
 0.64 cfs @
 12.16 hrs, Volume=
 3,050 cf, Atten= 31%, Lag= 5.2 min

 Primary =
 0.64 cfs @
 12.16 hrs, Volume=
 3,050 cf

 Routed to Link AP2-P : AP2-P
 AP2-P

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Peak Elev= 689.28' @ 12.16 hrs Surf.Area= 793 sf Storage= 459 cf

Plug-Flow detention time= 16.1 min calculated for 3,050 cf (100% of inflow) Center-of-Mass det. time= 14.4 min (789.3 - 774.9)

Volume	Invert	Avail.Storage	Storage Description
#1	685.75'	106 cf	18.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	685.75'	246 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	688.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	689.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		7EE of	Total Available Starage

Device	Routing	Invert	Outlet Devices
#1	Primary	685.75'	6.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 685.75' / 685.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	689.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	685.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	688.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.64 cfs @ 12.16 hrs HW=689.27' TW=0.00' (Dynamic Tailwater)

-**1=Culvert** (Passes 0.64 cfs of 1.71 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

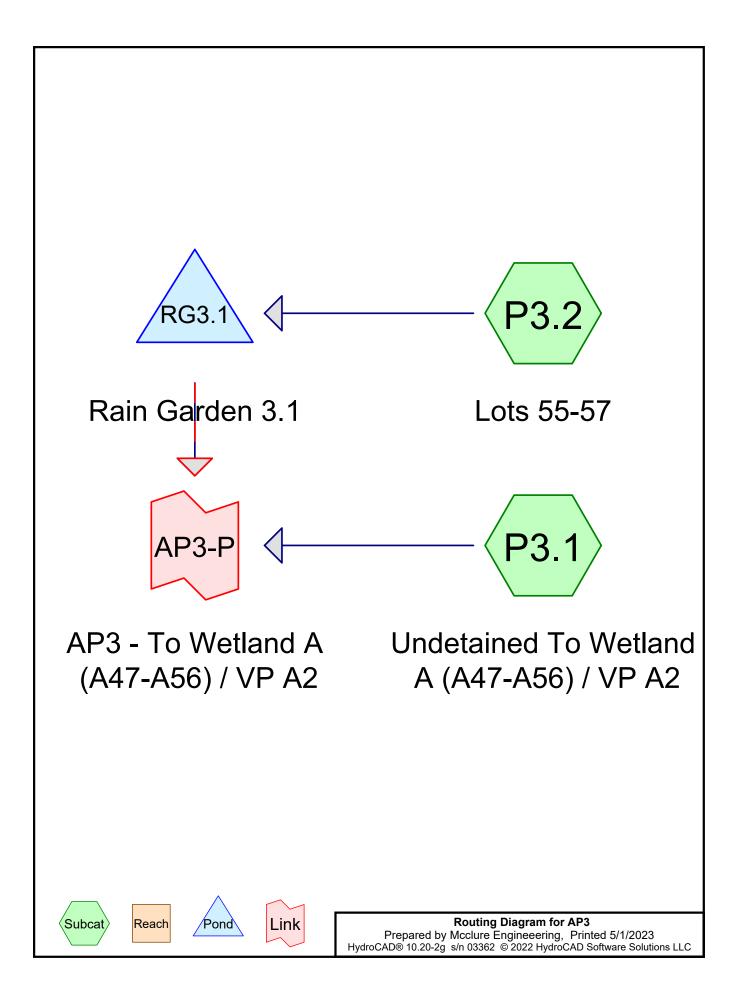
-3=Orifice/Grate (Orifice Controls 0.19 cfs @ 8.93 fps)

-4=Orifice/Grate (Orifice Controls 0.44 cfs @ 5.06 fps)

Summary for Link AP2-P: AP2-P

Inflow Are	a =	550,840 sf, 23.14% Impervious	Inflow Depth > 4.52"	for 100YearMass event
Inflow	=	47.25 cfs @ 12.16 hrs, Volume=	207,448 cf	
Primary	=	47.25 cfs @ 12.16 hrs, Volume=	207,448 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



Summary for Subcatchment P3.1: Undetained To Wetland A (A47-A56) / VP A2

Runoff = 0.33 cfs @ 12.35 hrs, Volume= 2,511 cf, Depth> 0.32" Routed to Link AP3-P : AP3 - To Wetland A (A47-A56) / VP A2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN E	Description				
	6,255	77 V	77 Woods, Good, HSG D				
	75,890	55 V	Voods, Goo	od, HSG B			
	11,530	61 >	75% Grass	s cover, Go	ood, HSG B		
	93,675	57 Weighted Average					
	93,675 100.00% Pervious Area				а		
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
8.2	50	0.0600	0.10		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.00"		
2.2	250	0.1400	1.87		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
10.4	300	Total					

Summary for Subcatchment P3.2: Lots 55-57

Runoff = 0.76 cfs @ 12.07 hrs, Volume= Routed to Pond RG3.1 : Rain Garden 3.1 2,276 cf, Depth> 1.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN E	CN Description					
	15,255	85 1	1/8 acre lots, 65% imp, HSG B					
	5,339 9,916		35.00% Pervious Area 65.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Pond RG3.1: Rain Garden 3.1

Inflow Area = 15,255 sf, 65.00% Impervious, Inflow Depth > 1.79" for 2YearMass event Inflow 0.76 cfs @ 12.07 hrs, Volume= 2.276 cf = Outflow 0.08 cfs @ 11.79 hrs, Volume= 2,277 cf, Atten= 90%, Lag= 0.0 min = 0.08 cfs @ 11.79 hrs. Volume= 2.277 cf Discarded = 0.00 cfs @ 0.00 hrs, Volume= Primary = 0 cf Routed to Link AP3-P : AP3 - To Wetland A (A47-A56) / VP A2

	Postdevelopment
AP3	Type III 24-hr 2YearMass Rainfall=3.24"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Sc	blutions LLC Page 3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 684.88' @ 12.90 hrs Surf.Area= 1,430 sf Storage= 801 cf Flood Elev= 687.50' Surf.Area= 4,668 sf Storage= 3,120 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 79.4 min (903.4 - 824.0)

Volume	Invert	Avail.Stor	vail.Storage Storage Description	
#1	683.50'	20 cf		
#2	683.50'	1,708 cf		L= 100.0' 13.00'W x 110.00'L x 3.00'H Soil Media and Gravel 4,290 cf Overall - 20 cf Embedded = 4,270 cf x 40.0% Voids
#3	686.50'	179 cf		13.00'W x 110.00'L x 0.25'H Mulch 358 cf Overall x 50.0% Voids
#4	686.75'	2,717 cf		13.00'W x 110.00'L x 1.50'H Ponding Z=2.0
		4,62	23 cf	Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Discarded	683.50'	2.41	0 in/hr Exfiltration over Surface area
#2	Primary	685.50'	6.0"	Round Culvert X 3.00
				2.0' CPP, square edge headwall, Ke= 0.500
			Inlet	/ Outlet Invert= 685.50' / 685.00' S= 0.0417 '/' Cc= 0.900
				.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	685.50'	0.5"	Vert. Orifice/Grate X 3.00 C= 0.600
			Limi	ted to weir flow at low heads
#4	Device 2	686.50'	3.0"	Vert. Orifice/Grate X 3.00 C= 0.600
			Limi	ted to weir flow at low heads
#5	Device 2	687.25'		Horiz. Orifice/Grate X 3.00 C= 0.600 ted to weir flow at low heads

Discarded OutFlow Max=0.08 cfs @ 11.79 hrs HW=683.55' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=683.50' TW=0.00' (Dynamic Tailwater) -2=Culvert (Controls 0.00 cfs)

- -3=Orifice/Grate (Controls 0.00 cfs)
- -4=Orifice/Grate (Controls 0.00 cfs)

5=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP3-P: AP3 - To Wetland A (A47-A56) / VP A2

Inflow Area =	108,930 sf,	9.10% Impervious,	Inflow Depth > 0.28"	for 2YearMass event
Inflow =	0.33 cfs @ 1	12.35 hrs, Volume=	2,511 cf	
Primary =	0.33 cfs @ 1	12.35 hrs, Volume=	2,511 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P3.1: Undetained To Wetland A (A47-A56) / VP A2

Runoff = 2.07 cfs @ 12.17 hrs, Volume= 8,803 cf, Depth> 1.13" Routed to Link AP3-P : AP3 - To Wetland A (A47-A56) / VP A2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

 A	rea (sf)	CN [Description					
	6,255	77 \	Woods, Good, HSG D					
	75,890	55 \	Woods, Good, HSG B					
	11,530	61 >	>75% Gras	s cover, Go	bod, HSG B			
	93,675	57 \	Veighted A	verage				
	93,675		100.00% Pe	ervious Are	а			
Tc	Length	Slope	Velocity	Capacity	Description			
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
 8.2	50	0.0600	0.10		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
2.2	250	0.1400	1.87		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
 10.4	300	Total						

Summary for Subcatchment P3.2: Lots 55-57

Runoff = 1.44 cfs @ 12.07 hrs, Volume= Routed to Pond RG3.1 : Rain Garden 3.1 4,337 cf, Depth> 3.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN E	Description				
	15,255	85 1	/8 acre lots	s, 65% imp	o, HSG B		
	5,339 9,916	-	35.00% Pervious Area 65.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

Summary for Pond RG3.1: Rain Garden 3.1

Inflow Area = 15,255 sf, 65.00% Impervious, Inflow Depth > 3.41" for 10YearMass event Inflow 1.44 cfs @ 12.07 hrs, Volume= 4.337 cf = Outflow 0.18 cfs @ 12.63 hrs, Volume= 4,338 cf, Atten= 87%, Lag= 33.1 min = 0.16 cfs @ 12.51 hrs, Volume= Discarded = 4.036 cf 0.02 cfs @ 12.63 hrs, Volume= Primary = 302 cf Routed to Link AP3-P : AP3 - To Wetland A (A47-A56) / VP A2

		Postdevelopment
AP3	Type III 24-hr	10YearMass Rainfall=5.05"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	olutions LLC	Page 5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 686.52' @ 12.63 hrs Surf.Area= 2,860 sf Storage= 1,744 cf Flood Elev= 687.50' Surf.Area= 4,668 sf Storage= 3,120 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 165.3 min (970.9 - 805.7)

Volume	Invert	Avail.Sto	rage	Storage Description		
#1	683.50'		20 cf	6.0" Round Pipe Storage Inside #2		
# 0		4 70	00 -f	L= 100.0'		
#2	683.50'	1,70		13.00'W x 110.00'L x 3.00'H Soil Media and Gravel 4,290 cf Overall - 20 cf Embedded = 4,270 cf x 40.0% Voids		
#3	686.50'	17	79 cf	13.00'W x 110.00'L x 0.25'H Mulch		
				358 cf Overall x 50.0% Voids		
#4	686.75'	2,71	17 cf	13.00'W x 110.00'L x 1.50'H Ponding Z=2.0		
		4,62	23 cf	Total Available Storage		
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	683.50'	2.41	0 in/hr Exfiltration over Surface area		
#2	Primary	685.50'	6.0"	Round Culvert X 3.00		
	-		L= 1	2.0' CPP, square edge headwall, Ke= 0.500		
			Inlet	/ Outlet Invert= 685.50' / 685.00' S= 0.0417 '/' Cc= 0.900		
			n= 0	.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#3	Device 2	685.50'		Vert. Orifice/Grate X 3.00 C= 0.600		
			Limi	ted to weir flow at low heads		
#4	Device 2	686.50'	3.0"	Vert. Orifice/Grate X 3.00 C= 0.600		
			Limi	ted to weir flow at low heads		
#5	Device 2	687.25'	6.0"	Horiz. Orifice/Grate X 3.00 C= 0.600		
			Limi	ted to weir flow at low heads		

Discarded OutFlow Max=0.16 cfs @ 12.51 hrs HW=686.50' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.16 cfs)

Primary OutFlow Max=0.02 cfs @ 12.63 hrs HW=686.52' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 0.02 cfs of 2.49 cfs potential flow)

3=Orifice/Grate (Orifice Controls 0.02 cfs @ 4.82 fps)

-4=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.52 fps)

5=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP3-P: AP3 - To Wetland A (A47-A56) / VP A2

Inflow Area =	108,930 sf, 9.10% Impervious,	Inflow Depth > 1.00"	for 10YearMass event
Inflow =	2.07 cfs @ 12.17 hrs, Volume=	9,104 cf	
Primary =	2.07 cfs @ 12.17 hrs, Volume=	9,104 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P3.1: Undetained To Wetland A (A47-A56) / VP A2

Runoff = 3.56 cfs @ 12.16 hrs, Volume= 13,905 cf, Depth> 1.78" Routed to Link AP3-P : AP3 - To Wetland A (A47-A56) / VP A2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

 A	rea (sf)	CN [Description					
	6,255	77 \	Woods, Good, HSG D					
	75,890	55 \	Woods, Good, HSG B					
	11,530	61 >	>75% Gras	s cover, Go	bod, HSG B			
	93,675	57 \	Veighted A	verage				
	93,675		100.00% Pe	ervious Are	а			
Tc	Length	Slope	Velocity	Capacity	Description			
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
 8.2	50	0.0600	0.10		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
2.2	250	0.1400	1.87		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
 10.4	300	Total						

Summary for Subcatchment P3.2: Lots 55-57

Runoff = 1.86 cfs @ 12.07 hrs, Volume= Routed to Pond RG3.1 : Rain Garden 3.1 5,681 cf, Depth> 4.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN E	Description				
	15,255	85 1	/8 acre lots	s, 65% imp	o, HSG B		
	5,339 9,916	-	35.00% Pervious Area 65.00% Impervious Area				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

Summary for Pond RG3.1: Rain Garden 3.1

Inflow Area = 15,255 sf, 65.00% Impervious, Inflow Depth > 4.47" for 25YearMass event Inflow 1.86 cfs @ 12.07 hrs, Volume= 5.681 cf = Outflow 0.56 cfs @ 12.39 hrs, Volume= 5,643 cf, Atten= 70%, Lag= 19.1 min = 0.24 cfs @ 12.39 hrs, Volume= Discarded = 4.761 cf 0.32 cfs @ 12.39 hrs, Volume= Primary = 882 cf Routed to Link AP3-P : AP3 - To Wetland A (A47-A56) / VP A2

	Postdevelopment
AP3	Type III 24-hr 25YearMass Rainfall=6.18"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC Page 7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 686.80' @ 12.39 hrs Surf.Area= 4,313 sf Storage= 1,972 cf Flood Elev= 687.50' Surf.Area= 4,668 sf Storage= 3,120 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 142.8 min (940.9 - 798.1)

Volume	Invert	Avail.Stor	rage	Storage Description
#1	683.50'	2	20 cf	6.0" Round Pipe Storage Inside #2
#2	683.50'	1,70)8 cf	L= 100.0' 13.00'W x 110.00'L x 3.00'H Soil Media and Gravel
#3	686.50'	17	79 cf	4,290 cf Overall - 20 cf Embedded = 4,270 cf x 40.0% Voids 13.00'W x 110.00'L x 0.25'H Mulch
			• • •	358 cf Overall x 50.0% Voids
#4	686.75'	2,71	17 cf	13.00'W x 110.00'L x 1.50'H Ponding Z=2.0
		4,62	23 cf	Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Discarded	683.50'	2.41	0 in/hr Exfiltration over Surface area
#2	Primary	685.50'	6.0"	Round Culvert X 3.00
			L= 1	2.0' CPP, square edge headwall, Ke= 0.500
			Inlet	/ Outlet Invert= 685.50' / 685.00' S= 0.0417 '/' Cc= 0.900
			n= 0	.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	685.50'	0.5"	Vert. Orifice/Grate X 3.00 C= 0.600
			Limi	ted to weir flow at low heads
#4	Device 2	686.50'		Vert. Orifice/Grate X 3.00 C= 0.600
			Limi	ted to weir flow at low heads
#5	Device 2	687.25'		Horiz. Orifice/Grate X 3.00 C= 0.600
			Limi	ted to weir flow at low heads

Discarded OutFlow Max=0.24 cfs @ 12.39 hrs HW=686.80' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.24 cfs)

Primary OutFlow Max=0.32 cfs @ 12.39 hrs HW=686.80' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 0.32 cfs of 2.90 cfs potential flow)

3=Orifice/Grate (Orifice Controls 0.02 cfs @ 5.44 fps)

4=Orifice/Grate (Orifice Controls 0.29 cfs @ 1.99 fps)

5=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP3-P: AP3 - To Wetland A (A47-A56) / VP A2

Inflow Area =	108,930 sf, 9.10% Impervious,	Inflow Depth > 1.63"	for 25YearMass event
Inflow =	3.58 cfs @ 12.16 hrs, Volume=	14,787 cf	
Primary =	3.58 cfs @ 12.16 hrs, Volume=	14,787 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P3.1: Undetained To Wetland A (A47-A56) / VP A2

Runoff = 6.21 cfs @ 12.15 hrs, Volume= 22,989 cf, Depth> 2.94" Routed to Link AP3-P : AP3 - To Wetland A (A47-A56) / VP A2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN E	Description					
	6,255	77 V	Woods, Good, HSG D					
	75,890	55 V	Noods, Good, HSG B					
	11,530	61 >	>75% Grass cover, Good, HSG B					
	93,675	57 V	57 Weighted Average					
	93,675		100.00% Pervious Area					
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.2	50	0.0600	0.10		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
2.2	250	0.1400	1.87		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
10.4	300	Total						

Summary for Subcatchment P3.2: Lots 55-57

Runoff = 2.52 cfs @ 12.07 hrs, Volume= Routed to Pond RG3.1 : Rain Garden 3.1 7,807 cf, Depth> 6.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN E	Description							
	15,255	85 1	1/8 acre lots, 65% imp, HSG B							
	5,339 9,916	-	35.00% Pervious Area 65.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					

Summary for Pond RG3.1: Rain Garden 3.1

Inflow Area = 15,255 sf, 65.00% Impervious, Inflow Depth > 6.14" for 100YearMass event Inflow 2.52 cfs @ 12.07 hrs, Volume= 7.807 cf = Outflow 0.81 cfs @ 12.36 hrs, Volume= 7,462 cf, Atten= 68%, Lag= 17.3 min = 0.25 cfs @ 12.36 hrs, Volume= Discarded = 5.450 cf 0.56 cfs @ 12.36 hrs, Volume= Primary = 2.013 cf Routed to Link AP3-P : AP3 - To Wetland A (A47-A56) / VP A2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 687.19' @ 12.36 hrs Surf.Area= 4,507 sf Storage= 2,576 cf Flood Elev= 687.50' Surf.Area= 4,668 sf Storage= 3,120 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 102.0 min (891.3 - 789.3)

Volume	Invert	Avail.Stor	rage	Storage Description		
#1	683.50'	2	20 cf	6.0" Round Pipe Storage Inside #2		
#2	683.50'	1,70)8 cf	L= 100.0' 13.00'W x 110.00'L x 3.00'H Soil Media and Gravel 4,290 cf Overall - 20 cf Embedded = 4,270 cf x 40.0% Voids		
#3	686.50'	-		13.00'W x 110.00'L x 0.25'H Mulch		
#4	686.75'			358 cf Overall x 50.0% Voids 13.00'W x 110.00'L x 1.50'H Ponding Z=2.0		
		4,62	23 cf	Total Available Storage		
Device	Routing	Invert	Outl	et Devices		
#1	Discarded	683.50'	2.41	0 in/hr Exfiltration over Surface area		
#2	Primary	685.50'	6.0"	Round Culvert X 3.00		
			Inlet	2.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 685.50' / 685.00' S= 0.0417 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#3	Device 2	685.50'	0.5"	Vert. Orifice/Grate X 3.00 C= 0.600		
#4	Device 2	686.50'				
#5	Device 2	687.25'	6.0"	ted to weir flow at low heads Horiz. Orifice/Grate X 3.00 C= 0.600 ted to weir flow at low heads		

Discarded OutFlow Max=0.25 cfs @ 12.36 hrs HW=687.19' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.25 cfs)

Primary OutFlow Max=0.56 cfs @ 12.36 hrs HW=687.19' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 0.56 cfs of 3.40 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.03 cfs @ 6.21 fps)

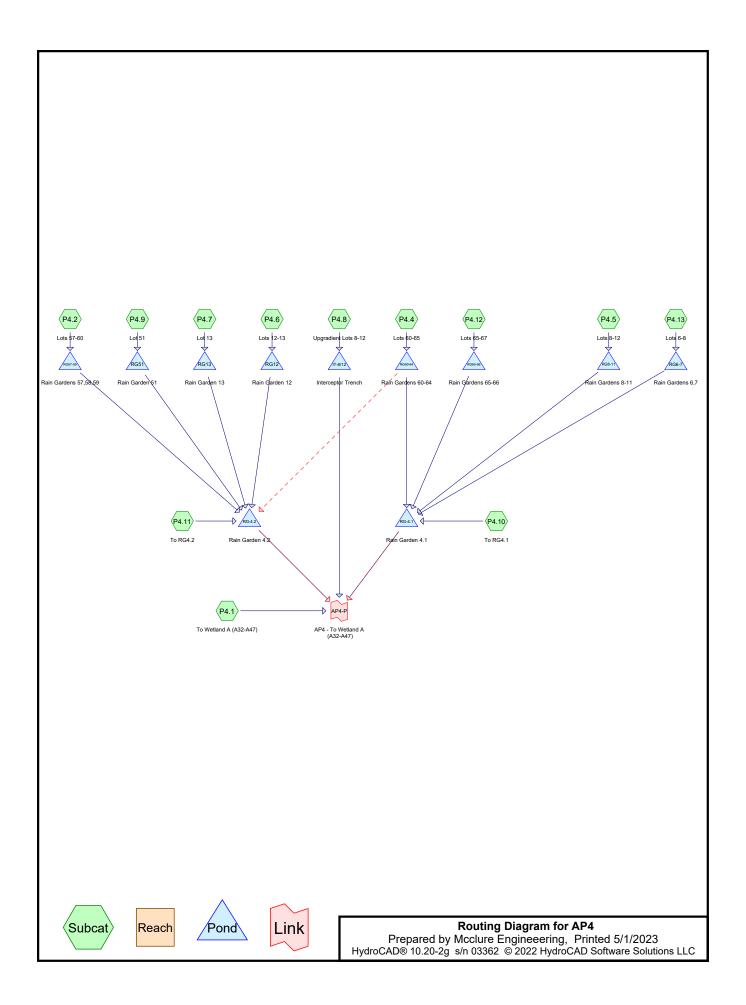
-4=Orifice/Grate (Orifice Controls 0.53 cfs @ 3.60 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP3-P: AP3 - To Wetland A (A47-A56) / VP A2

Inflow Area	a =	108,930 sf,	9.10% Imperv	ious, Inflov	v Depth >	2.75"	for	100YearMass event
Inflow	=	6.69 cfs @ 1	12.15 hrs, Volu	me=	25,002 0	of		
Primary	=	6.69 cfs @ 1	12.15 hrs, Volu	me=	25,002 0	of, Atter	%0 =ר	6, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs



Summary for Subcatchment P4.1: To Wetland A (A32-A47)

Runoff = 2.65 cfs @ 12.11 hrs, Volume= 8,767 cf, Depth= 1.18" Routed to Link AP4-P : AP4 - To Wetland A (A32-A47)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Α	rea (sf)	CN E	Description		
	630	55 V	Voods, Go	od, HSG B	
	73,383	77 V	Voods, Go	od, HSG D	
	4,661	61 >	75% Gras	s cover, Go	bod, HSG B
	10,490	80 >	75% Gras	s cover, Go	bod, HSG D
	89,164	76 V	Veighted A	verage	
	89,164	1	00.00% Pe	ervious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.7	50	0.1500	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.2	135	0.1500	1.94		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
6.9	185	Total			

Summary for Subcatchment P4.10: To RG4.1

Runoff = 0.46 cfs @ 12.11 hrs, Volume= Routed to Pond RG-4.1 : Rain Garden 4.1 1,488 cf, Depth= 1.43"

	A	rea (sf)	CN I	Description						
		12,335	80 :	>75% Gras	s cover, Go	ood, HSG D				
		125	61 ;	>75% Gras	s cover, Go	ood, HSG B				
		12,460	80 V	Neighted A	verage					
		12,460		100.00% Pe	ervious Are	а				
		Length	Slope		Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.7	50	0.1500	0.15		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.00"				
	1.3	150	0.1500	1.94		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	7.0	200	Total							

Summary for Subcatchment P4.11: To RG4.2

Runoff = 0.12 cfs @ 12.14 hrs, Volume= Routed to Pond RG-4.2 : Rain Garden 4.2 600 cf, Depth= 0.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

_	A	rea (sf)	CN	Description							
		15,520									
_		110	80	>75% Gras	s cover, Go	ood, HSG D					
		15,630	61	Weighted A	verage						
		15,630		100.00% Pe	ervious Are	а					
	Тс	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.7	50	0.1500	0.15		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.00"					
	1.3	150	0.1500	1.94		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
_	7.0	200	Total								

Summary for Subcatchment P4.12: Lots 65-67

Runoff	=	0.69 cfs @	12.07 hrs,	Volume=	2,111 cf,	Depth=	2.30"
Routed	to Pond	d RG65-66 : R	ain Garden	s 65-66			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Ar	rea (sf)	CN	Description						
	5,185	90	1/8 acre lots	s, 65% imp	, HSG C				
	5,850	92	1/8 acre lots	s, 65% imp	, HSG D				
	11,035	91	Weighted A	verage					
	3,862		35.00% Per	vious Area					
	7,173		65.00% Imp	pervious Ar	ea				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry,				

Summary for Subcatchment P4.13: Lots 6-8

Runoff = 1.17 cfs @ 12.07 hrs, Volume= 3,501 cf, Depth= 1.79" Routed to Pond RG6-7 : Rain Gardens 6,7

Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

AP4

 Prepared by Mcclure Engineeering
 Printed 5/1/2023

 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC
 Page 4

A	rea (sf)	CN	Description		
	23,175	85	1/8 acre lots	s, 65% imp	, HSG B
	270	90	1/8 acre lots	s, 65% imp	, HSG C
	23,445	85	Weighted A	verage	
	8,206		35.00% Per	vious Area	
	15,239		65.00% Imp	pervious Are	ea
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P4.2: Lots 57-60

Runoff = 1.05 cfs @ 12.07 hrs, Volume= 3,126 cf, Depth= 1.79" Routed to Pond RG57-59 : Rain Gardens 57,58,59

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Area (sf)	CN	CN Description							
20,938	85	85 1/8 acre lots, 65% imp, HSG B							
7,328		35.00% Per	vious Area						
13,610		65.00% Imp	pervious Are	ea					
Tc Length (min) (feet)	Slop (ft/f	,	Capacity (cfs)	Description					
5.0				Direct Entry,					

Summary for Subcatchment P4.4: Lots 60-65

Runoff = 2.04 cfs @ 12.07 hrs, Volume= 6,114 cf, Depth= 2.03" Routed to Pond RG60-64 : Rain Gardens 60-64

A	rea (sf)	CN	Description	Description						
	21,420	85	1/8 acre lots	s, 65% imp	HSG B					
	10,990	92	1/8 acre lots	s, 65% imp	HSG D					
	3,680	90	1/8 acre lots	s, 65% imp	HSG C					
	36,090	88	Weighted A	verage						
	12,632		35.00% Per	vious Area						
	23,459		65.00% Imp	ervious Ar	a					
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description					
5.0					Direct Entry,					

Summary for Subcatchment P4.5: Lots 8-12

Runoff = 1.55 cfs @ 12.07 hrs, Volume= 4,629 cf, Depth= 1.79" Routed to Pond RG8-11 : Rain Gardens 8-11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Area (sf)	CN	Description							
31,000	85	1/8 acre lots, 65% imp, HSG B							
10,850		35.00% Pei	rvious Area						
20,150		65.00% Imp	pervious Ar	ea					
Tc Length (min) (feet)		Slope Velocity Capacity Description (ft/ft) (ft/sec) (cfs)							
5.0		Direct Entry,							
	Summary for Subactabrant D4 61 Lata 12 12								

Summary for Subcatchment P4.6: Lots 12-13

Runoff = 0.51 cfs @ 12.09 hrs, Volume= 1,594 cf, Depth= 1.79" Routed to Pond RG12 : Rain Garden 12

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

_	A	rea (sf)	CN E	Description					
		10,675	85 1	1/8 acre lots, 65% imp, HSG B					
_		3,736	3	35.00% Pervious Area					
		6,939	6	5.00% Imp	ervious Ar	ea			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	6.2	50	0.1200	0.13		Sheet Flow,			
	0.2	30	0.1000	2.21		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
-	6.4	80	Total						

Summary for Subcatchment P4.7: Lot 13

Runoff	=	0.50 cfs @	12.07 hrs,	Volume=	1,487 cf,	Depth= 1.87"
Routed	to Pond	RG13 : Rain	Garden 13			

Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

935 cf, Depth= 0.29"

Page 6

Prepared by Mcclure Engineeering

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description		
	7,505	85	1/8 acre lot	s, 65% imp	, HSG B
	2,040	90	1/8 acre lots	s, 65% imp	, HSG C
	9,545	86	Weighted Average		
	3,341		35.00% Pervious Area		
	6,204		65.00% Imp	pervious Ar	ea
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P4.8: Upgradient Lots 8-12

Runoff = 0.11 cfs @ 12.47 hrs, Volume= Routed to Pond IT-8/12 : Interceptor Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

_	A	rea (sf)	CN I	CN Description					
		3,850	61 :	>75% Gras	s cover, Go	bod, HSG B			
_		34,550	55	Woods, Go	od, HSG B				
	38,400 56 Weighted Average								
	38,400 100.00% Pervious Are					а			
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	12.7	50	0.0200	0.07	· · ·	Sheet Flow,			
	3.5	280	0.0700	1.32		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps			
	16.2	330	Total						

Summary for Subcatchment P4.9: Lot 51

Runoff = 0.48 cfs @ 12.07 hrs, Volume= 1,419 cf, Depth= 1.87" Routed to Pond RG51 : Rain Garden 51

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Area (sf)	CN	Description
1,910	90	1/8 acre lots, 65% imp, HSG C
7,195	85	1/8 acre lots, 65% imp, HSG B
9,105	86	Weighted Average
3,187		35.00% Pervious Area
5,918		65.00% Impervious Area

AP4

	cclure Engineeerir 0-2g_s/n 03362_©20) Software Soli		Postdevelo 2 <i>YearMass Rainfall=</i> Printed 5/1 F	=3.24"	
Tc Lengtl (min) (feet	n Slope Velocity	Capacity	Description			<u> </u>	
5.0			Direct Entry	y ,			
	Summar	y for Pond	1 IT-8/12: In	terceptor Tre	ench		
Inflow Area = Inflow = Outflow = Primary = Routed to Lir	Inflow = 0.11 cfs @ 12.47 hrs, Volume= 935 cf Outflow = 0.08 cfs @ 12.64 hrs, Volume= 933 cf, Atten= 20%, Lag= 10.2 min						
Peak Elev= 680	Stor-Ind method, Ti .46' @ 12.64 hrs S	urf.Area= 1,	050 sf Stora	ge= 79 cf			
	tion time= 28.5 min det. time= 27.3 min			% of inflow)			
Volume Ir	nvert Avail.Stora	ge Storag	e Description				
#1 680	0.30' 1,211			3.00'H Prisma			
#2 680).30' 122		ound Pipe S	2 cf Embedded torage Inside #	= 3,028 cf x 40.0% Vc 1	Ids	
	1,333		vailable Stora	age			
Device Routin	g Invert	Outlet Devic	es				
#1 Primar		8.0" Round					
				dge headwall,			
					: 0.0549 '/' Cc= 0.900 Flow Area= 0.35 sf		
n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf Primary OutFlow Max=0.08 cfs @ 12.64 hrs HW=680.46' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.08 cfs @ 1.35 fps)							
	Summa	ry for Por	nd RG-4.1: I	Rain Garden	4.1		
Inflow Area = Inflow = Outflow = Discarded =	114,030 sf, 57 2.43 cfs @ 12. 0.95 cfs @ 12. 0.10 cfs @ 11.	22 hrs, Volı 89 hrs, Volı	ime= ime=	Depth = 1.26" 11,949 cf 11,748 cf, Atte 7,326 cf	for 2YearMass event n= 61%, Lag= 40.3 mi		

Inflow Area =	114,030 sf	, 57.90% Impervious,	Inflow Depth = 1.26"	for 2YearMass event
Inflow =	2.43 cfs @	12.22 hrs, Volume=	11,949 cf	
Outflow =	0.95 cfs @	12.89 hrs, Volume=	11,748 cf, Atte	n= 61%, Lag= 40.3 min
Discarded =	0.10 cfs @	11.54 hrs, Volume=	7,326 cf	
Primary =	0.84 cfs @	12.89 hrs, Volume=	4,422 cf	
Routed to Link	AP4-P : AP4	- To Wetland A (A32-/	447)	
		0.00 hrs, Volume=	0 cf	
Routed to Link	AP4-P : AP4	- To Wetland A (A32-/	447)	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 671.51' @ 12.89 hrs Surf.Area= 4,320 sf Storage= 4,764 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 232.1 min (1,060.8 - 828.6) AP4

Prepared by Mcclure Engineeering

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Printed 5/1/2023 Page 8

Volume	Invert	Avail.Stor	rage	Storage Description
#1	668.75'	3.75' 5,184 cf		
40	074 751	-	10 - 5	12,960 cf Overall x 40.0% Voids
#2	671.75'	54	10 cf	12.00'W x 360.00'L x 0.25'H Mulch 1,080 cf Overall x 50.0% Voids
#3	672.00'	11,65	59 cf	12.00'W x 360.00'L x 2.00'H Ponding Z=2.0
				Total Available Storage
			.	
Device	Routing	Invert	Outl	et Devices
#1	Discarded	668.75'	1.02	0 in/hr Exfiltration over Surface area
#2	Primary	670.75'	8.0"	Round Culvert X 4.00
			L= 1	0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 670.75' / 670.65' S= 0.0100 '/' Cc= 0.900
				.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Device 2	670.75'		Vert. Orifice/Grate X 4.00 C= 0.600
110	Dovido L	010110	-	ted to weir flow at low heads
#4	Device 2	671.25'		Vert. Orifice/Grate X 4.00 C= 0.600
π -		071.20		ted to weir flow at low heads
#5	Device 2	673.25'		Horiz. Orifice/Grate X 2.00 C= 0.600
#5	Device Z	075.25		ted to weir flow at low heads
40	Device 2	672.051		
#6	Device 2	673.25'		Horiz. Orifice/Grate X 2.00 C= 0.600
	0			ted to weir flow at low heads
#7	Secondary	673.50'		' long x 10.0' breadth Broad-Crested Rectangular Weir
				d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coe	f. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=0.10 cfs @ 11.54 hrs HW=668.80' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.84 cfs @ 12.89 hrs HW=671.51' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 0.84 cfs of 3.95 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.34 cfs @ 3.95 fps)

-4=Orifice/Grate (Orifice Controls 0.50 cfs @ 1.73 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=668.75' TW=0.00' (Dynamic Tailwater) -7=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG-4.2: Rain Garden 4.2

65,893 sf, 49.58% Impervious, Inflow Depth = 1.18" for 2YearMass event Inflow Area = Inflow = 1.59 cfs @ 12.21 hrs, Volume= 6,502 cf Outflow 0.49 cfs @ 12.96 hrs, Volume= 6,408 cf, Atten= 69%, Lag= 45.1 min = Discarded = 0.06 cfs @ 11.66 hrs, Volume= 4.076 cf 0.44 cfs @ 12.96 hrs, Volume= 2,332 cf = Primary Routed to Link AP4-P : AP4 - To Wetland A (A32-A47) Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Link AP4-P : AP4 - To Wetland A (A32-A47)

Posideve	elopment
AP4 Type III 24-hr 2YearMass Rainfa	all=3.24"
Prepared by Mcclure Engineeering Printed	5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 9

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 678.36' @ 12.96 hrs Surf.Area= 2,400 sf Storage= 2,507 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 230.3 min (1,062.2 - 831.8)

Volume	Invert	Avail.Storage	Storage Description
#1	675.75'	2,880 cf	10.00'W x 240.00'L x 3.00'H Soil Media and Gravel
			7,200 cf Overall x 40.0% Voids
#2	678.75'	300 cf	10.00'W x 240.00'L x 0.25'H Mulch
			600 cf Overall x 50.0% Voids
#3	679.00'	6,843 cf	10.00'W x 240.00'L x 2.00'H Ponding Z=2.0
		10,023 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	675.75'	1.020 in/hr Exfiltration over Surface area
#2	Primary	677.75'	6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 677.75' / 677.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	677.75'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	678.25'	
			Limited to weir flow at low heads
#5	Device 2	680.50'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#6	Secondary	680.50'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=0.06 cfs @ 11.66 hrs HW=675.80' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.44 cfs @ 12.96 hrs HW=678.36' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 0.44 cfs of 2.08 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.31 cfs @ 3.50 fps)

-4=Orifice/Grate (Orifice Controls 0.13 cfs @ 1.14 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=675.75' TW=0.00' (Dynamic Tailwater) -6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG12: Rain Garden 12

 Inflow Area =
 10,675 sf, 65.00% Impervious, Inflow Depth =
 1.79" for 2YearMass event

 Inflow =
 0.51 cfs @
 12.09 hrs, Volume=
 1,594 cf

 Outflow =
 0.31 cfs @
 12.20 hrs, Volume=
 1,594 cf, Atten= 38%, Lag= 6.5 min

 Primary =
 0.31 cfs @
 12.20 hrs, Volume=
 1,594 cf

 Routed to Pond RG-4.2 : Rain Garden 4.2
 1,594 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Peak Elev= 688.79' @ 12.20 hrs Surf.Area= 240 sf Storage= 308 cf

Plug-Flow detention time= 19.0 min calculated for 1,594 cf (100% of inflow) Center-of-Mass det. time= 18.9 min (844.8 - 825.9)

Volume	Invert	Avail.Storage	Storage Description
#1	686.25'	106 cf	18.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	686.25'	246 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	689.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	689.50'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		755 of	Total Aveilable Starses

755 cf	Total <i>i</i>	Availab	le Storage
--------	----------------	---------	------------

Device	Routing	Invert	Outlet Devices
#1	Primary	686.25'	6.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 686.25' / 686.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	690.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	686.25'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	688.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.31 cfs @ 12.20 hrs HW=688.79' TW=676.53' (Dynamic Tailwater)

-1=Culvert (Passes 0.31 cfs of 1.43 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.16 cfs @ 7.55 fps)

-4=Orifice/Grate (Orifice Controls 0.15 cfs @ 1.84 fps)

Summary for Pond RG13: Rain Garden 13

Inflow Area	a =	9,545 sf, 65.00%	Impervious,	Inflow Depth =	1.87"	for 2YearMass event
Inflow	=	0.50 cfs @ 12.07 hrs	, Volume=	1,487 c	f	
Outflow	=	0.27 cfs @ 12.20 hrs	, Volume=	1,487 c	f, Atten	= 47%, Lag= 7.4 min
Primary	=	0.27 cfs @ 12.20 hrs	, Volume=	1,487 c	f	-
Routed	to Pond	RG-4.2 : Rain Garden	4.2			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 697.23' @ 12.20 hrs Surf.Area= 240 sf Storage= 301 cf

Plug-Flow detention time= 19.3 min calculated for 1,487 cf (100% of inflow) Center-of-Mass det. time= 19.3 min (840.3 - 821.1)

Postdevelopment

Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 11

Volume	Invert	Avail.Storage	Storage Description	
#1	694.75'	106 cf		
#2	694.75'	246 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids	
#3	697.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch	
#4	698.00'	373 cf	60 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0	
		755 cf		
Device	Routing	Invert Out	let Devices	
#1	Primary	L= ^ Inle	' Round Culvert 10.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 694.75' / 694.65' S= 0.0100 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf	
#2	Device 1	698.50' 6.0'	'Horiz. Orifice/Grate C= 0.600 ited to weir flow at low heads	
#3 #4	Device 1 Device 1	694.75' 2.0'	'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads 'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads	

Primary OutFlow Max=0.26 cfs @ 12.20 hrs HW=697.23' TW=676.50' (Dynamic Tailwater)

1=Culvert (Passes 0.26 cfs of 1.41 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs) -3=Orifice/Grate (Orifice Controls 0.16 cfs @ 7.45 fps)

-4=Orifice/Grate (Orifice Controls 0.10 cfs @ 1.62 fps)

Summary for Pond RG51: Rain Garden 51

Inflow Area	a =	9,105 sf, 65.0	00% Impervious,	Inflow Depth = 1	.87" for 2YearMass event
Inflow	=	0.48 cfs @ 12.0	7 hrs, Volume=	1,419 cf	
Outflow	=	0.28 cfs @ 12.1	8 hrs, Volume=	1,419 cf,	Atten= 42%, Lag= 6.2 min
Primary	=	0.28 cfs @ 12.1	8 hrs, Volume=	1,419 cf	-
Routed to Pond RG-4.2 : Rain Garden 4.2					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 694.24' @ 12.18 hrs Surf.Area= 240 sf Storage= 268 cf

Plug-Flow detention time= 17.4 min calculated for 1,418 cf (100% of inflow) Center-of-Mass det. time= 17.3 min (838.4 - 821.1)

Volume	Invert	Avail.Storage	Storage Description
#1	691.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	691.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	694.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	695.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage

AP4

Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24"

Printed 5/1/2023

Page 12

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g_s/n 03362 © 2022 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	691.75'	6.0" Round Culvert
	-		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 691.75' / 691.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	695.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	691.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	694.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.28 cfs @ 12.18 hrs HW=694.24' TW=676.39' (Dynamic Tailwater)

1=Culvert (Passes 0.28 cfs of 1.42 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.16 cfs @ 7.47 fps)

-4=Orifice/Grate (Orifice Controls 0.11 cfs @ 1.68 fps)

Summary for Pond RG57-59: Rain Gardens 57,58,59

Inflow Area =	20,938 sf, 65.00% Impervious,	Inflow Depth = 1.79" for 2YearMass event			
Inflow =	1.05 cfs @ 12.07 hrs, Volume=	3,126 cf			
Outflow =	0.52 cfs @ 12.22 hrs, Volume=	3,127 cf, Atten= 50%, Lag= 8.6 min			
Discarded =	0.04 cfs @ 11.37 hrs, Volume=	2,092 cf			
Primary =	0.48 cfs @ 12.22 hrs, Volume=	1,035 cf			
Routed to Pond RG-4.2 : Rain Garden 4.2					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 689.94' @ 12.22 hrs Surf.Area= 720 sf Storage= 860 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 118.0 min (942.6 - 824.6)

Volume	Invert	Avail.Storage	Storage Description
#1	687.25'	141 cf	12.0" Round Pipe Storage x 3 Inside #2
			L= 60.0'
#2	687.25'	807 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 3
			2,160 cf Overall - 141 cf Embedded = 2,019 cf \times 40.0% Voids
#3	690.25'	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
			180 cf Overall x 50.0% Voids
#4	690.50'	1,120 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 3
		2,159 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	687.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	689.25'	6.0" Round Culvert X 3.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 689.25' / 689.00' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	691.00'	6.0" Horiz. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	689.25'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads

AP4

	Postdevelopment
AP4	Type III 24-hr 2YearMass Rainfall=3.24"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software So	blutions LLC Page 13

#5 Device 2 689.75' 4.0" Vert. Orifice/Grate X 3.00 C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 11.37 hrs HW=687.29' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.48 cfs @ 12.22 hrs HW=689.94' TW=676.62' (Dynamic Tailwater)

-**2=Culvert** (Passes 0.48 cfs of 1.89 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.25 cfs @ 3.76 fps)

-5=Orifice/Grate (Orifice Controls 0.24 cfs @ 1.50 fps)

Summary for Pond RG6-7: Rain Gardens 6,7

Inflow Area =		23,445 sf, 65.00% Impervious	, Inflow Depth = 1.79" for 2YearMass event			
Inflow	=	1.17 cfs @ 12.07 hrs, Volume=	3,501 cf			
Outflow	=	0.62 cfs @ 12.20 hrs, Volume=	3,500 cf, Atten= 47%, Lag= 7.5 min			
Primary	=	0.62 cfs @ 12.20 hrs, Volume=	3,500 cf			
Routed to Pond RG-4.1 : Rain Garden 4.1						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 678.60' @ 12.20 hrs Surf.Area= 600 sf Storage= 500 cf

Plug-Flow detention time= 11.4 min calculated for 3,500 cf (100% of inflow) Center-of-Mass det. time= 11.3 min (835.9 - 824.6)

Volume	Invert	Avail.Storage	Storage Description
#1	676.75'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
			L= 60.0'
#2	676.75'	682 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel × 2
			1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids
#3	679.75'	75 cf	5.00'W x 60.00'L x 0.25'H Mulch x 2
			150 cf Overall x 50.0% Voids
#4	680.00'	871 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	676.75'	6.0" Round Culvert X 2.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 676.75' / 676.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	680.50'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	676.75'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	679.00'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

	Postdevelopment
AP4	Type III 24-hr 2YearMass Rainfall=3.24"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software So	olutions LLC Page 14

Primary OutFlow Max=0.62 cfs @ 12.20 hrs HW=678.60' TW=669.79' (Dynamic Tailwater) **1=Culvert** (Passes 0.62 cfs of 2.39 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.62 cfs @ 6.32 fps)

4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond RG60-64: Rain Gardens 60-64

Inflow Area =	36,090 sf, 65.00% Im	pervious, Inflow Depth = 2.03	3" for 2YearMass event
Inflow =	2.04 cfs @ 12.07 hrs,	Volume= 6,114 cf	
Outflow =	0.94 cfs @ 12.23 hrs, `	Volume= 6,114 cf, A	tten= 54%, Lag= 9.7 min
Discarded =	0.08 cfs @ 11.30 hrs, 1	Volume= 4,271 cf	
Primary =	0.69 cfs @ 12.23 hrs, 1	Volume= 1,475 cf	
Routed to Pond	RG-4.1 : Rain Garden 4.	1	
Secondary =	0.17 cfs @ 12.23 hrs, 1	Volume= 369 cf	
Routed to Pond	RG-4.2 : Rain Garden 4.	2	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 686.46' @ 12.23 hrs Surf.Area= 1,500 sf Storage= 1,766 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 113.6 min (927.3 - 813.6)

Volume	Invert	Avail.Storage	Storage Description
#1	683.75'	236 cf	12.0" Round Pipe Storage x 5 Inside #2
			L= 60.0'
#2	683.75'	1,706 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 5
			4,500 cf Overall - 236 cf Embedded = 4,264 cf x 40.0% Voids
#3	686.75'	188 cf	5.00'W x 60.00'L x 0.25'H Mulch x 5
			375 cf Overall x 50.0% Voids
#4	687.00'	2,177 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 5
		4,306 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1 #2	Discarded Primary	683.75' 685.75'	 2.410 in/hr Exfiltration over Surface area 6.0" Round Culvert X 4.00 L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 685.75' / 684.65' S= 0.1100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Secondary	685.75'	6.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 685.75' / 684.65' S= 0.1100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#4	Device 2	687.50'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 3	687.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#6	Device 2	685.75'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#7	Device 3	685.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

AP4Type III 24-hr2YearMass Rainfall=3.24"Prepared by Mcclure EngineeeringPrinted5/1/2023HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLCPage 15

#8 Device 2
686.25' 4.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
#9 Device 3
686.25' 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.08 cfs @ 11.30 hrs HW=683.79' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=0.69 cfs @ 12.23 hrs HW=686.46' TW=669.95' (Dynamic Tailwater)

-**2=Culvert** (Passes 0.69 cfs of 2.56 cfs potential flow)

4=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Orifice Controls 0.33 cfs @ 3.81 fps)

Secondary OutFlow Max=0.17 cfs @ 12.23 hrs HW=686.46' TW=676.72' (Dynamic Tailwater)

3=Culvert (Passes 0.17 cfs of 0.64 cfs potential flow)

5=Orifice/Grate (Controls 0.00 cfs)

-7=Orifice/Grate (Orifice Controls 0.08 cfs @ 3.81 fps)

-9=Orifice/Grate (Orifice Controls 0.09 cfs @ 1.55 fps)

Summary for Pond RG65-66: Rain Gardens 65-66

Inflow Area =	11,035 sf, 65.00% Impervious,	Inflow Depth = 2.30" for 2YearMass event
Inflow =	0.69 cfs @ 12.07 hrs, Volume=	2,111 cf
Outflow =	0.35 cfs @ 12.21 hrs, Volume=	1,980 cf, Atten= 50%, Lag= 8.0 min
Discarded =	0.01 cfs @ 9.98 hrs, Volume=	1,121 cf
Primary =	0.33 cfs @ 12.21 hrs, Volume=	859 cf
Routed to Pond	d RG-4.1 : Rain Garden 4.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 677.95' @ 12.21 hrs Surf.Area= 600 sf Storage= 704 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 184.3 min (985.4 - 801.1)

Volume	Invert	Avail.Stor	rage Storage Description		
#1	675.25'	g	94 cf	12.0" Round Pipe Storage x 2 Inside #2 L= 60.0'	
#2	675.25'	68	82 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids	
#3	678.25'	75 cf		5.00'W x 60.00'L x 0.25'H Mulch x 2 150 cf Overall x 50.0% Voids	
#4	678.50'	871 cf		5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2	
		1,72	22 cf	Total Available Storage	
Device	Routing	Invert	Outle	et Devices	
#1	Discarded	675.25'	1.02	0 in/hr Exfiltration over Surface area	
#2	Primary	677.25'	6.0"	Round Culvert X 2.00	
	-		L= 10.0' CPP, square edge headwall, Ke= 0.500		
			Inlet	/ Outlet Invert= 677.25' / 675.15' S= 0.2100 '/' Cc= 0.900	
			n= 0	.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf	
#3	Device 2	679.00'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600	

	Postdevelopment
AP4	Type III 24-hr 2YearMass Rainfall=3.24"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software So	olutions LLC Page 16

			Limited to weir flow at low heads
#4	Device 2	677.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 2	677.75'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 9.98 hrs HW=675.27' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.33 cfs @ 12.21 hrs HW=677.95' TW=669.81' (Dynamic Tailwater) 2=Culvert (Passes 0.33 cfs of 1.27 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -4=Orifice/Grate (Orifice Controls 0.16 cfs @ 3.78 fps) -5=Orifice/Grate (Orifice Controls 0.17 cfs @ 1.52 fps)

Summary for Pond RG8-11: Rain Gardens 8-11

Inflow Are	a =	31,000 sf, 65.00% Impervious,	Inflow Depth = 1.79" for 2YearMass event
Inflow	=	1.55 cfs @ 12.07 hrs, Volume=	4,629 cf
Outflow	=	0.53 cfs @ 12.37 hrs, Volume=	4,627 cf, Atten= 66%, Lag= 17.4 min
Primary	=	0.53 cfs @ 12.37 hrs, Volume=	4,627 cf
Routed	l to Pond	d RG-4.1 : Rain Garden 4.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 677.45' @ 12.37 hrs Surf.Area= 1,200 sf Storage= 1,068 cf

Plug-Flow detention time= 23.0 min calculated for 4,627 cf (100% of inflow) Center-of-Mass det. time= 22.8 min (847.4 - 824.6)

Volume	Invert	Avail.Stor	rage	Storage Description		
#1	675.75'	42	24 cf			
#2	675.75'	1,27	70 cf	L= 60.0' 5.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 4 3,600 cf Overall - 424 cf Embedded = 3,176 cf x 40.0% Voids		
#3	678.75'	15	50 cf			
				300 cf Overall x 50.0% Voids		
#4	679.00'	1,74	11 ct	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 4		
		3,586 cf		Total Available Storage		
				-		
Device	Routing	Invert	Outle	et Devices		
#1	Primary	675.75'	6.0"	Round Culvert X 4.00		
			L= 1	0.0' CPP, square edge headwall, Ke= 0.500		
				/ Outlet Invert= 675.75' / 675.65' S= 0.0100 '/' Cc= 0.900		
			n= 0	.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	679.50'		Horiz. Orifice/Grate X 4.00 C= 0.600		
			Limi	ted to weir flow at low heads		
#3	Device 1	675.75'	2.0"	Vert. Orifice/Grate X 4.00 C= 0.600		
			Limi	ted to weir flow at low heads		
#4	Device 1	678.00'	4.0"	Vert. Orifice/Grate X 4.00 C= 0.600		
			Limi	ted to weir flow at low heads		

Primary OutFlow Max=0.53 cfs @ 12.37 hrs HW=677.45' TW=670.54' (Dynamic Tailwater) 1=Culvert (Passes 0.53 cfs of 4.55 cfs potential flow) 2=Orifice/Grate (Controls 0.00 cfs) 3=Orifice/Grate (Orifice Controls 0.53 cfs @ 6.11 fps) 4=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP4-P: AP4 - To Wetland A (A32-A47)

 Inflow Area =
 307,487 sf, 32.10% Impervious, Inflow Depth =
 0.64" for 2YearMass event

 Inflow =
 2.65 cfs @
 12.11 hrs, Volume=
 16,455 cf

 Primary =
 2.65 cfs @
 12.11 hrs, Volume=
 16,455 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P4.1: To Wetland A (A32-A47)

Runoff = 5.98 cfs @ 12.10 hrs, Volume= 19,146 cf, Depth= 2.58" Routed to Link AP4-P : AP4 - To Wetland A (A32-A47)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Α	rea (sf)	CN E	Description					
	630	55 V	Voods, Go	od, HSG B				
	73,383	77 V	Voods, Go	od, HSG D				
	4,661	61 >	75% Gras	s cover, Go	bod, HSG B			
	10,490	80 >	75% Gras	s cover, Go	bod, HSG D			
	89,164	76 V	Veighted A	verage				
	89,164	1	00.00% Pe	ervious Are	a			
Тс	Length	Slope		Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	50	0.1500	0.15		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
1.2	135	0.1500	1.94		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
6.9	185	Total						

9 185 I otal

Summary for Subcatchment P4.10: To RG4.1

Runoff = 0.95 cfs @ 12.10 hrs, Volume= Routed to Pond RG-4.1 : Rain Garden 4.1 3,049 cf, Depth= 2.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

 A	rea (sf)	CN I	Description						
	12,335	80 :	>75% Gras	s cover, Go	bod, HSG D				
	125	61 >	>75% Gras	s cover, Go	ood, HSG B				
	12,460	80 V	Neighted A	verage					
	12,460		100.00% Pe	ervious Are	а				
	Length	Slope		Capacity	Description				
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.7	50	0.1500	0.15		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.00"				
1.3	150	0.1500	1.94		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
7.0	200	Total							

Summary for Subcatchment P4.11: To RG4.2

Runoff = 0.52 cfs @ 12.11 hrs, Volume= 1,822 cf, Depth= 1.40" Routed to Pond RG-4.2 : Rain Garden 4.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN I	Description							
		15,520									
_		110	80 :	>75% Gras	s cover, Go	ood, HSG D					
		15,630 61 Weighted Average									
		15,630	·	100.00% Pe	ervious Are	a					
	Tc	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.7	50	0.1500	0.15		Sheet Flow,					
						Woods: Light underbrush n= 0.400 P2= 3.00"					
	1.3	150	0.1500	1.94		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
_	7.0	200	Total								

Summary for Subcatchment P4.12: Lots 65-67

Runoff	=	1.19 cfs @ 12.0	7 hrs, Volume=	3,707 cf,	Depth= 4.03"
Routed	I to Pond	RG65-66 : Rain G	Gardens 65-66		-

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Ar	rea (sf)	CN	Description					
	5,185	90	1/8 acre lots	s, 65% imp	, HSG C			
	5,850	92	1/8 acre lots, 65% imp, HSG D					
	11,035	91	Weighted Average					
	3,862		35.00% Per	vious Area				
	7,173		65.00% Imp	pervious Ar	ea			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

Summary for Subcatchment P4.13: Lots 6-8

Runoff = 2.21 cfs @ 12.07 hrs, Volume= 6,671 cf, Depth= 3.41" Routed to Pond RG6-7 : Rain Gardens 6,7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

Page 20

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Ar	rea (sf)	CN	Description		
	23,175	85	1/8 acre lots	s, 65% imp	, HSG B
	270	90	1/8 acre lots	s, 65% imp	, HSG C
	23,445	85	Weighted A	verage	
	8,206		35.00% Per	vious Area	
	15,239		65.00% Imp	ervious Are	ea
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P4.2: Lots 57-60

Runoff = 1.97 cfs @ 12.07 hrs, Volume= 5,957 cf, Depth= 3.41" Routed to Pond RG57-59 : Rain Gardens 57,58,59

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Area (sf)	CN	N Description							
20,938	85	85 1/8 acre lots, 65% imp, HSG B							
7,328		35.00% Pervious Area							
13,610		65.00% Imp	pervious Ar	rea					
Tc Lengtł <u>(min) (feet</u> 5.0			Capacity (cfs)	Description Direct Entry,					

Summary for Subcatchment P4.4: Lots 60-65

Runoff	=	3.65 cfs @	12.07 hrs,	Volume=	11,177 cf,	Depth= 3.72"
Routed	to Pond	I RG60-64 : F	≀ain Garden	s 60-64		

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN	Description							
	21,420	85	1/8 acre lots	s, 65% imp	o, HSG B					
	10,990	92	1/8 acre lots	1/8 acre lots, 65% imp, HSG D						
	3,680	90	1/8 acre lots	s, 65% imp	o, HSG C					
	36,090	88	Weighted Average							
	12,632		35.00% Per	vious Area	3					
	23,459		65.00% Imp	ervious Ar	rea					
Та	l a la aith	Class	Volocity	Consister	Description					
Tc	Length	Slope		Capacity	Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
5.0					Direct Entry,					

AP4

Summary for Subcatchment P4.5: Lots 8-12

Runoff = 2.92 cfs @ 12.07 hrs, Volume= 8,820 cf, Depth= 3.41" Routed to Pond RG8-11 : Rain Gardens 8-11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Area (sf)) CN Description	Description								
31,000	85 1/8 acre lots, 65% imp, HSG B									
10,850	10,850 35.00% Pervious Area									
20,150) 65.00% Impervious Area									
Tc Length (min) (feet										
5.0	Direct Entry,									
Summary for Subcatchment P4.6: Lots 12-13										

Runoff = 0.96 cfs @ 12.09 hrs, Volume= 3,037 cf, Depth= 3.41" Routed to Pond RG12 : Rain Garden 12

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN E	Description							
		10,675	85 1	1/8 acre lots, 65% imp, HSG B							
_		3,736	3	35.00% Pervious Area							
		6,939	6	5.00% Imp	ervious Ar	ea					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
-	6.2	50	0.1200	0.13		Sheet Flow,					
	0.2	30	0.1000	2.21		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps					
_	6.4	80	Total								

Summary for Subcatchment P4.7: Lot 13

Runoff	=	0.92 cfs @	12.07 hrs,	Volume=	2,795 cf,	Depth= 3.51"
Routed	to Pond	I RG13 : Rain	Garden 13			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

Page 22

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description		
	7,505	85	1/8 acre lot	s, 65% imp	o, HSG B
	2,040	90	1/8 acre lot	s, 65% imp	o, HSG C
	9,545		Weighted A		
	3,341	;	35.00% Pei	vious Area	1
	6,204	(65.00% Imp	pervious Are	ea
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P4.8: Upgradient Lots 8-12

Runoff = 0.66 cfs @ 12.26 hrs, Volume= 3,416 cf, Depth= 1.07" Routed to Pond IT-8/12 : Interceptor Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN [Description						
	3,850	61 >	>75% Grass cover, Good, HSG B						
	34,550	55 \	Voods, Go	od, HSG B					
	38,400	56 \	Veighted A	verage					
	38,400		00.00% Pe	ervious Are	a				
Тс	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
12.7	50	0.0200	0.07		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.00"				
3.5	280	0.0700	1.32		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
16.2	330	Total							

Summary for Subcatchment P4.9: Lot 51

Runoff = 0.88 cfs @ 12.07 hrs, Volume= 2,666 cf, Depth= 3.51" Routed to Pond RG51 : Rain Garden 51

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Area (sf)	CN	Description
1,910	90	1/8 acre lots, 65% imp, HSG C
7,195	85	1/8 acre lots, 65% imp, HSG B
9,105	86	Weighted Average
3,187		35.00% Pervious Area
5,918		65.00% Impervious Area

AP4

	clure Engineeering -2g_s/n 03362_© 2022 HydroCA	Postdevelopme Type III 24-hr 10YearMass Rainfall=5.0 Printed 5/1/202 AD Software Solutions LLC Page 2)5" 23		
Tc Length (min) (feet)	Slope Velocity Capacity (ft/ft) (ft/sec) (cfs)				
5.0		Direct Entry,			
	Summary for Pond IT-8/12: Interceptor Trench				
Inflow Area = Inflow = Outflow = Primary = Routed to Link	38,400 sf, 0.00% Imper 0.66 cfs @ 12.26 hrs, Vol 0.60 cfs @ 12.36 hrs, Vol 0.60 cfs @ 12.36 hrs, Vol x AP4-P : AP4 - To Wetland A	blume= 3,413 cf, Atten= 9%, Lag= 6.0 min blume= 3,413 cf			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 680.76' @ 12.36 hrs Surf.Area= 1,050 sf Storage= 250 cf

Plug-Flow detention time= 15.0 min calculated for 3,413 cf (100% of inflow) Center-of-Mass det. time= 14.6 min (912.1 - 897.5)

Volume	Invert	Avail.Storage	Storage Description
#1	680.30'	1,211 cf	3.00'W x 350.00'L x 3.00'H Prismatoid
#2	680.30'	122 cf	3,150 cf Overall - 122 cf Embedded = 3,028 cf x 40.0% Voids 8.0" Round Pipe Storage Inside #1 L= 350.0'
		1,333 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	L=	" Round Culvert 224.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 680.30' / 668.00' S= 0.0549 '/' Cc= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.60 cfs @ 12.36 hrs HW=680.76' TW=0.00' (Dynamic Tailwater) ☐ 1=Culvert (Inlet Controls 0.60 cfs @ 2.32 fps)

Summary for Pond RG-4.1: Rain Garden 4.1

57.90% Impervious, Inflow	Depth = 2.66" for 10YearMass event
12.14 hrs, Volume=	25,291 cf
12.49 hrs, Volume=	24,252 cf, Atten= 63%, Lag= 21.2 min
12.49 hrs, Volume=	8,863 cf
12.49 hrs, Volume=	15,389 cf
To Wetland A (A32-A47)	
0.00 hrs, Volume=	0 cf
· To Wetland A (A32-A47)	
-	12.14 hrs, Volume= 12.49 hrs, Volume= 12.49 hrs, Volume= 12.49 hrs, Volume= To Wetland A (A32-A47) 0.00 hrs, Volume=

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 672.53' @ 12.49 hrs Surf.Area= 13,753 sf Storage= 8,224 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 114.5 min (923.1 - 808.6) AP4

Prepared by Mcclure Engineeering

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Printed 5/1/2023 Page 24

Volume	Invert	Avail.Sto	rage	Storage Description
#1	668.75'	5,18	34 cf	
	074 751	F .	10.5	12,960 cf Overall x 40.0% Voids
#2	671.75'	54	40 cf	12.00'W x 360.00'L x 0.25'H Mulch 1,080 cf Overall x 50.0% Voids
#3	672.00'	11 65	59 cf	12.00'W x 360.00'L x 2.00'H Ponding Z=2.0
	012.00			Total Available Storage
)		3
Device	Routing	Invert	Outl	et Devices
#1	Discarded	668.75'		0 in/hr Exfiltration over Surface area
#2	Primary	670.75'		Round Culvert X 4.00
				0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 670.75' / 670.65' S= 0.0100 '/' Cc= 0.900
			n= 0	0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Device 2	670.75'	2.0"	Vert. Orifice/Grate X 4.00 C= 0.600
			Limi	ted to weir flow at low heads
#4	Device 2	671.25'	4.0"	Vert. Orifice/Grate X 4.00 C= 0.600
			Limi	ted to weir flow at low heads
#5	Device 2	673.25'	8.0"	Horiz. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads
#6	Device 2	673.25'	8.0"	Horiz. Orifice/Grate X 2.00 C= 0.600
				ted to weir flow at low heads
#7	Secondary	673.50'		' long x 10.0' breadth Broad-Crested Rectangular Weir
				d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coe	f. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=0.32 cfs @ 12.49 hrs HW=672.53' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.32 cfs)

Primary OutFlow Max=2.32 cfs @ 12.49 hrs HW=672.53' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 2.32 cfs of 8.09 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.55 cfs @ 6.27 fps) -4=Orifice/Grate (Orifice Controls 1.77 cfs @ 5.08 fps) -5=Orifice/Grate (Controls 0.00 cfs) -6=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=668.75' TW=0.00' (Dynamic Tailwater) -7=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG-4.2: Rain Garden 4.2

65,893 sf, 49.58% Impervious, Inflow Depth = 2.67" for 10YearMass event Inflow Area = Inflow = 4.25 cfs @ 12.13 hrs, Volume= 14,685 cf Outflow 2.85 cfs @ 12.34 hrs, Volume= 14,078 cf, Atten= 33%, Lag= 12.4 min = Discarded = 0.17 cfs @ 12.34 hrs, Volume= 4.563 cf 2.68 cfs @ 12.34 hrs, Volume= 9,515 cf = Primary Routed to Link AP4-P : AP4 - To Wetland A (A32-A47) Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Link AP4-P : AP4 - To Wetland A (A32-A47)

		Postdevelopment
AP4	Type III 24-hr	10YearMass Rainfall=5.05"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC	Page 25

D - - + - | - - - - - - - - - - + +

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 679.16' @ 12.34 hrs Surf.Area= 7,357 sf Storage= 3,568 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 99.0 min (910.1 - 811.1)

Volume	Invert	Avail.Storage	Storage Description
#1	675.75'	2,880 cf	10.00'W x 240.00'L x 3.00'H Soil Media and Gravel
			7,200 cf Overall x 40.0% Voids
#2	678.75'	300 cf	10.00'W x 240.00'L x 0.25'H Mulch
			600 cf Overall x 50.0% Voids
#3	679.00'	6,843 cf	10.00'W x 240.00'L x 2.00'H Ponding Z=2.0
		10,023 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	675.75'	1.020 in/hr Exfiltration over Surface area
#2	Primary	677.75'	6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 677.75' / 677.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	677.75'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	678.25'	5.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 2	680.50'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#6	Secondary	680.50'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=0.17 cfs @ 12.34 hrs HW=679.16' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.17 cfs)

Primary OutFlow Max=2.68 cfs @ 12.34 hrs HW=679.16' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 2.68 cfs of 4.07 cfs potential flow)

1-3=Orifice/Grate (Orifice Controls 0.48 cfs @ 5.54 fps)

-4=Orifice/Grate (Orifice Controls 2.19 cfs @ 4.02 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=675.75' TW=0.00' (Dynamic Tailwater) -6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG12: Rain Garden 12

 Inflow Area =
 10,675 sf, 65.00% Impervious, Inflow Depth =
 3.41" for 10YearMass event

 Inflow =
 0.96 cfs @
 12.09 hrs, Volume=
 3,037 cf

 Outflow =
 0.65 cfs @
 12.18 hrs, Volume=
 3,037 cf, Atten= 32%, Lag= 5.4 min

 Primary =
 0.65 cfs @
 12.18 hrs, Volume=
 3,037 cf

 Routed to Pond RG-4.2 : Rain Garden 4.2
 3,037 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Peak Elev= 689.82' @ 12.18 hrs Surf.Area= 802 sf Storage= 470 cf

Plug-Flow detention time= 16.4 min calculated for 3,037 cf (100% of inflow) Center-of-Mass det. time= 16.3 min (823.8 - 807.5)

Volume	Invert	Avail.Storage	Storage Description
#1	686.25'	106 cf	18.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	686.25'	246 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	689.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	689.50'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		755 of	Total Available Starage

755 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	686.25'	6.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 686.25' / 686.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	690.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	686.25'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	688.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.65 cfs @ 12.18 hrs HW=689.82' TW=678.93' (Dynamic Tailwater)

-1=Culvert (Passes 0.65 cfs of 1.72 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.20 cfs @ 8.98 fps)

-4=Orifice/Grate (Orifice Controls 0.45 cfs @ 5.16 fps)

Summary for Pond RG13: Rain Garden 13

Inflow Are	a =	9,545 sf, 65.00% Impervious,	Inflow Depth = 3.51" for 10YearMass event
Inflow	=	0.92 cfs @ 12.07 hrs, Volume=	2,795 cf
Outflow	=	0.63 cfs @ 12.15 hrs, Volume=	2,795 cf, Atten= 32%, Lag= 4.7 min
Primary	=	0.63 cfs @ 12.15 hrs, Volume=	2,795 cf
Routed	l to Pond	RG-4.2 : Rain Garden 4.2	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 698.23' @ 12.15 hrs Surf.Area= 779 sf Storage= 443 cf

Plug-Flow detention time= 16.5 min calculated for 2,795 cf (100% of inflow) Center-of-Mass det. time= 16.4 min (819.5 - 803.1)

Postdevelopment

Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

r repared by mediate Engineeering	
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 27

Volume	Invert	Avail.Storage	Storage Description		
#1	694.75'	106 cf	18.0" Round Pipe Storage Inside #2		
#2	694.75'	246 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids		
#3	697.75'	30 cf			
#4	609 00'	272 of	60 cf Overall x 50.0% Voids		
#4	698.00'	373 cf			
		755 cf	Total Available Storage		
Device	Routing	Invert Out	let Devices		
#1	Primary	694.75' 6.0'	' Round Culvert		
	•	L= 1	10.0' CPP, square edge headwall, Ke= 0.500		
			t / Outlet Invert= 694.75' / 694.65' S= 0.0100 '/' Cc= 0.900		
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1		'Horiz. Orifice/Grate C= 0.600		
<i>π</i> ∠	Device		ited to weir flow at low heads		
#3	Device 1				
			Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#4	Device 1	697.00' 4.0'	'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		

Primary OutFlow Max=0.63 cfs @ 12.15 hrs HW=698.23' TW=678.75' (Dynamic Tailwater)

1=Culvert (Passes 0.63 cfs of 1.70 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs) -3=Orifice/Grate (Orifice Controls 0.19 cfs @ 8.87 fps)

-4=Orifice/Grate (Orifice Controls 0.43 cfs @ 4.96 fps)

Summary for Pond RG51: Rain Garden 51

Inflow Are	a =	9,105 sf, 65.00% Impervious,	Inflow Depth = 3.51" for 10YearMass event	
Inflow	=	0.88 cfs @ 12.07 hrs, Volume=	2,666 cf	
Outflow	=	0.62 cfs @ 12.15 hrs, Volume=	2,666 cf, Atten= 30%, Lag= 4.5 min	
Primary	=	0.62 cfs @ 12.15 hrs, Volume=	2,666 cf	
Routed to Pond RG-4.2 : Rain Garden 4.2				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 695.18' @ 12.15 hrs Surf.Area= 768 sf Storage= 395 cf

Plug-Flow detention time= 14.8 min calculated for 2,665 cf (100% of inflow) Center-of-Mass det. time= 14.8 min (817.9 - 803.1)

Volume	Invert	Avail.Storage	Storage Description
#1	691.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	691.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	694.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	695.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage

AP4 Prenared by Mcclure Engineeering

Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05"

Printed 5/1/2023

Page 28

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g_s/n 03362 © 2022 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	691.75'	6.0" Round Culvert
	·		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 691.75' / 691.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	695.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	691.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	694.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.62 cfs @ 12.15 hrs HW=695.18' TW=678.71' (Dynamic Tailwater)

1=Culvert (Passes 0.62 cfs of 1.69 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.19 cfs @ 8.81 fps)

-4=Orifice/Grate (Orifice Controls 0.42 cfs @ 4.85 fps)

Summary for Pond RG57-59: Rain Gardens 57,58,59

Inflow Area =	20,938 sf, 65.00% Impervious,	Inflow Depth = 3.41" for 10YearMass event
Inflow =	1.97 cfs @ 12.07 hrs, Volume=	5,957 cf
Outflow =	1.52 cfs @ 12.13 hrs, Volume=	5,958 cf, Atten= 23%, Lag= 3.7 min
Discarded =	0.12 cfs @ 12.13 hrs, Volume=	2,717 cf
Primary =	1.40 cfs @ 12.13 hrs, Volume=	3,241 cf
Routed to Pond	RG-4.2 : Rain Garden 4.2	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 690.60' @ 12.13 hrs Surf.Area= 2,240 sf Storage= 1,117 cf

Plug-Flow detention time= 86.8 min calculated for 5,956 cf (100% of inflow) Center-of-Mass det. time= 86.9 min (893.1 - 806.2)

Volume	Invert	Avail.Storage	Storage Description
#1	687.25'	141 cf	12.0" Round Pipe Storage x 3 Inside #2
			L= 60.0'
#2	687.25'	807 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 3
			2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids
#3	690.25'	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
			180 cf Overall x 50.0% Voids
#4	690.50'	1,120 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3
		2,159 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	687.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	689.25'	6.0" Round Culvert X 3.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 689.25' / 689.00' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	691.00'	6.0" Horiz. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	689.25'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads

AP4

		Postdevelopment
AP4	Type III 24-hr	10YearMass Rainfall=5.05"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC	Page 29

#5 Device 2 689.75' 4.0" Vert. Orifice/Grate X 3.00 C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.12 cfs @ 12.13 hrs HW=690.60' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.12 cfs)

Primary OutFlow Max=1.40 cfs @ 12.13 hrs HW=690.60' TW=678.58' (Dynamic Tailwater) **2=Culvert** (Passes 1.40 cfs of 2.98 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.36 cfs @ 5.42 fps)

-5=Orifice/Grate (Orifice Controls 1.04 cfs @ 3.99 fps)

Summary for Pond RG6-7: Rain Gardens 6,7

Inflow Are	a =	23,445 sf, 65.00% Impervious,	Inflow Depth = 3.41" for 10YearMass event
Inflow	=	2.21 cfs @ 12.07 hrs, Volume=	6,671 cf
Outflow	=	1.61 cfs @ 12.14 hrs, Volume=	6,670 cf, Atten= 27%, Lag= 4.1 min
Primary	=	1.61 cfs @ 12.14 hrs, Volume=	6,670 cf
Routed	to Pond	d RG-4.1 : Rain Garden 4.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 680.01' @ 12.14 hrs Surf.Area= 1,806 sf Storage= 858 cf

Plug-Flow detention time= 10.2 min calculated for 6,668 cf (100% of inflow) Center-of-Mass det. time= 10.2 min (816.4 - 806.2)

Volume	Invert	Avail.Storage	Storage Description
#1	676.75'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
			L= 60.0'
#2	676.75'	682 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel × 2
			1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids
#3	679.75'	75 cf	5.00'W x 60.00'L x 0.25'H Mulch x 2
			150 cf Overall x 50.0% Voids
#4	680.00'	871 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	676.75'	6.0" Round Culvert X 2.00
	·		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 676.75' / 676.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	680.50'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	676.75'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	679.00'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

		Postdevelopment
AP4	Type III 24-hr	10YearMass Rainfall=5.05"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	olutions LLC	Page 30

Primary OutFlow Max=1.61 cfs @ 12.14 hrs HW=680.01' TW=671.75' (Dynamic Tailwater)

1=Culvert (Passes 1.61 cfs of 3.28 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.84 cfs @ 8.53 fps)

4=Orifice/Grate (Orifice Controls 0.77 cfs @ 4.42 fps)

Summary for Pond RG60-64: Rain Gardens 60-64

Inflow Area =	36,090 sf, 65.00% Impervious,	Inflow Depth = 3.72" for 10YearMass event
Inflow =	3.65 cfs @ 12.07 hrs, Volume=	11,177 cf
Outflow =	2.63 cfs @ 12.14 hrs, Volume=	11,177 cf, Atten= 28%, Lag= 4.2 min
Discarded =	0.26 cfs @ 12.14 hrs, Volume=	5,557 cf
Primary =	1.90 cfs @ 12.14 hrs, Volume=	4,496 cf
Routed to Pond	RG-4.1 : Rain Garden 4.1	
Secondary =	0.47 cfs @ 12.14 hrs, Volume=	1,124 cf
Routed to Pond	I RG-4.2 : Rain Garden 4.2	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 687.13' @ 12.14 hrs Surf.Area= 4,671 sf Storage= 2,335 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 87.1 min (883.7 - 796.6)

Volume	Invert	Avail.Storage	Storage Description
#1	683.75'	236 cf	12.0" Round Pipe Storage x 5 Inside #2
			L= 60.0'
#2	683.75'	1,706 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 5
			4,500 cf Overall - 236 cf Embedded = 4,264 cf x 40.0% Voids
#3	686.75'	188 cf	5.00'W x 60.00'L x 0.25'H Mulch x 5
			375 cf Overall x 50.0% Voids
#4	687.00'	2,177 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 5
		4,306 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1 #2	Discarded Primary	683.75' 685.75'	2.410 in/hr Exfiltration over Surface area 6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 685.75' / 684.65' S= 0.1100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Secondary	685.75'	6.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 685.75' / 684.65' S= 0.1100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#4	Device 2	687.50'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 3	687.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#6	Device 2	685.75'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
#7	Device 3	685.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Postdevelopment AP4 Type III 24-hr 10YearMass Rainfall=5.05" Prepared by Mcclure Engineeering Printed 5/1/2023 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 31 #8 Device 2 686.25' 4.0" Vert. Orifice/Grate X 4.00 C= 0.600

Limited to weir flow at low heads

#9 Device 3 686.25' **4.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.26 cfs @ 12.14 hrs HW=687.13' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.26 cfs)

Primary OutFlow Max=1.90 cfs @ 12.14 hrs HW=687.13' TW=671.75' (Dynamic Tailwater)

-2=Culvert (Passes 1.90 cfs of 4.02 cfs potential flow)

-4=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Orifice Controls 0.48 cfs @ 5.48 fps)

-8=Orifice/Grate (Orifice Controls 1.42 cfs @ 4.07 fps)

Secondary OutFlow Max=0.47 cfs @ 12.14 hrs HW=687.13' TW=678.66' (Dynamic Tailwater)

3=Culvert (Passes 0.47 cfs of 1.00 cfs potential flow)

5=Orifice/Grate (Controls 0.00 cfs)

-7=Orifice/Grate (Orifice Controls 0.12 cfs @ 5.48 fps)

-9=Orifice/Grate (Orifice Controls 0.35 cfs @ 4.07 fps)

Summary for Pond RG65-66: Rain Gardens 65-66

Inflow Area =	11,035 sf, 65.00% Impervious,	Inflow Depth = 4.03" for 10YearMass event
Inflow =	1.19 cfs @ 12.07 hrs, Volume=	3,707 cf
Outflow =	0.93 cfs @ 12.13 hrs, Volume=	3,486 cf, Atten= 22%, Lag= 3.6 min
Discarded =	0.04 cfs @ 12.13 hrs, Volume=	1,228 cf
Primary =	0.88 cfs @ 12.13 hrs, Volume=	2,257 cf
Routed to Pond	RG-4.1 : Rain Garden 4.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 678.52' @ 12.13 hrs Surf.Area= 1,811 sf Storage= 865 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 102.3 min (887.9 - 785.6)

Volume	Invert	Avail.Stor	age	Storage Description	
#1	675.25'	9	94 cf	12.0" Round Pipe Storage x 2 Inside #2 L= 60.0'	
#2	675.25'	68	32 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids	
#3	678.25'	75 cf		5.00'W x 60.00'L x 0.25'H Mulch x 2 150 cf Overall x 50.0% Voids	
#4	678.50'	871 cf		5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2	
		1,72	22 cf Total Available Storage		
Device	Routing	Invert	Outle	et Devices	
#1	Discarded	675.25'	1.02	0 in/hr Exfiltration over Surface area	
#2	Primary	677.25'		Round Culvert X 2.00	
				0.0' CPP, square edge headwall, Ke= 0.500	
				/ Outlet Invert= 677.25' / 675.15' S= 0.2100 '/' Cc= 0.900	
#3	Device 2	679.00'		0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf Horiz, Orifice/Grate X 2.00 C= 0.600	
#3		079.00	0.0		

		Postdevelopment
AP4	Type III 24-hr	10YearMass Rainfall=5.05"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC	Page 32

			Limited to weir flow at low heads
#4	Device 2	677.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 2	677.75'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.13 hrs HW=678.52' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.88 cfs @ 12.13 hrs HW=678.52' TW=671.62' (Dynamic Tailwater) 2=Culvert (Passes 0.88 cfs of 1.91 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -4=Orifice/Grate (Orifice Controls 0.23 cfs @ 5.25 fps) -5=Orifice/Grate (Orifice Controls 0.65 cfs @ 3.75 fps)

Summary for Pond RG8-11: Rain Gardens 8-11

Inflow Are	a =	31,000 sf, 65.00% Impervious,	Inflow Depth = 3.41" for 10YearMass event
Inflow	=	2.92 cfs @ 12.07 hrs, Volume=	8,820 cf
Outflow	=	1.96 cfs @ 12.15 hrs, Volume=	8,819 cf, Atten= 33%, Lag= 4.8 min
Primary	=	1.96 cfs @ 12.15 hrs, Volume=	8,819 cf
Routed	l to Pond	d RG-4.1 : Rain Garden 4.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 678.71' @ 12.15 hrs Surf.Area= 1,200 sf Storage= 1,677 cf

Plug-Flow detention time= 21.1 min calculated for 8,819 cf (100% of inflow) Center-of-Mass det. time= 21.0 min (827.2 - 806.2)

Volume	Invert	Avail.Stor	Storage Description			
#1	675.75'	42	24 cf 18.0" Round Pipe Storage x 4 Inside #2 L= 60.0'			
#2	675.75'	1,27	70 cf 5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 4 3,600 cf Overall - 424 cf Embedded = 3,176 cf x 40.0% Voids			
#3	678.75'	15	50 cf 5.00'W x 60.00'L x 0.25'H Mulch x 4 300 cf Overall x 50.0% Voids			
#4	679.00'	1,74	41 cf 5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 4			
		3,58	36 cf Total Available Storage			
Device	Routing	Invert	Outlet Devices			
#1	Primary	675.75'	6.0" Round Culvert X 4.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 675.75' / 675.65' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf			
#2	Device 1	679.50'	 6.0" Horiz. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads 2.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads 			
#3	Device 1	675.75'				
#4	Device 1	678.00'	4.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads			

Primary OutFlow Max=1.95 cfs @ 12.15 hrs HW=678.71' TW=671.86' (Dynamic Tailwater) 1=Culvert (Passes 1.95 cfs of 6.23 cfs potential flow) 2=Orifice/Grate (Controls 0.00 cfs) 3=Orifice/Grate (Orifice Controls 0.71 cfs @ 8.17 fps) 4=Orifice/Grate (Orifice Controls 1.24 cfs @ 3.56 fps)

Summary for Link AP4-P: AP4 - To Wetland A (A32-A47)

 Inflow Area =
 307,487 sf, 32.10% Impervious, Inflow Depth =
 1.85" for 10YearMass event

 Inflow =
 8.81 cfs @
 12.17 hrs, Volume=
 47,464 cf

 Primary =
 8.81 cfs @
 12.17 hrs, Volume=
 47,464 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P4.1: To Wetland A (A32-A47)

Runoff = 8.22 cfs @ 12.10 hrs, Volume= 26,273 cf, Depth= 3.54" Routed to Link AP4-P : AP4 - To Wetland A (A32-A47)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Α	rea (sf)	CN E	Description		
	630	55 V	Voods, Go	od, HSG B	
	73,383	77 V	Voods, Go	od, HSG D	
	4,661	61 >	75% Gras	s cover, Go	bod, HSG B
	10,490	80 >	75% Gras	s cover, Go	bod, HSG D
	89,164	76 V	Veighted A	verage	
	89,164	1	00.00% Pe	ervious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.7	50	0.1500	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.2	135	0.1500	1.94		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
6.9	185	Total			

Summary for Subcatchment P4.10: To RG4.1

Runoff = 1.27 cfs @ 12.10 hrs, Volume= Routed to Pond RG-4.1 : Rain Garden 4.1 4,095 cf, Depth= 3.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

	А	rea (sf)	CN I	Description		
		12,335	80 :	>75% Gras	s cover, Go	ood, HSG D
		125	61 3	>75% Gras	s cover, Go	ood, HSG B
		12,460	80	Neighted A	verage	
		12,460		100.00% Pe	ervious Are	а
		Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.7	50	0.1500	0.15		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	1.3	150	0.1500	1.94		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	7.0	200	Total			

Summary for Subcatchment P4.11: To RG4.2

Runoff = 0.83 cfs @ 12.11 hrs, Volume= 2,770 cf, Depth= 2.13" Routed to Pond RG-4.2 : Rain Garden 4.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

_	A	rea (sf)	CN	Description						
		15,520		61 >75% Grass cover, Good, HSG B						
_		110	80	>75% Gras	s cover, Go	ood, HSG D				
		15,630	61	Weighted A	verage					
		15,630		100.00% Pe	ervious Are	a				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.7	50	0.1500	0.15		Sheet Flow,				
						Woods: Light underbrush n= 0.400 P2= 3.00"				
	1.3	150	0.1500	1.94		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	7.0	200	Total							

Summary for Subcatchment P4.12: Lots 65-67

Runoff	=	1.49 cfs @ 1	12.07 hrs,	Volume=	4,721 cf,	Depth= 5.13"
Routed	l to Pone	d RG65-66 ː Ra	ain Garden	s 65-66		-

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN	Description				
	5,185	90	1/8 acre lots	s, 65% imp	, HSG C		
	5,850	92	1/8 acre lots	s, 65% imp	, HSG D		
	11,035	91	Neighted A	verage			
	3,862	;	35.00% Pervious Area				
	7,173		65.00% Imp	pervious Are	ea		
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
5.0					Direct Entry,		

Summary for Subcatchment P4.13: Lots 6-8

Runoff = 2.86 cfs @ 12.07 hrs, Volume= 8,738 cf, Depth= 4.47" Routed to Pond RG6-7 : Rain Gardens 6,7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Postdevelopment Type III 24-hr 25YearMass Rainfall=6.18" Printed 5/1/2023

Page 36

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Area (sf)	CN	Description				
23,175	85	1/8 acre lots	s, 65% imp	, HSG B		
270	90	1/8 acre lots	s, 65% imp	, HSG C		
23,445	85	Weighted A	verage			
8,206		35.00% Pervious Area				
15,239		65.00% Imp	ervious Ar	ea		
Tc Length (min) (feet)	Slope (ft/ft		Capacity (cfs)	Description		
5.0				Direct Entry,		

Summary for Subcatchment P4.2: Lots 57-60

Runoff = 2.56 cfs @ 12.07 hrs, Volume= 7,804 cf, Depth= 4.47" Routed to Pond RG57-59 : Rain Gardens 57,58,59

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Area (sf)	CN	CN Description						
20,938	85	85 1/8 acre lots, 65% imp, HSG B						
7,328		35.00% Pervious Area						
13,610		65.00% Impervious Area						
Tc Length (min) (feet) 5.0	Slop (ft/f	,	Capacity (cfs)	Description Direct Entry,				

Summary for Subcatchment P4.4: Lots 60-65

Runoff	=	4.66 cfs @	12.07 hrs,	Volume=	14,434 cf,	Depth= 4.80"
Routed	to Pond	RG60-64 : F	≀ain Garden	s 60-64		

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN	Description				
	21,420	85	1/8 acre lots	s, 65% imp	, HSG B		
	10,990	92	1/8 acre lots	s, 65% imp	, HSG D		
	3,680	90	1/8 acre lots	s, 65% imp	, HSG C		
	36,090	88	88 Weighted Average				
	12,632		35.00% Pervious Area				
	23,459		65.00% Impervious Area				
т.	المربع مراجع	<u>Olana</u>	V al a situ	0	Decemination		
Tc	Length	Slope		Capacity	Description		
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
5.0					Direct Entry,		

AP4

Summary for Subcatchment P4.5: Lots 8-12

Runoff = 3.79 cfs @ 12.07 hrs, Volume= 11,554 cf, Depth= 4.47" Routed to Pond RG8-11 : Rain Gardens 8-11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Area (sf)	CN	CN Description						
31,000	85	85 1/8 acre lots, 65% imp, HSG B						
10,850		35.00% Pervious Area						
20,150		65.00% Impervious Area						
Tc Length (min) (feet)	Slop (ft/f	,	Capacity (cfs)	Description				
5.0				Direct Entry,				
Summary for Subastabrant B4 6: Late 12 12								

Summary for Subcatchment P4.6: Lots 12-13

Runoff = 1.24 cfs @ 12.09 hrs, Volume= 3,979 cf, Depth= 4.47" Routed to Pond RG12 : Rain Garden 12

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

_	A	rea (sf)	CN E	escription					
		10,675	85 1	1/8 acre lots, 65% imp, HSG B					
		3,736	3	5.00% Per	vious Area				
		6,939	6	65.00% Impervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
-	6.2	50	0.1200	0.13		Sheet Flow,			
	0.2	30	0.1000	2.21		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
_	6.4	80	Total						

Summary for Subcatchment P4.7: Lot 13

Runoff	=	1.19 cfs @	12.07 hrs,	Volume=	3,644 cf,	Depth= 4.58"
Routed	l to Pond	d RG13 : Rain	Garden 13			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Postdevelopment Type III 24-hr 25YearMass Rainfall=6.18" Printed 5/1/2023

Page 38

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description			
	7,505	85	1/8 acre lots	s, 65% imp	, HSG B	
	2,040	90	1/8 acre lots	s, 65% imp	, HSG C	
	9,545	86	Neighted A	verage		
	3,341		35.00% Per	vious Area		
	6,204		65.00% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description	
5.0					Direct Entry,	

Summary for Subcatchment P4.8: Upgradient Lots 8-12

Runoff = 1.17 cfs @ 12.25 hrs, Volume= 5,452 cf, Depth= 1.70" Routed to Pond IT-8/12 : Interceptor Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN [Description					
	3,850	61 >	75% Gras	s cover, Go	bod, HSG B			
	34,550	55 \	Voods, Go	od, HSG B				
	38,400	56 \	56 Weighted Average					
	38,400		a					
Тс	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
12.7	50	0.0200	0.07		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
3.5	280	0.0700	1.32		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
16.2	330	Total						

Summary for Subcatchment P4.9: Lot 51

Runoff = 1.13 cfs @ 12.07 hrs, Volume= 3,476 cf, Depth= 4.58" Routed to Pond RG51 : Rain Garden 51

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Area (sf)	CN	Description
1,910	90	1/8 acre lots, 65% imp, HSG C
7,195	85	1/8 acre lots, 65% imp, HSG B
9,105	86	Weighted Average
3,187		35.00% Pervious Area
5,918		65.00% Impervious Area

AP4

AP4 Type III 24-hr 25YearMass Rainfall=6.18 Prepared by Mcclure Engineeering Printed 5/1/202 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 3	8″ 23		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)			
5.0 Direct Entry,			
Summary for Pond IT-8/12: Interceptor Trench			
Inflow Area = 38,400 sf, 0.00% Impervious, Inflow Depth = 1.70" for 25YearMass event Inflow = 1.17 cfs @ 12.25 hrs, Volume= 5,452 cf Outflow = 1.06 cfs @ 12.33 hrs, Volume= 5,450 cf, Atten= 9%, Lag= 5.2 min Primary = 1.06 cfs @ 12.33 hrs, Volume= 5,450 cf Routed to Link AP4-P : AP4 - To Wetland A (A32-A47) Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 681.03' @ 12.33 hrs Surf.Area= 1,050 sf Storage= 380 cf Plug-Flow detention time= 12.1 min calculated for 5,448 cf (100% of inflow) Center-of-Mass det. time= 11.9 min (893.3 - 881.4)			
Volume Invert Avail.Storage Storage Description			
#1 680.30' 1,211 cf 3.00'W x 350.00'L x 3.00'H Prismatoid			
#2 680.30' 122 cf 3,150 cf Overall - 122 cf Embedded = 3,028 cf x 40.0% Voids #2 680.30' 122 cf 8.0" Round Pipe Storage Inside #1 L= 350.0' L= 350.0'			
1,333 cf Total Available Storage			
Device Routing Invert Outlet Devices			
#1 Primary 680.30' 8.0" Round Culvert L= 224.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 680.30' / 668.00' S= 0.0549 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf			
Primary OutFlow Max=1.06 cfs @ 12.33 hrs HW=681.03' TW=0.00' (Dynamic Tailwater) ↑ 1=Culvert (Inlet Controls 1.06 cfs @ 3.03 fps)			

Summary for Pond RG-4.1: Rain Garden 4.1

Inflow Area =	114,030 sf, 57.90% Impervious, Inflo	<i>w</i> Depth = 3.60" for 25YearMass event
Inflow =	8.56 cfs @ 12.13 hrs, Volume=	34,165 cf
Outflow =	3.25 cfs @ 12.51 hrs, Volume=	32,871 cf, Atten= 62%, Lag= 22.7 min
Discarded =	0.35 cfs @ 12.51 hrs, Volume=	9,758 cf
Primary =	2.90 cfs @ 12.51 hrs, Volume=	23,113 cf
Routed to Link	AP4-P : AP4 - To Wetland A (A32-A47)	
	0.00 cfs @ 0.00 hrs, Volume=	0 cf
Routed to Link	AP4-P : AP4 - To Wetland A (A32-A47)	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 673.21' @ 12.51 hrs Surf.Area= 14,780 sf Storage= 12,034 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 94.1 min (896.4 - 802.3) AP4

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Printed 5/1/2023 Page 40

Volume	Invert	Avail.Stor	rage	Storage Description
#1	668.75'	5,184 cf		
40		-	10 -5	12,960 cf Overall x 40.0% Voids
#2	671.75'	54	10 cf	12.00'W x 360.00'L x 0.25'H Mulch 1,080 cf Overall x 50.0% Voids
#3	672.00'	11.65	59 cf	12.00'W x 360.00'L x 2.00'H Ponding Z=2.0
		· · · · · ·		Total Available Storage
				u u u u u u u u u u u u u u u u u u u
Device	Routing	Invert	Outle	et Devices
#1	Discarded	668.75'	1.02	0 in/hr Exfiltration over Surface area
#2	Primary	670.75'	8.0"	Round Culvert X 4.00
			L= 1	0.0' CPP, square edge headwall, Ke= 0.500
			Inlet	/ Outlet Invert= 670.75' / 670.65' S= 0.0100 '/' Cc= 0.900
			n= 0	.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Device 2	670.75'	2.0"	Vert. Orifice/Grate X 4.00 C= 0.600
			Limi	ted to weir flow at low heads
#4	Device 2	671.25'	4.0"	Vert. Orifice/Grate X 4.00 C= 0.600
			Limi	ted to weir flow at low heads
#5	Device 2	673.25'	8.0"	Horiz. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads
#6	Device 2	673.25'	8.0"	Horiz. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads
#7	Secondary	673.50'	10.0	' long x 10.0' breadth Broad-Crested Rectangular Weir
			Hea	d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coe	f. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=0.35 cfs @ 12.51 hrs HW=673.21' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.35 cfs)

Primary OutFlow Max=2.90 cfs @ 12.51 hrs HW=673.21' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 2.90 cfs of 9.80 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.65 cfs @ 7.42 fps) -4=Orifice/Grate (Orifice Controls 2.25 cfs @ 6.44 fps) -5=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=668.75' TW=0.00' (Dynamic Tailwater) -7=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG-4.2: Rain Garden 4.2

65,893 sf, 49.58% Impervious, Inflow Depth = 3.70" for 25YearMass event Inflow Area = Inflow = 5.31 cfs @ 12.15 hrs, Volume= 20,292 cf Outflow 3.53 cfs @ 12.40 hrs, Volume= 19,559 cf, Atten= 34%, Lag= 15.1 min = Discarded = 0.18 cfs @ 12.40 hrs, Volume= 4.879 cf 3.34 cfs @ 12.40 hrs, Volume= 14,680 cf = Primary Routed to Link AP4-P : AP4 - To Wetland A (A32-A47) Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Link AP4-P : AP4 - To Wetland A (A32-A47)

	Postdevelopment
AP4	Type III 24-hr 25YearMass Rainfall=6.18"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software	Solutions LLC Page 41

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 679.58' @ 12.40 hrs Surf.Area= 7,790 sf Storage= 4,756 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 73.7 min (879.0 - 805.3)

Volume	Invert	Avail.Storage	Storage Description
#1	675.75'	2,880 cf	10.00'W x 240.00'L x 3.00'H Soil Media and Gravel
			7,200 cf Overall x 40.0% Voids
#2	678.75'	300 cf	10.00'W x 240.00'L x 0.25'H Mulch
			600 cf Overall x 50.0% Voids
#3	679.00'	6,843 cf	10.00'W x 240.00'L x 2.00'H Ponding Z=2.0
		10,023 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	675.75'	1.020 in/hr Exfiltration over Surface area
#2	Primary	677.75'	6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 677.75' / 677.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	677.75'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	678.25'	
			Limited to weir flow at low heads
#5	Device 2	680.50'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#6	Secondary	680.50'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=0.18 cfs @ 12.40 hrs HW=679.58' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.18 cfs)

Primary OutFlow Max=3.34 cfs @ 12.40 hrs HW=679.58' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 3.34 cfs of 4.76 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.56 cfs @ 6.37 fps)

-4=Orifice/Grate (Orifice Controls 2.79 cfs @ 5.11 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=675.75' TW=0.00' (Dynamic Tailwater) -6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG12: Rain Garden 12

Inflow Are	a =	10,675 sf, 65.00% Impervious,	Inflow Depth = 4.47" for 25YearMass event		
Inflow	=	1.24 cfs @ 12.09 hrs, Volume=	3,979 cf		
Outflow	=	0.93 cfs @ 12.16 hrs, Volume=	3,978 cf, Atten= 25%, Lag= 4.4 min		
Primary	=	0.93 cfs @ 12.16 hrs, Volume=	3,978 cf		
Routed to Pond RG-4.2 : Rain Garden 4.2					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Peak Elev= 690.12' @ 12.16 hrs Surf.Area= 885 sf Storage= 581 cf

Plug-Flow detention time= 16.1 min calculated for 3,977 cf (100% of inflow) Center-of-Mass det. time= 16.1 min (816.0 - 799.9)

Volume	Invert	Avail.Storage	Storage Description
#1	686.25'	106 cf	18.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	686.25'	246 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	689.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	689.50'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		755 of	Total Available Starage

755 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	686.25'	6.0" Round Culvert
	-		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 686.25' / 686.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	690.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	686.25'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	688.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.93 cfs @ 12.16 hrs HW=690.12' TW=679.27' (Dynamic Tailwater)

-1=Culvert (Passes 0.93 cfs of 1.80 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.22 cfs @ 1.14 fps)

-3=Orifice/Grate (Orifice Controls 0.20 cfs @ 9.37 fps)

-4=Orifice/Grate (Orifice Controls 0.51 cfs @ 5.81 fps)

Summary for Pond RG13: Rain Garden 13

Inflow Are	a =	9,545 sf, 65.00% Impervious,	Inflow Depth = 4.58" for 25YearMass event		
Inflow	=	1.19 cfs @ 12.07 hrs, Volume=	3,644 cf		
Outflow	=	0.79 cfs @ 12.15 hrs, Volume=	3,643 cf, Atten= 33%, Lag= 4.8 min		
Primary	=	0.79 cfs @ 12.15 hrs, Volume=	3,643 cf		
Routed to Pond RG-4.2 : Rain Garden 4.2					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 698.57' @ 12.15 hrs Surf.Area= 871 sf Storage= 561 cf

Plug-Flow detention time= 16.2 min calculated for 3,642 cf (100% of inflow) Center-of-Mass det. time= 16.2 min (811.9 - 795.7)

Postdevelopment

Type III 24-hr 25YearMass Rainfall=6.18" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software So	blutions LLC Page 43

Volume	Invert	Avail.Storage	Storage Description
#1	694.75'	106 cf	· · · · · · · · · · · · · · · · · · ·
#2	694.75'	246 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 106 cf Embedded = 614 cf _x 40.0% Voids
#3	697.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
		070 (60 cf Overall x 50.0% Voids
#4	698.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		755 cf	Total Available Storage
			6
Device	Routing	Invert Out	let Devices
#1	Primary	694.75' 6.0'	' Round Culvert
		L= '	10.0' CPP, square edge headwall, Ke= 0.500
			t / Outlet Invert= 694.75' / 694.65' S= 0.0100 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		'Horiz. Orifice/Grate C= 0.600
#2	Device		
			ited to weir flow at low heads
#3	Device 1	694.75' 2.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	697.00' 4.0'	'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.79 cfs @ 12.15 hrs HW=698.57' TW=679.24' (Dynamic Tailwater)

1=Culvert (Passes 0.79 cfs of 1.79 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.09 cfs @ 0.86 fps) **3=Orifice/Grate** (Orifice Controls 0.20 cfs @ 9.31 fps)

-4=Orifice/Grate (Orifice Controls 0.50 cfs @ 5.70 fps)

Summary for Pond RG51: Rain Garden 51

Inflow Are	a =	9,105 sf, 65.00% Impervious,	Inflow Depth = 4.58" for 25YearMass event	
Inflow	=	1.13 cfs @ 12.07 hrs, Volume=	3,476 cf	
Outflow	=	0.71 cfs @ 12.16 hrs, Volume=	3,475 cf, Atten= 37%, Lag= 5.3 min	
Primary	=	0.71 cfs @ 12.16 hrs, Volume=	3,475 cf	
Routed to Pond RG-4.2 : Rain Garden 4.2				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 695.53' @ 12.16 hrs Surf.Area= 859 sf Storage= 509 cf

Plug-Flow detention time= 14.6 min calculated for 3,474 cf (100% of inflow) Center-of-Mass det. time= 14.6 min (810.3 - 795.7)

Volume	Invert	Avail.Storage	Storage Description
#1	691.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	691.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	694.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	695.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage

AP4

Postdevelopment Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023

Page 44

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g_s/n 03362_© 2022 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	691.75'	6.0" Round Culvert
	-		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 691.75' / 691.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	695.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	691.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	694.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.71 cfs @ 12.16 hrs HW=695.53' TW=679.26' (Dynamic Tailwater)

1=Culvert (Passes 0.71 cfs of 1.78 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.02 cfs @ 0.54 fps)

-3=Orifice/Grate (Orifice Controls 0.20 cfs @ 9.25 fps)

-4=Orifice/Grate (Orifice Controls 0.49 cfs @ 5.62 fps)

Summary for Pond RG57-59: Rain Gardens 57,58,59

Inflow Area =	20,938 sf, 65.00% Impervious,	Inflow Depth = 4.47" for 25YearMass event	
Inflow =	2.56 cfs @ 12.07 hrs, Volume=	7,804 cf	
Outflow =	1.74 cfs @ 12.15 hrs, Volume=	7,804 cf, Atten= 32%, Lag= 4.7 min	
Discarded =	0.14 cfs @ 12.15 hrs, Volume=	3,026 cf	
Primary =	1.60 cfs @ 12.15 hrs, Volume=	4,778 cf	
Routed to Pond RG-4.2 : Rain Garden 4.2			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 690.84' @ 12.15 hrs Surf.Area= 2,430 sf Storage= 1,333 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 77.0 min (875.6 - 798.6)

Volume	Invert	Avail.Storage	Storage Description
#1	687.25'	141 cf	12.0" Round Pipe Storage x 3 Inside #2
#0	697 051	907 of	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 3
#2	687.25'	007 CI	2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids
#3	690.25'	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
			180 cf Overall x 50.0% Voids
#4	690.50'	1,120 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3
		2,159 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	687.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	689.25'	6.0" Round Culvert X 3.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 689.25' / 689.00' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	691.00'	6.0" Horiz. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	689.25'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads

AP4

Postdevelopment AP4 Type III 24-hr 25YearMass Rainfall=6.18" Prepared by Mcclure Engineeering Printed 5/1/2023 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 45

#5 Device 2 689.75' 4.0" Vert. Orifice/Grate X 3.00 C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.14 cfs @ 12.15 hrs HW=690.84' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.14 cfs)

Primary OutFlow Max=1.60 cfs @ 12.15 hrs HW=690.84' TW=679.23' (Dynamic Tailwater)

-2=Culvert (Passes 1.60 cfs of 3.29 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.39 cfs @ 5.92 fps)

-5=Orifice/Grate (Orifice Controls 1.21 cfs @ 4.64 fps)

Summary for Pond RG6-7: Rain Gardens 6,7

Inflow Area = 23.445 sf, 65.00% Impervious, Inflow Depth = 4.47" for 25YearMass event 2.86 cfs @ 12.07 hrs, Volume= Inflow = 8,738 cf 1.83 cfs @ 12.16 hrs, Volume= Outflow = 8,738 cf, Atten= 36%, Lag= 5.2 min Primary = 1.83 cfs @ 12.16 hrs, Volume= 8,738 cf Routed to Pond RG-4.1 : Rain Garden 4.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 680.41' @ 12.16 hrs Surf.Area= 2,020 sf Storage= 1,145 cf

Plug-Flow detention time= 9.9 min calculated for 8,738 cf (100% of inflow) Center-of-Mass det. time= 9.9 min (808.5 - 798.6)

Volume	Invert	Avail.Storage	Storage Description
#1	676.75'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
			L= 60.0'
#2	676.75'	682 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel × 2
			1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids
#3	679.75'	75 cf	5.00'W x 60.00'L x 0.25'H Mulch x 2
			150 cf Overall x 50.0% Voids
#4	680.00'	871 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	676.75'	6.0" Round Culvert X 2.00
	-		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 676.75' / 676.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	680.50'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	676.75'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	679.00'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

	Postdevelopment
AP4	Type III 24-hr 25YearMass Rainfall=6.18"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC Page 46

Primary OutFlow Max=1.83 cfs @ 12.16 hrs HW=680.41' TW=672.46' (Dynamic Tailwater)

1=Culvert (Passes 1.83 cfs of 3.49 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.89 cfs @ 9.06 fps)

4=Orifice/Grate (Orifice Controls 0.94 cfs @ 5.38 fps)

Summary for Pond RG60-64: Rain Gardens 60-64

Inflow Area =	36,090 sf, 65.00% Impervious,	Inflow Depth = 4.80" for 25YearMass event
Inflow =	4.66 cfs @ 12.07 hrs, Volume=	14,434 cf
Outflow =	2.97 cfs @ 12.16 hrs, Volume=	14,435 cf, Atten= 36%, Lag= 5.2 min
Discarded =	0.28 cfs @ 12.16 hrs, Volume=	6,206 cf
Primary =	2.16 cfs @ 12.16 hrs, Volume=	6,584 cf
Routed to Pond	I RG-4.1 : Rain Garden 4.1	
Secondary =	0.54 cfs @ 12.16 hrs, Volume=	1,646 cf
Routed to Pond	I RG-4.2 : Rain Garden 4.2	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 687.36' @ 12.16 hrs Surf.Area= 4,985 sf Storage= 2,764 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 79.0 min (868.6 - 789.6)

Volume	Invert	Avail.Storage	Storage Description
#1	683.75'	236 cf	12.0" Round Pipe Storage x 5 Inside #2
			L= 60.0'
#2	683.75'	1,706 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 5
			4,500 cf Overall - 236 cf Embedded = 4,264 cf x 40.0% Voids
#3	686.75'	188 cf	5.00'W x 60.00'L x 0.25'H Mulch x 5
			375 cf Overall x 50.0% Voids
#4	687.00'	2,177 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 5
		4,306 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1 #2	Discarded Primary	683.75' 685.75'	2.410 in/hr Exfiltration over Surface area 6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 685.75' / 684.65' S= 0.1100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Secondary	685.75'	6.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 685.75' / 684.65' S= 0.1100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#4	Device 2	687.50'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 3	687.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#6	Device 2	685.75'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
#7	Device 3	685.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

 AP4
 Type III 24-hr
 25YearMass Rainfall=6.18"

 Prepared by Mcclure Engineeering
 Printed
 5/1/2023

 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC
 Page 47

#8 Device 2 686.25' 4.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
#9 Device 3 686.25' 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.28 cfs @ 12.16 hrs HW=687.36' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.28 cfs)

Primary OutFlow Max=2.16 cfs @ 12.16 hrs HW=687.36' TW=672.45' (Dynamic Tailwater)

-**2=Culvert** (Passes 2.16 cfs of 4.42 cfs potential flow)

4=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Orifice Controls 0.52 cfs @ 5.96 fps)

Secondary OutFlow Max=0.54 cfs @ 12.16 hrs HW=687.36' TW=679.25' (Dynamic Tailwater)

3=Culvert (Passes 0.54 cfs of 1.10 cfs potential flow)

5=Orifice/Grate (Controls 0.00 cfs)

-7=Orifice/Grate (Orifice Controls 0.13 cfs @ 5.96 fps)

-9=Orifice/Grate (Orifice Controls 0.41 cfs @ 4.69 fps)

Summary for Pond RG65-66: Rain Gardens 65-66

Inflow Area =	11,035 sf, 65.00% Impervious,	Inflow Depth = 5.13" for 25YearMass event			
Inflow =	1.49 cfs @ 12.07 hrs, Volume=	4,721 cf			
Outflow =	1.03 cfs @ 12.15 hrs, Volume=	4,485 cf, Atten= 31%, Lag= 4.5 min			
Discarded =	0.04 cfs @ 12.15 hrs, Volume=	1,289 cf			
Primary =	0.98 cfs @ 12.15 hrs, Volume=	3,196 cf			
Routed to Pond RG-4.1 : Rain Garden 4.1					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 678.69' @ 12.15 hrs Surf.Area= 1,900 sf Storage= 975 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 83.6 min (862.8 - 779.2)

Volume	Invert	Avail.Stora	age	Storage Description
#1	675.25'	94	4 cf	12.0" Round Pipe Storage x 2 Inside #2 L= 60.0'
#2	675.25'	682	2 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids
#3	678.25'	7	5 cf	5.00'W x 60.00'L x 0.25'H Mulch x 2 150 cf Overall x 50.0% Voids
#4	678.50'	87	1 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf Total Available Storage		
Device	Routing	Invert	Outle	et Devices
#1	Discarded	675.25'	675.25' 1.020 in/hr Exfiltration over Surface area	
#2	Primary	677.25'	5' 6.0" Round Culvert X 2.00	
				0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 677.25' / 675.15' S= 0.2100 '/' Cc= 0.900
				.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	679.00'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600

	Postdevelopment
AP4	Type III 24-hr 25YearMass Rainfall=6.18"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC Page 48

v heads
X 2.00 C= 0.600
v heads
X 2.00 C= 0.600
v heads

Discarded OutFlow Max=0.04 cfs @ 12.15 hrs HW=678.69' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.98 cfs @ 12.15 hrs HW=678.69' TW=672.40' (Dynamic Tailwater) 2=Culvert (Passes 0.98 cfs of 2.06 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -4=Orifice/Grate (Orifice Controls 0.24 cfs @ 5.61 fps) -5=Orifice/Grate (Orifice Controls 0.74 cfs @ 4.23 fps)

Summary for Pond RG8-11: Rain Gardens 8-11

Inflow Are	a =	31,000 sf, 65.00% Impervious,	Inflow Depth = 4.47" for 25YearMass event		
Inflow	=	3.79 cfs @ 12.07 hrs, Volume=	11,554 cf		
Outflow	=	2.44 cfs @ 12.16 hrs, Volume=	11,553 cf, Atten= 36%, Lag= 5.1 min		
Primary	=	2.44 cfs @ 12.16 hrs, Volume=	11,553 cf		
Routed to Pond RG-4.1 : Rain Garden 4.1					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 679.16' @ 12.16 hrs Surf.Area= 3,770 sf Storage= 2,052 cf

Plug-Flow detention time= 19.9 min calculated for 11,549 cf (100% of inflow) Center-of-Mass det. time= 19.8 min (818.5 - 798.6)

Volume	Invert	Avail.Stor	rage	Storage Description
#1	675.75'	424 cf		18.0" Round Pipe Storage x 4 Inside #2 L= 60.0'
#2	675.75'	1,270 cf		5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 4 3,600 cf Overall - 424 cf Embedded = 3,176 cf x 40.0% Voids
#3	678.75'	15	50 cf	5.00'W x 60.00'L x 0.25'H Mulch x 4 300 cf Overall x 50.0% Voids
#4	679.00'	1,74	11 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 4
		3,58	36 cf	Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Primary	675.75'	6.0"	Round Culvert X 4.00
	-		L= 1	0.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 675.75' / 675.65' S= 0.0100 '/' Cc= 0.900	
			n= 0	.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	679.50'	6.0"	Horiz. Orifice/Grate X 4.00 C= 0.600
			Limit	ted to weir flow at low heads
#3	Device 1	675.75'	2.0"	Vert. Orifice/Grate X 4.00 C= 0.600
			Limit	ted to weir flow at low heads
#4	Device 1	678.00'	4.0"	Vert. Orifice/Grate X 4.00 C= 0.600
			Limi	ted to weir flow at low heads

Primary OutFlow Max=2.44 cfs @ 12.16 hrs HW=679.16' TW=672.45' (Dynamic Tailwater) 1=Culvert (Passes 2.44 cfs of 6.72 cfs potential flow) 2=Orifice/Grate (Controls 0.00 cfs) 3=Orifice/Grate (Orifice Controls 0.77 cfs @ 8.78 fps) 4=Orifice/Grate (Orifice Controls 1.68 cfs @ 4.80 fps)

Summary for Link AP4-P: AP4 - To Wetland A (A32-A47)

 Inflow Area =
 307,487 sf, 32.10% Impervious, Inflow Depth =
 2.71" for 25YearMass event

 Inflow =
 13.20 cfs @
 12.11 hrs, Volume=
 69,516 cf

 Primary =
 13.20 cfs @
 12.11 hrs, Volume=
 69,516 cf

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P4.1: To Wetland A (A32-A47)

Runoff = 11.78 cfs @ 12.10 hrs, Volume= 37,852 cf, Depth= 5.09" Routed to Link AP4-P : AP4 - To Wetland A (A32-A47)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN E	Description		
	630	55 V	Voods, Go	od, HSG B	
	73,383	77 V	Voods, Go	od, HSG D	
	4,661	61 >	75% Gras	s cover, Go	ood, HSG B
	10,490	80 >	75% Gras	s cover, Go	ood, HSG D
	89,164	76 V	Veighted A	verage	
	89,164 100.00% Pervious Are			ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.7	50	0.1500	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.2	135	0.1500	1.94		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
6.9	185	Total			

Summary for Subcatchment P4.10: To RG4.1

Runoff = 1.77 cfs @ 12.10 hrs, Volume= Routed to Pond RG-4.1 : Rain Garden 4.1 5,773 cf, Depth= 5.56"

	Area	ı (sf)	CN	Description		
	12	,335	80	>75% Gras	s cover, Go	ood, HSG D
		125	61	>75% Gras	s cover, Go	ood, HSG B
	12,460 80 Weighted Average					
	12,460 100.00% Pervious A					a
		ength	Slope		Capacity	Description
(mii	า)	(feet)	(ft/ft) (ft/sec)	(cfs)	
5	.7	50	0.1500	0.15		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
1.	.3	150	0.1500	1.94		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
7.	.0	200	Total			

Summary for Subcatchment P4.11: To RG4.2

Runoff = 1.36 cfs @ 12.11 hrs, Volume= 4,417 cf, Depth= 3.39" Routed to Pond RG-4.2 : Rain Garden 4.2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	A	rea (sf)	CN I	Description		
		15,520				ood, HSG B
_		110	80 :	>75% Gras	s cover, Go	ood, HSG D
	15,630 61 Weighted Average					
	15,630 100.00% Pervious Area					a
	·					
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.7	50	0.1500	0.15		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	1.3	150	0.1500	1.94		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
_	7.0	200	Total			

Summary for Subcatchment P4.12: Lots 65-67

Runoff	=	1.96 cfs @	12.07 hrs,	Volume=
Routed	to Pond	RG65-66 : R	ain Garden	s 65-66

ns 65-66

6,304 cf, Depth= 6.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN	Description		
	5,185	90	1/8 acre lots	s, 65% imp	o, HSG C
	5,850	92	1/8 acre lots	s, 65% imp	, HSG D
	11,035	91	Weighted A	verage	
	3,862		35.00% Per	vious Area	1
	7,173		65.00% Imp	pervious Ar	ea
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry,

Summary for Subcatchment P4.13: Lots 6-8

Runoff = 3.88 cfs @ 12.07 hrs, Volume= 12,007 cf, Depth= 6.15" Routed to Pond RG6-7 : Rain Gardens 6,7

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93" Printed 5/1/2023

Page 52

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description		
	23,175	85	1/8 acre lots	s, 65% imp	, HSG B
	270	90	1/8 acre lots	s, 65% imp	, HSG C
	23,445	85	Weighted A	verage	
	8,206		35.00% Per	vious Area	l
	15,239		65.00% Imp	ervious Are	ea
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P4.2: Lots 57-60

Runoff = 3.46 cfs @ 12.07 hrs, Volume= 10,723 cf, Depth= 6.15" Routed to Pond RG57-59 : Rain Gardens 57,58,59

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Area (sf)	CN	Description						
20,938	85	85 1/8 acre lots, 65% imp, HSG B						
7,328		35.00% Pervious Area						
13,610		65.00% Imp	pervious Ar	rea				
Tc Lengtł <u>(min) (feet</u> 5.0			Capacity (cfs)	Description Direct Entry,				

Summary for Subcatchment P4.4: Lots 60-65

Runoff	=	6.20 cfs @	12.07 hrs,	Volume=	19,548 cf,	Depth= 6.50"
Routed	to Pond	I RG60-64 : F	s 60-64			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN	Description			
	21,420	85	1/8 acre lots	s, 65% imp	HSG B	
	10,990	92	1/8 acre lots	s, 65% imp	HSG D	
	3,680	90	1/8 acre lots	s, 65% imp	HSG C	
	36,090	88	Weighted A	verage		
	12,632		35.00% Per	vious Area		
	23,459		65.00% Imp	pervious Are	a	
Та	l a ra artha	Clana	Valasity	Conseitu	Deceminitien	
TC	Length	Slope		Capacity	Description	
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)		
5.0					Direct Entry,	

AP4

Summary for Subcatchment P4.5: Lots 8-12

Runoff = 5.13 cfs @ 12.07 hrs, Volume= 15,876 cf, Depth= 6.15" Routed to Pond RG8-11 : Rain Gardens 8-11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Area (sf)	CN	Description							
31,000	85	85 1/8 acre lots, 65% imp, HSG B							
10,850		35.00% Pervious Area							
20,150		65.00% Imp	pervious Ar	ea					
Tc Length (min) (feet)	Slop (ft/f	,	Capacity (cfs)	Description					
5.0				Direct Entry,					
	Summary for Subcatchmont P4 6: Lots 12 13								

Summary for Subcatchment P4.6: Lots 12-13

Runoff = 1.68 cfs @ 12.09 hrs, Volume= 5,467 cf, Depth= 6.15" Routed to Pond RG12 : Rain Garden 12

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

_	A	rea (sf)	CN E	Description		
		10,675	85 1	/8 acre lots	s, 65% imp	, HSG B
_		3,736	3	5.00% Per	vious Area	
		6,939	6	5.00% Imp	ervious Ar	ea
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	6.2	50	0.1200	0.13		Sheet Flow,
	0.2	30	0.1000	2.21		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
-	6.4	80	Total			

Summary for Subcatchment P4.7: Lot 13

Runoff	=	1.60 cfs @	12.07 hrs,	Volume=	4,	982 cf,	Depth= 6.26"	
Routed	I to Pond	RG13 : Rain	Garden 13					

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93" Printed 5/1/2023

Page 54

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description		
	7,505	85	1/8 acre lot	s, 65% imp	, HSG B
	2,040	90	1/8 acre lots	s, 65% imp	, HSG C
	9,545	86	Weighted A	verage	
	3,341		35.00% Per	vious Area	
	6,204		65.00% Imp	pervious Are	ea
Tc	Length	Slope		Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry,

Summary for Subcatchment P4.8: Upgradient Lots 8-12

Runoff = 2.07 cfs @ 12.23 hrs, Volume= 9,101 cf, Depth= 2.84" Routed to Pond IT-8/12 : Interceptor Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	A	rea (sf)	CN	Description		
		3,850	61	>75% Gras	s cover, Go	bod, HSG B
		34,550	55	Woods, Go	od, HSG B	
		38,400	56	Weighted A	verage	
		38,400		100.00% Pe	ervious Are	a
-	Гс	Length	Slope		Capacity	Description
(mi	n)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
12	.7	50	0.0200	0.07		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
3	.5	280	0.0700	1.32		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
16	.2	330	Total			

Summary for Subcatchment P4.9: Lot 51

Runoff = 1.53 cfs @ 12.07 hrs, Volume= 4,752 cf, Depth= 6.26" Routed to Pond RG51 : Rain Garden 51

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Area (sf)	CN	Description
1,910	90	1/8 acre lots, 65% imp, HSG C
7,195	85	1/8 acre lots, 65% imp, HSG B
9,105	86	Weighted Average
3,187		35.00% Pervious Area
5,918		65.00% Impervious Area

AP4

AP4 Prepared by Mcclure Engine HydroCAD® 10.20-2g_s/n 03362		Postdevelopment <i>Type III 24-hr 100YearMass Rainfall=7.93</i> " Printed 5/1/2023 tware Solutions LLC Page 55			
0 1	elocity Capacity De (ft/sec) (cfs)	scription			
5.0	Dir	ect Entry,			
Summary for Pond IT-8/12: Interceptor Trench					
		, Inflow Depth = 2.84" for 100YearMass event			
	12.23 hrs, Volume= 12.34 hrs, Volume=				
Primary = 1.77 cfs @ 12.34 hrs, Volume= 9,099 cf Routed to Link AP4-P : AP4 - To Wetland A (A32-A47)					
Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 681.75' @ 12.34 hrs Surf.Area= 1,050 sf Storage= 682 cf					

Plug-Flow detention time= 10.2 min calculated for 9,099 cf (100% of inflow) Center-of-Mass det. time= 10.1 min (875.3 - 865.3)

Volume	Invert	Avail.Stor	rage	Storage Description
#1	680.30'	1,21	11 cf	3.00'W x 350.00'L x 3.00'H Prismatoid
#2	680.30'	12	22 cf	3,150 cf Overall - 122 cf Embedded = 3,028 cf x 40.0% Voids 8.0" Round Pipe Storage Inside #1 L= 350.0'
		1,33	33 cf	Total Available Storage
Device	Routing	Invert	Outl	et Devices
#1	Primary	680.30'	L= 2 Inlet	Round Culvert 24.0' CPP, square edge headwall, Ke= 0.500 : / Outlet Invert= 680.30' / 668.00' S= 0.0549 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=1.77 cfs @ 12.34 hrs HW=681.75' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 1.77 cfs @ 5.08 fps)

Summary for Pond RG-4.1: Rain Garden 4.1

Inflow Area =	114,030 sf,	57.90% Impervious,	Inflow Depth = 5.09"	for 100YearMass event
Inflow =	12.17 cfs @	12.14 hrs, Volume=	48,397 cf	
Outflow =	7.59 cfs @	12.43 hrs, Volume=	46,972 cf, Atte	en= 38%, Lag= 17.0 min
Discarded =	0.36 cfs @	12.43 hrs, Volume=	10,635 cf	
Primary =	6.88 cfs @	12.43 hrs, Volume=	36,145 cf	
Routed to Link	AP4-P : AP4 ·	- To Wetland A (A32-A	47)	
Secondary =	0.34 cfs @	12.43 hrs, Volume=	192 cf	
Routed to Link	AP4-P : AP4 ·	- To Wetland A (A32-A	47)	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 673.56' @ 12.43 hrs Surf.Area= 15,317 sf Storage= 14,279 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 75.0 min (870.7 - 795.8)

Postdevelopment fall=7.93" 5/1/2023

Page 56

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Avail.Storage

Printed	Type III 24-hr	100YearMass Rainfa
		Printed

Storage Description 12.00'W x 360.00'L x 3.00'H Soil Media and Gravel #1 668.75' 5.184 cf 12,960 cf Overall x 40.0% Voids #2 671.75' 540 cf 12.00'W x 360.00'L x 0.25'H Mulch 1,080 cf Overall x 50.0% Voids 12.00'W x 360.00'L x 2.00'H Ponding Z=2.0 #3 672.00' 11.659 cf 17,383 cf Total Available Storage Device Routing Invert **Outlet Devices** 1.020 in/hr Exfiltration over Surface area #1 Discarded 668.75' #2 Primary 670.75' 8.0" Round Culvert X 4.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 670.75' / 670.65' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf #3 Device 2 670.75' 2.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads #4 Device 2 671.25' 4.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads #5 Device 2 673.25' 8.0" Horiz. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads #6 Device 2 673.25' 8.0" Horiz. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads #7 Secondary 673.50' 10.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=0.36 cfs @ 12.43 hrs HW=673.56' (Free Discharge) -1=Exfiltration (Exfiltration Controls 0.36 cfs)

Primary OutFlow Max=6.88 cfs @ 12.43 hrs HW=673.56' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 6.88 cfs of 10.58 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.69 cfs @ 7.95 fps)

-4=Orifice/Grate (Orifice Controls 2.46 cfs @ 7.05 fps)

-5=Orifice/Grate (Orifice Controls 1.86 cfs @ 2.67 fps)

-6=Orifice/Grate (Orifice Controls 1.86 cfs @ 2.67 fps)

Secondary OutFlow Max=0.34 cfs @ 12.43 hrs HW=673.56' TW=0.00' (Dynamic Tailwater) -7=Broad-Crested Rectangular Weir (Weir Controls 0.34 cfs @ 0.60 fps)

Summary for Pond RG-4.2: Rain Garden 4.2

Inflow Area =	65,893 sf,	49.58% Impervious,	Inflow Depth = 5.37"	for 100YearMass event
Inflow =	8.36 cfs @	12.13 hrs, Volume=	29,488 cf	
Outflow =	4.30 cfs @	12.44 hrs, Volume=	28,707 cf, Atter	i= 48%, Lag= 18.3 min
Discarded =	0.20 cfs @	12.44 hrs, Volume=	5,312 cf	
Primary =	4.11 cfs @	12.44 hrs, Volume=	23,394 cf	
Routed to Link	AP4-P : AP4	- To Wetland A (A32-A	.47)	
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Routed to Link	AP4-P: AP4	- To Wetland A (A32-A	.47)	

AP4

Volume

Invert

		Postdevelopment
AP4	Type III 24-hr	100YearMass Rainfall=7.93"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Softwar	re Solutions LLC	Page 57

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 680.19' @ 12.44 hrs Surf.Area= 8,417 sf Storage= 6,766 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 57.3 min (857.1 - 799.8)

Volume	Invert	Avail.Storage	Storage Description
#1	675.75'	2,880 cf	10.00'W x 240.00'L x 3.00'H Soil Media and Gravel
			7,200 cf Overall x 40.0% Voids
#2	678.75'	300 cf	10.00'W x 240.00'L x 0.25'H Mulch
			600 cf Overall x 50.0% Voids
#3	679.00'	6,843 cf	10.00'W x 240.00'L x 2.00'H Ponding Z=2.0
		10,023 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	675.75'	1.020 in/hr Exfiltration over Surface area
#2	Primary	677.75'	6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 677.75' / 677.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	677.75'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	678.25'	
			Limited to weir flow at low heads
#5	Device 2	680.50'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#6	Secondary	680.50'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Discarded OutFlow Max=0.20 cfs @ 12.44 hrs HW=680.19' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.20 cfs)

Primary OutFlow Max=4.11 cfs @ 12.44 hrs HW=680.19' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 4.11 cfs of 5.60 cfs potential flow)

1-3=Orifice/Grate (Orifice Controls 0.65 cfs @ 7.40 fps)

-4=Orifice/Grate (Orifice Controls 3.46 cfs @ 6.34 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=675.75' TW=0.00' (Dynamic Tailwater) -6=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG12: Rain Garden 12

Inflow Are	a =	10,675 sf, 65.00% Impervious	, Inflow Depth = 6.15" for 100YearMass event		
Inflow	=	1.68 cfs @ 12.09 hrs, Volume=	5,467 cf		
Outflow	=	1.33 cfs @ 12.15 hrs, Volume=	5,467 cf, Atten= 21%, Lag= 3.8 min		
Primary	=	1.33 cfs @ 12.15 hrs, Volume=	5,467 cf		
Routed to Pond RG-4.2 : Rain Garden 4.2					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

Peak Elev= 690.36' @ 12.15 hrs Surf.Area= 953 sf Storage= 687 cf

Plug-Flow detention time= 15.7 min calculated for 5,465 cf (100% of inflow) Center-of-Mass det. time= 15.7 min (806.8 - 791.1)

Volume	Invert	Avail.Storage	Storage Description
#1	686.25'	106 cf	18.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	686.25'	246 ct	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	689.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	689.50'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		755 of	Tatal Available Starage

755 cf Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	686.25'	6.0" Round Culvert
	-		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 686.25' / 686.15' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	690.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	686.25'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	688.50'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.33 cfs @ 12.15 hrs HW=690.36' TW=679.76' (Dynamic Tailwater)

-1=Culvert (Passes 1.33 cfs of 1.86 cfs potential flow)

1-2=Orifice/Grate (Orifice Controls 0.57 cfs @ 2.90 fps)

-3=Orifice/Grate (Orifice Controls 0.21 cfs @ 9.66 fps)

-4=Orifice/Grate (Orifice Controls 0.55 cfs @ 6.27 fps)

Summary for Pond RG13: Rain Garden 13

Inflow Are	a =	9,545 sf, 6	65.00% Impervious,	Inflow Depth =	6.26"	for 100YearMass event
Inflow	=	1.60 cfs @ 12	2.07 hrs, Volume=	4,982 c	f	
Outflow	=	1.25 cfs @ 12	2.13 hrs, Volume=	4,982 c	f, Atter	n= 22%, Lag= 3.5 min
Primary	=	1.25 cfs @ 12	2.13 hrs, Volume=	4,982 c	f	-
Routed to Pond RG-4.2 : Rain Garden 4.2						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 698.79' @ 12.13 hrs Surf.Area= 932 sf Storage= 653 cf

Plug-Flow detention time= 15.7 min calculated for 4,980 cf (100% of inflow) Center-of-Mass det. time= 15.7 min (802.9 - 787.1)

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

	0	0		
HydroCAD® 10.20-2g	s/n 03362	© 2022 Hy	droCAD Software Solutions LLC	Page 59

Volume	Invert	Avail.Storage	Storage Description
#1	694.75'	106 cf	
#2	694.75'	246 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 106 cf Embedded = 614 cf x 40.0% Voids
#3	697.75'	30 cf	
#4	698.00'	373 cf	60 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
	000.00	755 cf	Total Available Storage
			5
Device	Routing	Invert Out	let Devices
#1	Primary	694.75' 6.0 "	' Round Culvert
	•	L= 1	10.0' CPP, square edge headwall, Ke= 0.500
			t / Outlet Invert= 694.75' / 694.65' S= 0.0100 '/' Cc= 0.900
		n= (0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		Horiz. Orifice/Grate C= 0.600
		Limi	ted to weir flow at low heads
#3	Device 1	694.75' 2.0 "	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.25 cfs @ 12.13 hrs HW=698.79' TW=679.64' (Dynamic Tailwater)

-1=Culvert (Passes 1.25 cfs of 1.84 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 0.51 cfs @ 2.59 fps)

-3=Orifice/Grate (Orifice Controls 0.21 cfs @ 9.58 fps)

-4=Orifice/Grate (Orifice Controls 0.54 cfs @ 6.13 fps)

Summary for Pond RG51: Rain Garden 51

Inflow Are	a =	9,105 sf,	65.00% Impervious	Inflow Depth =	6.26"	for 100YearMass event
Inflow	=	1.53 cfs @	12.07 hrs, Volume=	4,752 0	of	
Outflow	=	1.20 cfs @	12.13 hrs, Volume=	4,752 0	of, Atter	n= 21%, Lag= 3.5 min
Primary	=	1.20 cfs @	12.13 hrs, Volume=	4,752 0	of	-
Routed to Pond RG-4.2 : Rain Garden 4.2						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 695.75' @ 12.13 hrs Surf.Area= 920 sf Storage= 599 cf

Plug-Flow detention time= 14.2 min calculated for 4,751 cf (100% of inflow) Center-of-Mass det. time= 14.2 min (801.4 - 787.1)

Volume	Invert	Avail.Storage	Storage Description
#1	691.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	691.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	694.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	695.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage

AP4

Prepared by Mcclure Engineeering

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

Page 60

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g_s/n 03362_© 2022 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	691.75'	6.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 691.75' / 691.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	695.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	691.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	694.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.20 cfs @ 12.13 hrs HW=695.75' TW=679.63' (Dynamic Tailwater)

-**1=Culvert** (Passes 1.20 cfs of 1.83 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.47 cfs @ 2.39 fps)

-3=Orifice/Grate (Orifice Controls 0.21 cfs @ 9.52 fps)

-4=Orifice/Grate (Orifice Controls 0.53 cfs @ 6.05 fps)

Summary for Pond RG57-59: Rain Gardens 57,58,59

Inflow Area =	20,938 sf, 65.00% Impervious,	Inflow Depth = 6.15" for 100YearMass event				
Inflow =	3.46 cfs @ 12.07 hrs, Volume=	10,723 cf				
Outflow =	2.61 cfs @ 12.14 hrs, Volume=	10,723 cf,Atten= 25%,Lag= 3.9 min				
Discarded =	0.15 cfs @ 12.14 hrs, Volume=	3,366 cf				
Primary =	2.46 cfs @ 12.14 hrs, Volume=	7,357 cf				
Routed to Pond RG-4.2 : Rain Garden 4.2						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 691.12' @ 12.14 hrs Surf.Area= 2,656 sf Storage= 1,639 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 64.7 min (854.6 - 789.8)

Volume	Invert	Avail.Storage	Storage Description
#1	687.25'	141 cf	12.0" Round Pipe Storage x 3 Inside #2
			L= 60.0'
#2	687.25'	807 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 3
			2,160 cf Overall - 141 cf Embedded = 2,019 cf x 40.0% Voids
#3	690.25'	90 cf	4.00'W x 60.00'L x 0.25'H Mulch x 3
			180 cf Overall x 50.0% Voids
#4	690.50'	1,120 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 3
		2,159 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	687.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	689.25'	6.0" Round Culvert X 3.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 689.25' / 689.00' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	691.00'	6.0" Horiz. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 2	689.25'	2.0" Vert. Orifice/Grate X 3.00 C= 0.600
			Limited to weir flow at low heads

AP4

		Postdevelopment
AP4	Type III 24-hr	100YearMass Rainfall=7.93"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software	Solutions LLC	Page 61

#5 Device 2 689.75' **4.0" Vert. Orifice/Grate X 3.00** C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.15 cfs @ 12.14 hrs HW=691.12' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.15 cfs)

Primary OutFlow Max=2.45 cfs @ 12.14 hrs HW=691.12' TW=679.67' (Dynamic Tailwater) -2=Culvert (Passes 2.45 cfs of 3.61 cfs potential flow) -3=Orifice/Grate (Weir Controls 0.65 cfs @ 1.14 fps) -4=Orifice/Grate (Orifice Controls 0.42 cfs @ 6.44 fps)

-5=Orifice/Grate (Orifice Controls 1.38 cfs @ 5.28 fps)

Summary for Pond RG6-7: Rain Gardens 6,7

Inflow Are	a =	23,445 sf, 65.00% Impervious,	Inflow Depth = 6.15" for 100YearMass event			
Inflow	=	3.88 cfs @ 12.07 hrs, Volume=	12,007 cf			
Outflow	=	2.93 cfs @ 12.14 hrs, Volume=	12,006 cf, Atten= 24%, Lag= 3.8 min			
Primary	=	2.93 cfs @ 12.14 hrs, Volume=	12,006 cf			
Routed to Pond RG-4.1 : Rain Garden 4.1						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 680.75' @ 12.14 hrs Surf.Area= 2,207 sf Storage= 1,450 cf

Plug-Flow detention time= 9.6 min calculated for 12,006 cf (100% of inflow) Center-of-Mass det. time= 9.5 min (799.4 - 789.8)

Volume	Invert	Avail.Storage	Storage Description
#1	676.75'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
			L= 60.0'
#2	676.75'	682 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel × 2
			1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids
#3	679.75'	75 cf	5.00'W x 60.00'L x 0.25'H Mulch x 2
			150 cf Overall x 50.0% Voids
#4	680.00'	871 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	676.75'	6.0" Round Culvert X 2.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 676.75' / 676.65' S= 0.0100 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	680.50'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	676.75'	3.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	679.00'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=2.93 cfs @ 12.14 hrs HW=680.75' TW=672.95' (Dynamic Tailwater)

2=Orifice/Grate (Orifice Controls 0.94 cfs @ 2.39 fps)

3=Orifice/Grate (Orifice Controls 0.94 cfs @ 2.95 fps)

4=Orifice/Grate (Orifice Controls 1.06 cfs @ 6.05 fps)

Summary for Pond RG60-64: Rain Gardens 60-64

Inflow Area =	36,090 sf, 65.00% Impervious,	Inflow Depth = 6.50" for 100YearMass event
Inflow =	6.20 cfs @ 12.07 hrs, Volume=	19,548 cf
Outflow =	4.49 cfs @ 12.14 hrs, Volume=	19,549 cf, Atten= 28%, Lag= 4.2 min
Discarded =	0.30 cfs @ 12.14 hrs, Volume=	6,987 cf
Primary =	3.35 cfs @ 12.14 hrs, Volume=	10,050 cf
Routed to Pond	RG-4.1 : Rain Garden 4.1	
Secondary =	0.84 cfs @ 12.14 hrs, Volume=	2,512 cf
Routed to Pond	I RG-4.2 : Rain Garden 4.2	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 687.63' @ 12.14 hrs Surf.Area= 5,348 sf Storage= 3,333 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 69.7 min (851.1 - 781.4)

Volume	Invert	Avail.Storage	Storage Description
#1	683.75'	236 cf	12.0" Round Pipe Storage x 5 Inside #2
			L= 60.0'
#2	683.75'	1,706 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 5
			4,500 cf Overall - 236 cf Embedded = 4,264 cf x 40.0% Voids
#3	686.75'	188 cf	5.00'W x 60.00'L x 0.25'H Mulch x 5
			375 cf Overall x 50.0% Voids
#4	687.00'	2,177 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 5
		4,306 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1 #2	Discarded Primary	683.75' 685.75'	2.410 in/hr Exfiltration over Surface area 6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 685.75' / 684.65' S= 0.1100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Secondary	685.75'	6.0" Round Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 685.75' / 684.65' S= 0.1100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#4	Device 2	687.50'	•
#5	Device 3	687.50'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#6	Device 2	685.75'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
#7	Device 3	685.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

AP4Type III 24-hr100YearMass Rainfall=7.93"Prepared by Mcclure EngineeeringPrinted 5/1/2023HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLCPage 63

 #8 Device 2 686.25' 4.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads
 #9 Device 3 686.25' 4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.30 cfs @ 12.14 hrs HW=687.63' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.30 cfs)

Primary OutFlow Max=3.35 cfs @ 12.14 hrs HW=687.63' TW=672.98' (Dynamic Tailwater)

2=Culvert (Passes 3.35 cfs of 4.82 cfs potential flow)

-4=Orifice/Grate (Weir Controls 0.94 cfs @ 1.17 fps)

-6=Orifice/Grate (Orifice Controls 0.56 cfs @ 6.45 fps) -8=Orifice/Grate (Orifice Controls 1.85 cfs @ 5.30 fps)

Secondary OutFlow Max=0.84 cfs @ 12.14 hrs HW=687.63' TW=679.70' (Dynamic Tailwater)

-3=Culvert (Passes 0.84 cfs of 1.21 cfs potential flow)

5=Orifice/Grate (Weir Controls 0.23 cfs @ 1.17 fps)

-7=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.45 fps)

-9=Orifice/Grate (Orifice Controls 0.46 cfs @ 5.30 fps)

Summary for Pond RG65-66: Rain Gardens 65-66

Inflow Area =	11,035 sf, 65.00% Impervious,	Inflow Depth = 6.86" for 100YearMass event
Inflow =	1.96 cfs @ 12.07 hrs, Volume=	6,304 cf
Outflow =	1.18 cfs @ 12.16 hrs, Volume=	6,063 cf,Atten= 40%,Lag= 5.6 min
Discarded =	0.05 cfs @ 12.16 hrs, Volume=	1,370 cf
Primary =	1.13 cfs @ 12.16 hrs, Volume=	4,693 cf
Routed to Pond	RG-4.1 : Rain Garden 4.1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 678.96' @ 12.16 hrs Surf.Area= 2,047 sf Storage= 1,186 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 68.4 min (840.3 - 771.9)

Volume	Invert	Avail.Storage		Storage Description
#1	675.25'	94 cf		12.0" Round Pipe Storage x 2 Inside #2 L= 60.0'
#2	675.25'	682 cf		5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,800 cf Overall - 94 cf Embedded = 1,706 cf x 40.0% Voids
#3	678.25'	75 cf		5.00'W x 60.00'L x 0.25'H Mulch x 2 150 cf Overall x 50.0% Voids
#4	678.50'	871 cf		5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,722 cf		Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Discarded	675.25'	1.02	0 in/hr Exfiltration over Surface area
#2	Primary	677.25'	6.0"	Round Culvert X 2.00
			L= 1	0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 677.25' / 675.15' S= 0.2100 '/' Cc= 0.900
				.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	679.00'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600

		Postdevelopment
AP4	Type III 24-hr	100YearMass Rainfall=7.93"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC	Page 64

			Limited to weir flow at low heads
#4	Device 2	677.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#5	Device 2	677.75'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.05 cfs @ 12.16 hrs HW=678.96' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=1.13 cfs @ 12.16 hrs HW=678.96' TW=673.11' (Dynamic Tailwater) 2=Culvert (Passes 1.13 cfs of 2.29 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -4=Orifice/Grate (Orifice Controls 0.27 cfs @ 6.15 fps) -5=Orifice/Grate (Orifice Controls 0.86 cfs @ 4.92 fps)

Summary for Pond RG8-11: Rain Gardens 8-11

Inflow Are	a =	31,000 sf, 65.00% Impervious	, Inflow Depth = 6.15" for 100YearMass event			
Inflow	=	5.13 cfs @ 12.07 hrs, Volume=	15,876 cf			
Outflow	=	3.25 cfs @ 12.16 hrs, Volume=	15,875 cf, Atten= 37%, Lag= 5.3 min			
Primary	=	3.25 cfs @ 12.16 hrs, Volume=	15,875 cf			
Routed to Pond RG-4.1 : Rain Garden 4.1						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 679.58' @ 12.16 hrs Surf.Area= 4,222 sf Storage= 2,715 cf

Plug-Flow detention time= 19.5 min calculated for 15,870 cf (100% of inflow) Center-of-Mass det. time= 19.5 min (809.3 - 789.8)

Volume	Invert	Avail.Storage		Storage Description		
#1	675.75'	424 cf		18.0" Round Pipe Storage x 4 Inside #2 L= 60.0'		
#2	675.75'	1,27	70 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 4 3,600 cf Overall - 424 cf Embedded = 3,176 cf x 40.0% Voids		
#3	678.75'	150 cf		5.00'W x 60.00'L x 0.25'H Mulch x 4 300 cf Overall x 50.0% Voids		
#4	679.00'	1,74	11 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 4		
		3,58	36 cf	Total Available Storage		
Device	Routing	Invert	Outle	et Devices		
#1	Primary	675.75'	L= 1 Inlet	Round Culvert X 4.00 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 675.75' / 675.65' S= 0.0100 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	679.50'	6.0"	Horiz. Orifice/Grate X 4.00 C= 0.600 ted to weir flow at low heads		
#3	Device 1	675.75'	-	Vert. Orifice/Grate X 4.00 C= 0.600 ted to weir flow at low heads		
#4	Device 1	678.00'	4.0"	Limited to weir flow at low heads 4.0" Vert. Orifice/Grate X 4.00 C= 0.600 Limited to weir flow at low heads		

Primary OutFlow Max=3.25 cfs @ 12.16 hrs HW=679.58' TW=673.08' (Dynamic Tailwater) **1=Culvert** (Passes 3.25 cfs of 7.15 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.44 cfs @ 0.91 fps)

-3=Orifice/Grate (Orifice Controls 0.81 cfs @ 0.91 ips)

4=Orifice/Grate (Orifice Controls 2.00 cfs @ 5.72 fps)

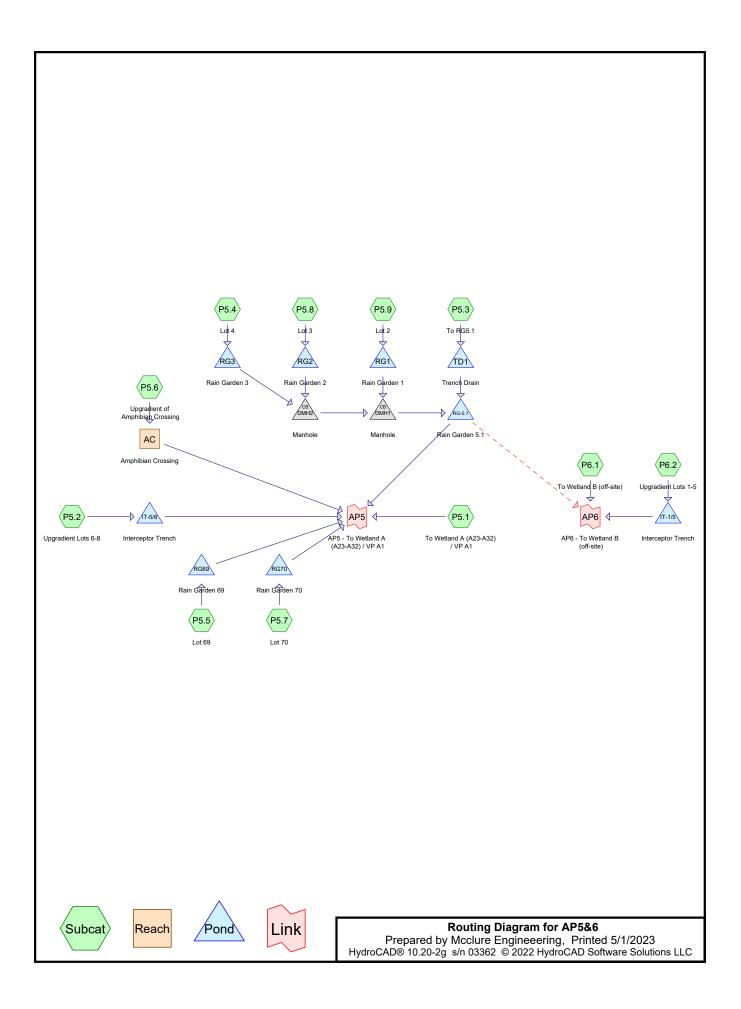
Summary for Link AP4-P: AP4 - To Wetland A (A32-A47)

 Inflow Area =
 307,487 sf, 32.10% Impervious, Inflow Depth =
 4.16"
 for 100YearMass event

 Inflow =
 18.44 cfs @
 12.11 hrs, Volume=
 106,682 cf

 Primary =
 18.44 cfs @
 12.11 hrs, Volume=
 106,682 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



Summary for Subcatchment P5.1: To Wetland A (A23-A32) / VP A1

Runoff = 2.05 cfs @ 12.14 hrs, Volume= 7,502 cf, Depth> 1.06" Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN [Description		
	34,145	70 V	Voods, Go	od, HSG C	
	38,725	77 V	Voods, Go	od, HSG D	
	2,390	80 >	75% Gras	s cover, Go	ood, HSG D
	8,015	74 >	75% Gras	s cover, Go	bod, HSG C
	1,535	98 l	Inconnecte	ed roofs, HS	SG C
	84,810	74 V	Veighted A	verage	
	83,275	ç	8.19% Per	vious Area	
	1,535	1	.81% Impe	ervious Area	а
	1,535	1	00.00% Üı	nconnected	1
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.5	50	0.0750	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.8	150	0.0750	1.37		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
9.3	200	Total			

Summary for Subcatchment P5.2: Upgradient Lots 6-8

Runoff	=	0.06 cfs @	12.32 hrs,	Volume=	518 cf,	Depth>	0.29"
Routed	I to Pond	d IT-5/8 : Inter	ceptor Tren	ich			

Area (sf)	CN	Description
300	61	>75% Grass cover, Good, HSG B
485	74	>75% Grass cover, Good, HSG C
19,455	55	Woods, Good, HSG B
1,085	70	Woods, Good, HSG C
21,325	56	Weighted Average
21,325		100.00% Pervious Area

	d by Mco		gineeering		Postdevelopment <i>Type III 24-hr 2YearMass Rainfall=3.24"</i> Printed 5/1/2023 Software Solutions LLC Page 3			
IIJUIOCA	D® 10.20-	<u>zy s/1103</u>	<u> 302 @ 202</u>		Fage 5			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.7	50		0.12		Sheet Flow,			
0.9	85	0.1000	1.58		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps			
0.0	5	0.1000	2.21		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
7.6	140	Total			· · · · · ·			
		:	Summar	y for Sub	catchment P5.3: To RG5.1			
Runoff Route	Runoff = 1.31 cfs @ 12.07 hrs, Volume= 3,957 cf, Depth> 2.20" Routed to Pond TD1 : Trench Drain							
			nod, UH=S Rainfall=3		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs			
А	rea (sf)	CN D	escription					
	21,545			s, 65% imp	HSG C			
	7,541 14,004	3	5.00% Pei	vious Area pervious Are				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			
			Summ	ary for S	ubcatchment P5.4: Lot 4			
Runoff Route	= ed to Pon		s @ 12.0 Rain Garde	7 hrs, Volu en 3	me= 1,148 cf, Depth> 2.03"			
			nod, UH=S Rainfall=3		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs			
A	rea (sf)	CN D	escription					
	6,030			s, 65% imp	, HSG C			
	750	70 V	Voods, Go	od, HSG Ċ				
	6,780		Veighted A					
	2,861 3,920			vious Area pervious Are				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P5.5: Lot 69

667 cf, Depth> 2.20"

936 cf, Depth> 0.50"

Runoff = 0.22 cfs @ 12.07 hrs, Volume= Routed to Pond RG69 : Rain Garden 69

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Area (sf)	CN	Description		
3,630	90	1/8 acre lots	s, 65% imp	p, HSG C
1,271 2,360 Tc Length (min) (feet)	Slop (ft/	,		rea ⁄ Description
5.0				Direct Entry,

Summary for Subcatchment P5.6: Upgradient of Amphibian Crossing

Runoff = 0.19 cfs @ 12.15 hrs, Volume= Routed to Reach AC : Amphibian Crossing

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN [Description		
	1,860				ood, HSG C
	8,330	70 N	Voods, Go	od, HSG C	
	12,415	55 V	Voods, Go	od, HSG B	
	22,605	62 V	Veighted A	verage	
	22,605			ervious Are	а
	,				
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
6.7	50	0.1000	0.12		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.6	150	0.1000	1.58		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.1	30	0.3000	3.83		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
8.4	230	Total			

Summary for Subcatchment P5.7: Lot 70

Runoff = 0.33 cfs @ 12.07 hrs, Volume= 994 cf, Depth> 2.20" Routed to Pond RG70 : Rain Garden 70

Prepared by Mcclure Engineeering

A	rea (sf)	CN [Description		
	5,415	90 1	/8 acre lots	s, 65% imp	, HSG C
	1,895	3	35.00% Per	vious Area	
	3,520	6	65.00% Imp	ervious Are	ea
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry,

Summary for Subcatchment P5.8: Lot 3

0.41 cfs @ 12.07 hrs, Volume= 1,253 cf, Depth> 2.20" Runoff = Routed to Pond RG2 : Rain Garden 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Are	ea (sf)	CN D	Description					
	6,820	90 1	/8 acre lots	s, 65% imp	o, HSG C			
	2,387 4,433	-	35.00% Pervious Area 65.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P5.9: Lot 2

Runoff	=	0.47 cfs @	12.07 hrs,	Volume=	1	1,418 cf,	Depth>	2.20"
Routed	l to Pond	d RG1 : Rain (Garden 1				·	

Α	rea (sf)	CN E	Description					
	7,720	90 1	/8 acre lots	s, 65% imp	, HSG C			
	2,702	3	35.00% Pervious Area					
	5,018	6	5.00% Imp	ervious Ar	ea			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

Summary for Subcatchment P6.1: To Wetland B (off-site)

Runoff = 1.28 cfs @ 12.14 hrs, Volume= 4,951 cf, Depth> 0.85" Routed to Link AP6 : AP6 - To Wetland B (off-site)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN [Description		
	6,465	74 >	>75% Gras	s cover, Go	ood, HSG C
	1,875	55 \	Noods, Go	od, HSG B	
	61,615	70 \	Noods, Go	od, HSG C	
	69,955	70 \	Neighted A	verage	
	69,955		100.00% Pe		а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.5	50	0.1600	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
0.3	45	0.2200	2.35		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.4	290	0.0800	1.41		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
9.2	385	Total			

Summary for Subcatchment P6.2: Upgradient Lots 1-5

Runoff = 0.28 cfs @ 12.20 hrs, Volume= 1,599 cf, Depth> 0.46" Routed to Pond IT-1/5 : Interceptor Trench

A	rea (sf)	CN [Description		
	2,130	74 >	75% Gras	s cover, Go	ood, HSG C
	24,405	55 N	Voods, Go	od, HSG B	
	15,310	70 V	Voods, Go	od, HSG C	
	41,845	61 V	Veighted A	verage	
	41,845	1	00.00% Pe	ervious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
8.2	50	0.0600	0.10		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
2.7	160	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
10.9	210	Total			

Summary for Reach AC: Amphibian Crossing

 Inflow Area =
 22,605 sf,
 0.00% Impervious,
 Inflow Depth >
 0.50"
 for 2YearMass event

 Inflow =
 0.19 cfs @
 12.15 hrs,
 Volume=
 936 cf

 Outflow =
 0.19 cfs @
 12.17 hrs,
 Volume=
 934 cf,
 Atten= 2%,
 Lag= 1.3 min

 Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1
 VP A1
 VP
 VP
 VP

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Max. Velocity= 0.28 fps, Min. Travel Time= 1.8 min Avg. Velocity = 0.14 fps, Avg. Travel Time= 3.6 min

Peak Storage= 20 cf @ 12.17 hrs Average Depth at Peak Storage= 0.11', Surface Width= 6.00' Bank-Full Depth= 3.50' Flow Area= 21.0 sf, Capacity= 35.65 cfs

6.00' x 3.50' deep channel, n= 0.022 Earth, clean & straight Length= 30.0' Slope= 0.0003 '/' Inlet Invert= 675.83', Outlet Invert= 675.82'



Summary for Pond DMH1: Manhole

 Inflow Area =
 21,320 sf, 62.71% Impervious, Inflow Depth > 2.14"
 for 2YearMass event

 Inflow =
 0.53 cfs @
 12.24 hrs, Volume=
 3,795 cf

 Outflow =
 0.53 cfs @
 12.24 hrs, Volume=
 3,795 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.53 cfs @
 12.24 hrs, Volume=
 3,795 cf

 Routed to Pond RG-5.1 : Rain Garden 5.1
 3,795 cf
 3,795 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 673.23' @ 12.55 hrs Flood Elev= 675.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	671.50'	12.0" Round Culvert L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= $671.50' / 671.00'$ S= $0.0100 '/$ ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.37 cfs @ 12.24 hrs HW=673.07' TW=673.06' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.37 cfs @ 0.48 fps)

	Postdevelopment
AP5&6	Type III 24-hr 2YearMass Rainfall=3.24"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Sc	blutions LLC Page 8

Summary for Pond DMH2: Manhole

Inflow Area = 13,600 sf, 61.42% Impervious, Inflow Depth > 2.11" for 2YearMass event Inflow 0.32 cfs @ 12.24 hrs, Volume= = 2.389 cf 0.32 cfs @ 12.24 hrs, Volume= 2,389 cf, Atten= 0%, Lag= 0.0 min Outflow = Primary = 0.32 cfs @ 12.24 hrs, Volume= 2,389 cf Routed to Pond DMH1 : Manhole Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 673.80' @ 12.43 hrs Flood Elev= 678.20' Device Routing Invert Outlet Devices Primary #1 673.50' 12.0" Round Culvert L= 80.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 673.50' / 671.60' S= 0.0237 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.32 cfs @ 12.24 hrs HW=673.79' TW=673.07' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.32 cfs @ 2.50 fps)

Summary for Pond IT-1/5: Interceptor Trench

 Inflow Area =
 41,845 sf,
 0.00% Impervious, Inflow Depth >
 0.46" for 2YearMass event

 Inflow =
 0.28 cfs @
 12.20 hrs, Volume=
 1,599 cf

 Outflow =
 0.24 cfs @
 12.34 hrs, Volume=
 1,580 cf, Atten= 12%, Lag= 8.5 min

 Primary =
 0.24 cfs @
 12.34 hrs, Volume=
 1,580 cf

 Routed to Link AP6 : AP6 - To Wetland B (off-site)
 1,580 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 677.28' @ 12.34 hrs Surf.Area= 750 sf Storage= 103 cf

Plug-Flow detention time= 14.7 min calculated for 1,579 cf (99% of inflow) Center-of-Mass det. time= 8.5 min (924.1 - 915.5)

Volume	Invert	Avail.Storage	e Storage Description		
#1	677.00'	865 c	of 3.00'W x 250.00'L x 3.00'H Prismatoid		
#2	677.00'	87 c	2,250 cf Overall - 87 cf Embedded = 2,163 cf x 40.0% Voids f 8.0" Round Pipe Storage Inside #1 L= 250.0'		
		952 c	of Total Available Storage		
Device	Routing	Invert O	utlet Devices		
#1	Primary	L= In	0" Round Culvert = 220.0' CPP, square edge headwall, Ke= 0.500 let / Outlet Invert= 677.00' / 668.00' S= 0.0409 '/' Cc= 0.900 = 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf		
Primary OutFlow Max=0.24 cfs @ 12.34 hrs HW=677.28' TW=0.00' (Dynamic Tailwater)					

1=Culvert (Inlet Controls 0.24 cfs @ 1.79 fps)

Summary for Pond IT-5/8: Interceptor Trench

Inflow Area = 21,325 sf, 0.00% Impervious, Inflow Depth > 0.29" for 2YearMass event 0.06 cfs @ 12.32 hrs, Volume= Inflow = 518 cf 0.06 cfs @ 12.43 hrs, Volume= 510 cf. Atten= 9%, Lag= 6.7 min Outflow = Primary = 0.06 cfs @ 12.43 hrs, Volume= 510 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1 Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 680.14' @ 12.43 hrs Surf.Area= 465 sf Storage= 31 cf Plug-Flow detention time= 16.8 min calculated for 510 cf (99% of inflow) Center-of-Mass det. time= 9.5 min (954.2 - 944.6) Volume Invert Avail.Storage Storage Description #1 680.00' 546 cf 3.00'W x 155.00'L x 3.00'H Prismatoid 1,395 cf Overall - 30 cf Embedded = 1,365 cf \times 40.0% Voids #2 680.00' 6.0" Round Pipe Storage Inside #1 30 cf L= 155.0' 576 cf Total Available Storage Device Routing **Outlet Devices** Invert 680.00' 6.0" Round Culvert #1 Primary L= 110.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 680.00' / 678.00' S= 0.0182 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf Primary OutFlow Max=0.06 cfs @ 12.43 hrs HW=680.14' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.06 cfs @ 1.28 fps)

Summary for Pond RG-5.1: Rain Garden 5.1

Inflow Area = 42,865 sf, 63.86% Impervious, Inflow Depth > 2.17" for 2YearMass event Inflow 1.66 cfs @ 12.08 hrs, Volume= 7,750 cf = Outflow 0.59 cfs @ 12.55 hrs, Volume= 6,166 cf, Atten= 65%, Lag= 28.2 min = 0.22 cfs @ 12.55 hrs, Volume= Discarded = 994 cf 5,172 cf Primary = 0.37 cfs @ 12.55 hrs, Volume= Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Link AP6 : AP6 - To Wetland B (off-site)

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 673.22' @ 12.55 hrs Surf.Area= 3,833 sf Storage= 2,922 cf

Plug-Flow detention time= 142.9 min calculated for 6,163 cf (80% of inflow) Center-of-Mass det. time= 67.7 min (885.0 - 817.3)

Postdevelopment

				Postdevelopment		
AP5&6	5			Type III 24-hr 2YearMass Rainfall=3.24"		
Prepare	ed by Mcclure	Engineeer	ina	Printed 5/1/2023		
				HydroCAD Software Solutions LLC Page 10		
<u></u>	<u></u>					
Volume	Invert	Avail.Stor	rage	Storage Description		
#1	669.75'	4	17 cf	12.0" Round Pipe Storage Inside #3		
				L= 60.0'		
#2	669.75'	1.12	23 cf	24.00'W x 39.00'L x 3.00'H Soil Media and Gravel-Impervious		
		,	-	2,808 cf Overall x 40.0% Voids		
#3	669.75'	1.10)4 cf	24.00'W x 39.00'L x 3.00'H Soil Media and GravelImpervious		
		, -		2,808 cf Overall - 47 cf Embedded = 2,761 cf x 40.0% Voids		
#4	672.75'	23	34 cf			
				468 cf Overall x 50.0% Voids		
#5	673.00'	4,60)3 cf	24.00'W x 78.00'L x 2.00'H Ponding Z=2.0		
				Total Available Storage		
		-,-				
Device	Routing	Invert	Outl	et Devices		
#1	Discarded	672.75'	2.41	0 in/hr Exfiltration over Wetted area		
#2	Primary	671.75'		" Round Culvert		
	j		-	'5.0' CPP, square edge headwall, Ke= 0.500		
				/ Outlet Invert= 671.75' / 668.00' S= 0.0500 '/' Cc= 0.900		
				0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf		
#3	Device 2	671.75'		Vert. Orifice/Grate X 3.00 C= 0.600		
			-	ted to weir flow at low heads		
#4	Device 2	674.25'	12.0	" Horiz. Orifice/Grate X 3.00 C= 0.600		
			Limi	ted to weir flow at low heads		
#5	Secondary	674.50'	10.0	' long x 10.0' breadth Broad-Crested Rectangular Weir		
	,			d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60		
				f. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64		
Discard	Discarded OutFlow Max=0.22 cfs @ 12.55 hrs HW=673.22' (Free Discharge)					

1=Exfiltration (Exfiltration Controls 0.22 cfs)

Primary OutFlow Max=0.37 cfs @ 12.55 hrs HW=673.22' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 0.37 cfs of 3.72 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.37 cfs @ 5.66 fps) -4=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=669.75' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG1: Rain Garden 1

Inflow Area	a =	7,720 sf	, 65.00% Impervious,	Inflow Depth >	2.20"	for 2YearMass event
Inflow	=	0.47 cfs @	12.07 hrs, Volume=	1,418 c	f	
Outflow	=	0.21 cfs @	12.24 hrs, Volume=	1,406 c	f, Atter	n= 55%, Lag= 9.9 min
Primary	=	0.21 cfs @	12.24 hrs, Volume=	1,406 c	f	
Routed	to Pond	d DMH1 : Mar	hole			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.22' @ 12.24 hrs Surf.Area= 300 sf Storage= 325 cf

Plug-Flow detention time= 37.1 min calculated for 1,406 cf (99% of inflow) Center-of-Mass det. time= 32.0 min (837.0 - 805.0)

Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24"

Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 11

Volume	Invert	Avail.Storage	Storage Description
#1	671.75'	47 cf	12.0" Round Pipe Storage Inside #2 L= 60.0'
#2	671.75'	341 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel 900 cf Overall - 47 cf Embedded = 853 cf x 40.0% Voids
#3	674.75'	38 cf	5.00'W x 60.00'L x 0.25'H Mulch 75 cf Overall x 50.0% Voids
#4	675.00'	435 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		861 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	L= 6 Inle	' Round Culvert 6.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 671.75' / 671.60' S= 0.0250 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	675.50' 6.0 '	Horiz. Orifice/Grate C= 0.600
#3 #4	Device 1 Device 1	671.75' 2.0 '	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.21 cfs @ 12.24 hrs HW=674.22' TW=673.07' (Dynamic Tailwater)

1=Culvert (Passes 0.21 cfs of 1.01 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs) -3=Orifice/Grate (Orifice Controls 0.11 cfs @ 5.17 fps)

-4=Orifice/Grate (Orifice Controls 0.10 cfs @ 1.59 fps)

Summary for Pond RG2: Rain Garden 2

Inflow Are	a =	6,820 sf, 65.00% Impervious,	Inflow Depth > 2.20" for 2YearMass event			
Inflow	=	0.41 cfs @ 12.07 hrs, Volume=	1,253 cf			
Outflow	=	0.14 cfs @ 12.34 hrs, Volume=	1,245 cf, Atten= 65%, Lag= 16.0 min			
Primary	=	0.14 cfs @ 12.34 hrs, Volume=	1,245 cf			
Routed to Pond DMH2 : Manhole						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 675.73' @ 12.34 hrs Surf.Area= 330 sf Storage= 288 cf

Plug-Flow detention time= 23.1 min calculated for 1,245 cf (99% of inflow) Center-of-Mass det. time= 19.6 min (824.5 - 805.0)

Volume	Invert	Avail.Storage	Storage Description
#1	673.75'	43 cf	12.0" Round Pipe Storage Inside #2
			L= 55.0'
#2	673.75'	379 cf	6.00'W x 55.00'L x 3.00'H Soil Media and Gravel
			990 cf Overall - 43 cf Embedded = 947 cf x 40.0% Voids
#3	676.75'	41 cf	6.00'W x 55.00'L x 0.25'H Mulch
			83 cf Overall x 50.0% Voids
#4	677.00'	457 cf	6.00'W x 55.00'L x 1.00'H Ponding Z=2.0
		921 cf	Total Available Storage

AP5&6

AP5&6

Postdevelopment "Type III 24-hr 2YearMass Rainfall=3.24"

> Printed 5/1/2023 Page 12

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	673.75'	6.0" Round Culvert
	•		L= 6.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 673.75' / 673.60' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	677.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	673.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	676.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.14 cfs @ 12.34 hrs HW=675.73' TW=673.79' (Dynamic Tailwater)

1=Culvert (Passes 0.14 cfs of 1.25 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.64 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond RG3: Rain Garden 3

Inflow Are	a =	6,780 sf,	57.81% Impervious,	Inflow Depth >	2.03"	for 2YearMass event
Inflow	=	0.38 cfs @	12.07 hrs, Volume=	1,148 c	f	
Outflow	=	0.18 cfs @	12.23 hrs, Volume=	1,143 c	f, Atter	n= 54%, Lag= 9.6 min
Primary	=	0.18 cfs @	12.23 hrs, Volume=	1,143 c	f	-
Routed to Pond DMH2 : Manhole						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 678.09' @ 12.23 hrs Surf.Area= 210 sf Storage= 213 cf

Plug-Flow detention time= 15.1 min calculated for 1,143 cf (100% of inflow) Center-of-Mass det. time= 12.7 min (825.7 - 813.1)

Volume	Invert	Avail.Storage	Storage Description		
#1	675.75'	27 cf	12.0" Round Pipe Storage Inside #2 L= 35.0'		
#2	675.75'	241 cf	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel 630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids		
#3	678.75'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch 53 cf Overall x 50.0% Voids		
#4	679.00'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0		
		592 cf	Total Available Storage		
Device	Routing	Invert Out	let Devices		
#1	Primary	L= 7 Inle	' Round Culvert 76.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 675.75' / 673.60' S= 0.0283 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	679.50' 6.0'	6.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#3 #4	Device 1 Device 1	675.75' 2.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		

Primary OutFlow Max=0.18 cfs @ 12.23 hrs HW=678.09' TW=673.79' (Dynamic Tailwater)

1=Culvert (Passes 0.18 cfs of 1.15 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.16 cfs @ 7.23 fps)

-4=Orifice/Grate (Orifice Controls 0.02 cfs @ 1.03 fps)

Summary for Pond RG69: Rain Garden 69

3,630 sf, 65.00% Impervious, Inflow Depth > 2.20" for 2YearMass event Inflow Area = Inflow = 0.22 cfs @ 12.07 hrs, Volume= 667 cf = 0.07 cfs @ 12.38 hrs, Volume= 667 cf, Atten= 69%, Lag= 18.5 min Outflow Discarded = 0.01 cfs @ 11.45 hrs, Volume= 543 cf Primary = 0.06 cfs @ 12.38 hrs, Volume= 124 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 677.63' @ 12.38 hrs Surf.Area= 210 sf Storage= 216 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 118.2 min (923.2 - 805.0)

Volume	Invert	Avail.Storage	Storage Description
#1	675.25'	27 cf	12.0" Round Pipe Storage Inside #2
		044.5	L= 35.0'
#2	675.25'	241 CT	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel 630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids
#3	678.25'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch
110	01 0120	20 0.	53 cf Overall x 50.0% Voids
#4	678.50'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	675.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	677.25'	6.0" Round Culvert
			L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 677.25' / 677.00' S= 0.0096 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	679.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2	677.25'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	677.75'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 11.45 hrs HW=675.29' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.06 cfs @ 12.38 hrs HW=677.63' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 0.06 cfs of 0.29 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.06 cfs @ 2.61 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond RG70: Rain Garden 70

5,415 sf, 65.00% Impervious, Inflow Depth > 2.20" for 2YearMass event Inflow Area = 0.33 cfs @ 12.07 hrs, Volume= Inflow = 994 cf 0.24 cfs @ 12.14 hrs, Volume= 992 cf, Atten= 28%, Lag= 4.3 min Outflow = Discarded = 0.01 cfs @ 10.97 hrs, Volume= 631 cf Primary = 0.23 cfs @ 12.14 hrs, Volume= 361 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.03' @ 12.14 hrs Surf.Area= 210 sf Storage= 250 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 100.2 min (905.2 - 805.0)

Volume	Invert	Avail.Storage	Storage Description
#1	671.25'	27 cf	12.0" Round Pipe Storage Inside #2
			L= 35.0'
#2	671.25'	241 cf	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel
			630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids
#3	674.25'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch
			53 cf Overall x 50.0% Voids
#4	674.50'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	671.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	673.25'	6.0" Round Culvert
			L= 20.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 673.25' / 673.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	675.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2		2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	673.75'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 10.97 hrs HW=671.29' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.22 cfs @ 12.14 hrs HW=674.02' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 0.22 cfs of 0.65 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.09 cfs @ 4.00 fps)

-5=Orifice/Grate (Orifice Controls 0.14 cfs @ 1.78 fps)

Summary for Pond TD1: Trench Drain

Outflow = Primary =	, , , , , , ,						
Pouting by Dy	n Stor Ind method	Time Span= 0.00-24.00 hrs, dt= 0.01 hrs					
		Surf.Area= 30 sf Storage= 20 cf					
	0.77 @ 12.001113						
•	ention time= 0.8 mir s det. time= 0.5 mir	n calculated for 3,955 cf (100% of inflow) n(805.5-805.0)					
Volume	Invert Avail.Sto	orage Storage Description					
#1 6	73.10'	60 cf 1.50'W x 20.00'L x 2.00'H Prismatoid					
Device Rout	ing Invert	Outlet Devices					
#1 Prim	ary 673.10'						
		L= 2.0' CPP, square edge headwall, Ke= 0.500					
		Inlet / Outlet Invert= 673.10' / 673.00' S= 0.0500 '/' Cc= 0.900					
		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf					
Primary OutFlow Max=1 31 cfs @ 12.08 hrs. HW=673.77' TW=672.48' (Dynamic Tailwater)							

Primary OutFlow Max=1.31 cfs @ 12.08 hrs HW=673.77' TW=672.48' (Dynamic Tailwater) -1=Culvert (Barrel Controls 1.31 cfs @ 3.32 fps)

Summary for Link AP5: AP5 - To Wetland A (A23-A32) / VP A1

Inflow Area =	180,650 sf, 19.26% Impervious,	Inflow Depth > 0.97"	for 2YearMass event
Inflow =	2.77 cfs @ 12.15 hrs, Volume=	14,603 cf	
Primary =	2.77 cfs @ 12.15 hrs, Volume=	14,603 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link AP6: AP6 - To Wetland B (off-site)

Inflow Area =	111,800 sf,	0.00% Impervious,	Inflow Depth > 0.7	0" for 2YearMass event
Inflow =	1.37 cfs @ 1	12.16 hrs, Volume=	6,530 cf	
Primary =	1.37 cfs @ 1	12.16 hrs, Volume=	6,530 cf, A	tten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P5.1: To Wetland A (A23-A32) / VP A1

Runoff = 4.88 cfs @ 12.13 hrs, Volume= 16,958 cf, Depth> 2.40" Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN E	escription		
	34,145	70 V	Voods, Go	od, HSG C	
	38,725	77 V	Voods, Go	od, HSG D	
	2,390	80 >	75% Gras	s cover, Go	ood, HSG D
	8,015	74 >	75% Gras	s cover, Go	ood, HSG C
	1,535	98 L	Inconnecte	ed roofs, HS	SG C
	84,810	74 V	Veighted A	verage	
	83,275	g	8.19% Per	vious Area	
	1,535	1	.81% Impe	ervious Area	а
	1,535	1	00.00% Üı	nconnected	1
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.5	50	0.0750	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.8	150	0.0750	1.37		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
9.3	200	Total			

Summary for Subcatchment P5.2: Upgradient Lots 6-8

Runoff = 0.48 cfs @ 12.13 hrs, Volume= 1,892 cf, Depth> 1.06" Routed to Pond IT-5/8 : Interceptor Trench

Area (sf)	CN	Description			
300	61	>75% Grass cover, Good, HSG B			
485	74	>75% Grass cover, Good, HSG C			
19,455	55	Woods, Good, HSG B			
1,085	70	Woods, Good, HSG C			
21,325	56	Weighted Average			
21,325		100.00% Pervious Area			

	d by Mc		gineeering 362 © 202		Postdevelopment <i>Type III 24-hr 10YearMass Rainfall=5.05"</i> Printed 5/1/2023 <u>D Software Solutions LLC</u> Page 17		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.7	50	0.1000	0.12		Sheet Flow,		
0.9	85	0.1000	1.58		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps		
0.0	5	0.1000	2.21		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
7.6	140	Total					
		;	Summar	y for Sub	ocatchment P5.3: To RG5.1		
Runoff Route	= ed to Pon		s @ 12.0 rench Dra	7 hrs, Volu in	Ime= 7,041 cf, Depth> 3.92"		
			nod, UH=S s Rainfall=		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs		
А	rea (sf)	CN D	escription				
	21,545			s, 65% imp	, HSG C		
	7,541 14,004			vious Area pervious Are			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		
	Summary for Subcatchment P5.4: Lot 4						
Runoff Route	= ed to Pon		s @ 12.0 Rain Garde	7 hrs, Volu en 3	Ime= 2,098 cf, Depth> 3.71"		
	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"						
А	rea (sf)	CN D	escription				
	6,030			s, 65% imp	, HSG C		
	750			od, HSG Ċ			
	6,780		Veighted A				
	2,861 3,920			vious Area pervious Are			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

Summary for Subcatchment P5.5: Lot 69

Runoff = 0.38 cfs @ 12.07 hrs, Volume= 1,186 cf, Depth> 3.92" Routed to Pond RG69 : Rain Garden 69

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN	Description					
	3,630	90	1/8 acre lot	s, 65% imp	o, HSG C			
	1,271	;	35.00% Pervious Area					
	2,360		65.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P5.6: Upgradient of Amphibian Crossing

2,762 cf, Depth> 1.47"

Runoff = 0.76 cfs @ 12.13 hrs, Volume= Routed to Reach AC : Amphibian Crossing

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN [Description		
	1,860			,	ood, HSG C
	8,330	70 \	Voods, Go	od, HSG C	
	12,415	55 \	Voods, Go	od, HSG B	
	22,605	62 \	Veighted A	verage	
	22,605			ervious Are	а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
6.7	50	0.1000	0.12		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.6	150	0.1000	1.58		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.1	30	0.3000	3.83		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
8.4	230	Total			

Summary for Subcatchment P5.7: Lot 70

Runoff = 0.57 cfs @ 12.07 hrs, Volume= 1,770 cf, Depth> 3.92" Routed to Pond RG70 : Rain Garden 70

A	rea (sf)	CN I	Description						
	5,415	90 [·]	1/8 acre lots, 65% imp, HSG C						
	1,895 3,520	35.00% Pervious Area 65.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				

5.0

Direct Entry,

Summary for Subcatchment P5.8: Lot 3

0.72 cfs @ 12.07 hrs, Volume= 2,229 cf, Depth> 3.92" Runoff = Routed to Pond RG2 : Rain Garden 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Α	rea (sf)	CN [Description						
	6,820	90 î	1/8 acre lots, 65% imp, HSG C						
	2,387	3	35.00% Pervious Area						
	4,433	6	65.00% Impervious Area						
_		<u>.</u>		• •	-				
Tc	Length	Slope		Capacity					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry,				

Summary for Subcatchment P5.9: Lot 2

Runoff	=	0.81 cfs @	12.07 hrs,	Volume=	2,523 cf,	Depth>	3.92"
Routed	to Pond	d RG1 : Rain (Garden 1			•	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Α	rea (sf)	CN E	Description						
	7,720	90 1	1/8 acre lots, 65% imp, HSG C						
	2,702	3	35.00% Pervious Area						
	5,018	6	65.00% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry,				

AP5&6

Summary for Subcatchment P6.1: To Wetland B (off-site)

Runoff = 3.43 cfs @ 12.13 hrs, Volume= 12,062 cf, Depth> 2.07" Routed to Link AP6 : AP6 - To Wetland B (off-site)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN [Description					
	6,465	74 >75% Grass cover, Good, HSG C						
	1,875	55 V	Voods, Go	od, HSG B				
	61,615	70 V	Voods, Go	od, HSG C				
	69,955	70 V	Veighted A	verage				
	69,955	1	00.00% Pe	ervious Are	a			
Тс	Length	Slope		Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.5	50	0.1600	0.15		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
0.3	45	0.2200	2.35		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
3.4	290	0.0800	1.41		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
9.2	385	Total						

Summary for Subcatchment P6.2: Upgradient Lots 1-5

Runoff = 1.21 cfs @ 12.17 hrs, Volume= 4,864 cf, Depth> 1.39" Routed to Pond IT-1/5 : Interceptor Trench

A	rea (sf)	CN E	escription					
	2,130	74 >	75% Grass	s cover, Go	bod, HSG C			
	24,405		Woods, Good, HSG B					
	15,310	70 V	70 Woods, Good, HSG C					
	41,845 61 Weighted Average							
	41,845 100.00% Pervious Area				а			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.2	50	0.0600	0.10		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
2.7	160	0.0400	1.00		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
10.9	210	Total						

Summary for Reach AC: Amphibian Crossing

Inflow Area = 22,605 sf, 0.00% Impervious, Inflow Depth > 1.47" for 10YearMass event Inflow = 0.76 cfs @ 12.13 hrs, Volume= 2,762 cf Outflow = 0.75 cfs @ 12.14 hrs, Volume= 2,758 cf, Atten= 1%, Lag= 0.7 min Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Max. Velocity= 0.48 fps, Min. Travel Time= 1.0 min Avg. Velocity = 0.16 fps, Avg. Travel Time= 3.0 min

Peak Storage= 47 cf @ 12.14 hrs Average Depth at Peak Storage= 0.26', Surface Width= 6.00' Bank-Full Depth= 3.50' Flow Area= 21.0 sf, Capacity= 35.65 cfs

6.00' x 3.50' deep channel, n= 0.022 Earth, clean & straight Length= 30.0' Slope= 0.0003 '/' Inlet Invert= 675.83', Outlet Invert= 675.82'



Summary for Pond DMH1: Manhole

 Inflow Area =
 21,320 sf, 62.71% Impervious, Inflow Depth > 3.84" for 10YearMass event

 Inflow =
 1.56 cfs @
 12.14 hrs, Volume=
 6,820 cf

 Outflow =
 1.56 cfs @
 12.14 hrs, Volume=
 6,820 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.56 cfs @
 12.14 hrs, Volume=
 6,820 cf

 Routed to Pond RG-5.1 : Rain Garden 5.1
 6,820 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.36' @ 12.41 hrs Flood Elev= 675.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	671.50'	12.0" Round Culvert L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= $671.50' / 671.00'$ S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.36 cfs @ 12.14 hrs HW=673.90' TW=673.76' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 1.36 cfs @ 1.74 fps)

		Postdevelopment
AP5&6	Type III 24-hr	10YearMass Rainfall=5.05"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	olutions LLC	Page 22

Summary for Pond DMH2: Manhole

Inflow Area = 13,600 sf, 61.42% Impervious, Inflow Depth > 3.80" for 10YearMass event Inflow 1.02 cfs @ 12.14 hrs, Volume= = 4.312 cf 1.02 cfs @ 12.14 hrs, Volume= 4,312 cf, Atten= 0%, Lag= 0.0 min Outflow = Primary = 1.02 cfs @ 12.14 hrs, Volume= 4,312 cf Routed to Pond DMH1 : Manhole Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.39' @ 12.40 hrs Flood Elev= 678.20' Device Routing Invert Outlet Devices Primary #1 673.50' 12.0" Round Culvert L= 80.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 673.50' / 671.60' S= 0.0237 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.95 cfs @ 12.14 hrs HW=674.21' TW=673.90' (Dynamic Tailwater) -1=Culvert (Outlet Controls 0.95 cfs @ 2.25 fps)

Summary for Pond IT-1/5: Interceptor Trench

 Inflow Area =
 41,845 sf, 0.00% Impervious, Inflow Depth > 1.39" for 10YearMass event

 Inflow =
 1.21 cfs @ 12.17 hrs, Volume=
 4,864 cf

 Outflow =
 1.10 cfs @ 12.22 hrs, Volume=
 4,835 cf, Atten= 9%, Lag= 3.5 min

 Primary =
 1.10 cfs @ 12.22 hrs, Volume=
 4,835 cf

 Routed to Link AP6 : AP6 - To Wetland B (off-site)
 4,835 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 677.76' @ 12.22 hrs Surf.Area= 750 sf Storage= 280 cf

Plug-Flow detention time= 8.8 min calculated for 4,833 cf (99% of inflow) Center-of-Mass det. time= 5.4 min (879.8 - 874.4)

Volume	Invert	Avail.Storag	e Storage Description	
#1	677.00'	865 (of 3.00'W x 250.00'L x 3.00'H Prismatoid	
#2	677.00'	87 0	2,250 cf Overall - 87 cf Embedded = 2,163 cf x 40.0% Voids f 8.0" Round Pipe Storage Inside #1 L= 250.0'	
		952 (of Total Available Storage	
Device	Routing	Invert O	utlet Devices	
#1	Primary	L: In	0" Round Culvert = 220.0' CPP, square edge headwall, Ke= 0.500 let / Outlet Invert= 677.00' / 668.00' S= 0.0409 '/' Cc= 0.900 = 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf	
Primary OutFlow Max=1.10 cfs @ 12.22 hrs HW=677.76' TW=0.00' (Dynamic Tailwater)				

1=Culvert (Inlet Controls 1.10 cfs @ 3.14 fps)

Summary for Pond IT-5/8: Interceptor Trench

Inflow Area = 21,325 sf, 0.00% Impervious, Inflow Depth > 1.06" for 10YearMass event 0.48 cfs @ 12.13 hrs, Volume= Inflow = 1.892 cf 0.43 cfs @ 12.18 hrs, Volume= 1,880 cf, Atten= 10%, Lag= 2.9 min Outflow = Primary = 0.43 cfs @ 12.18 hrs, Volume= 1.880 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1 Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 680.45' @ 12.18 hrs Surf.Area= 465 sf Storage= 102 cf Plug-Flow detention time= 8.9 min calculated for 1,879 cf (99% of inflow) Center-of-Mass det. time= 5.3 min (893.4 - 888.2) Volume Invert Avail.Storage Storage Description #1 680.00' 546 cf 3.00'W x 155.00'L x 3.00'H Prismatoid 1,395 cf Overall - 30 cf Embedded = 1,365 cf x 40.0% Voids #2 680.00' 6.0" Round Pipe Storage Inside #1 30 cf L= 155.0' 576 cf Total Available Storage Device Routing **Outlet Devices** Invert #1 680.00' 6.0" Round Culvert Primary L= 110.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 680.00' / 678.00' S= 0.0182 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf Primary OutFlow Max=0.43 cfs @ 12.18 hrs HW=680.45' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.43 cfs @ 2.29 fps)

Summary for Pond RG-5.1: Rain Garden 5.1

Inflow Area = 42,865 sf, 63.86% Impervious, Inflow Depth > 3.88" for 10YearMass event Inflow 3.66 cfs @ 12.09 hrs, Volume= 13.858 cf = Outflow 1.28 cfs @ 12.43 hrs, Volume= 12,258 cf, Atten= 65%, Lag= 20.5 min = 0.25 cfs @ 12.43 hrs, Volume= Discarded = 2,773 cf Primary = 1.03 cfs @ 12.43 hrs, Volume= 9,485 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Link AP6 : AP6 - To Wetland B (off-site)

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.32' @ 12.43 hrs Surf.Area= 4,309 sf Storage= 5,340 cf

Plug-Flow detention time= 128.7 min calculated for 12,258 cf (88% of inflow) Center-of-Mass det. time= 76.3 min (877.5 - 801.2)

Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05"

Printed 5/1/2023

		Engineeering		
HydroCA	D® 10.20-2g s/	/n 03362 © 2022	2 HydroCAD Software Solutions LLC Page 24	
Volume	Invert	Avail.Storage	e Storage Description	
#1	669.75'	47 c	f 12.0" Round Pipe Storage Inside #3 L= 60.0'	
#2	669.75'	1,123 c	f 24.00'W x 39.00'L x 3.00'H Soil Media and Gravel Impervious 2,808 cf Overall x 40.0% Voids	
#3	669.75'	1,104 c		
#4	672.75'	234 c		
#5	673.00'	4,603 c	f 24.00'W x 78.00'L x 2.00'H Ponding Z=2.0	
		7,111 c	f Total Available Storage	
Device	Routing		utlet Devices	
#1 #2	Discarded Primary	671.75' 12 L= In	410 in/hr Exfiltration over Wetted area 2.0" Round Culvert = 75.0' CPP, square edge headwall, Ke= 0.500 let / Outlet Invert= 671.75' / 668.00' S= 0.0500 '/' Cc= 0.900 = 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	
#3	Device 2		0" Vert. Orifice/Grate X 3.00 C= 0.600 mited to weir flow at low heads	
#4	Device 2			
#5	Secondary	674.50' 10.0' long x 10.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64		
		/lax=0.25 cfs @ iltration Control	12.43 hrs HW=674.32' (Free Discharge) s 0.25 cfs)	
¹ —2=Cu 1—3=	Ivert (Passes Orifice/Grate	1.03 cfs of 5.4 (Orifice Contro	2.43 hrs HW=674.32' TW=0.00' (Dynamic Tailwater) 4 cfs potential flow) Is 0.50 cfs @ 7.59 fps) 0.53 cfs @ 0.85 fps)	

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=669.75' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG1: Rain Garden 1

Inflow Area =		7,720 sf, 65.00% Impervious, Inflow Depth >	3.92" for 10YearMass event
Inflow	=	0.81 cfs @ 12.07 hrs, Volume= 2,523 cf	
Outflow	=	0.53 cfs @ 12.14 hrs, Volume= 2,508 cf	, Atten= 35%, Lag= 3.8 min
Primary	=	0.53 cfs @ 12.14 hrs, Volume= 2,508 cf	
Routed	I to Pond	I DMH1 : Manhole	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 675.14' @ 12.16 hrs Surf.Area= 936 sf Storage= 469 cf

Plug-Flow detention time= 37.5 min calculated for 2,508 cf (99% of inflow) Center-of-Mass det. time= 33.6 min (822.6 - 789.0)

AP5&6

Postdevelopment

Type III 24-hr 10YearMass Rainfall=5.05"

Prepared	by Mcclure	Engineeering	Printed 5/1/2023
HydroCAD	® 10.20-2g s	/n 03362 © 2022 H	HydroCAD Software Solutions LLC Page 25
Volume	Invert	Avail.Storage	Storage Description
#1	671.75'	47 cf	12.0" Round Pipe Storage Inside #2 L= 60.0'
#2	671.75'	341 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel

			900 cf Overall - 47 cf Embedded = 853 cf x 40.0% Voids
#3	674.75'	38 cf	5.00'W x 60.00'L x 0.25'H Mulch
			75 cf Overall x 50.0% Voids
#4	675.00'	435 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		861 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	671.75'	6.0" Round Culvert
			L= 6.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 671.75' / 671.60' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	675.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	671.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	674.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.53 cfs @ 12.14 hrs HW=675.13' TW=673.89' (Dynamic Tailwater)

1=Culvert (Passes 0.53 cfs of 1.05 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs) **3=Orifice/Grate** (Orifice Controls 0.12 cfs @ 5.36 fps)

-4=Orifice/Grate (Orifice Controls 0.41 cfs @ 4.72 fps)

Summary for Pond RG2: Rain Garden 2

Inflow Are	a =	6,820 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event		
Inflow	=	0.72 cfs @ 12.07 hrs, Volume=	2,229 cf		
Outflow	=	0.48 cfs @ 12.15 hrs, Volume=	2,219 cf, Atten= 33%, Lag= 4.7 min		
Primary	=	0.48 cfs @ 12.15 hrs, Volume=	2,219 cf		
Routed to Pond DMH2 : Manhole					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 676.72' @ 12.15 hrs Surf.Area= 330 sf Storage= 418 cf

Plug-Flow detention time= 21.8 min calculated for 2,219 cf (100% of inflow) Center-of-Mass det. time= 19.2 min (808.1 - 789.0)

Volume	Invert	Avail.Storage	Storage Description
#1	673.75'	43 cf	12.0" Round Pipe Storage Inside #2
			L= 55.0'
#2	673.75'	379 cf	6.00'W x 55.00'L x 3.00'H Soil Media and Gravel
			990 cf Overall - 43 cf Embedded = 947 cf x 40.0% Voids
#3	676.75'	41 cf	6.00'W x 55.00'L x 0.25'H Mulch
			83 cf Overall x 50.0% Voids
#4	677.00'	457 cf	6.00'W x 55.00'L x 1.00'H Ponding Z=2.0
		921 cf	Total Available Storage

AP5&6

AP5&6

Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05"

Printed 5/1/2023

Page 26

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	673.75'	6.0" Round Culvert
	-		L= 6.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 673.75' / 673.60' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	677.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	673.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	676.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.48 cfs @ 12.15 hrs HW=676.72' TW=674.23' (Dynamic Tailwater)

1=Culvert (Passes 0.48 cfs of 1.49 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.17 cfs @ 7.60 fps)

-4=Orifice/Grate (Orifice Controls 0.31 cfs @ 3.58 fps)

Summary for Pond RG3: Rain Garden 3

Inflow Area =		6,780 sf, 57.81% Impervious,	Inflow Depth > 3.71" for 10YearMass event		
Inflow	=	0.69 cfs @ 12.07 hrs, Volume=	2,098 cf		
Outflow	=	0.55 cfs @ 12.13 hrs, Volume=	2,092 cf, Atten= 20%, Lag= 3.4 min		
Primary	=	0.55 cfs @ 12.13 hrs, Volume=	2,092 cf		
Routed to Pond DMH2 : Manhole					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 678.92' @ 12.13 hrs Surf.Area= 420 sf Storage= 286 cf

Plug-Flow detention time= 12.7 min calculated for 2,091 cf (100% of inflow) Center-of-Mass det. time= 10.9 min (807.0 - 796.1)

Volume	Invert	Avail.Storage Storage Description			
#1	675.75'	27 cf	12.0" Round Pipe Storage Inside #2		
#2	675.75'	241 cf	L= 35.0' 6.00'W x 35.00'L x 3.00'H Soil Media and Gravel 630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids		
#3	678.75'	26 cf			
#4	679.00'	297 cf 6.00'W x 35.00'L x 1.00'H Ponding Z=2.0			
		592 cf	Total Available Storage		
Device	Routing	Invert Out	let Devices		
#1	Primary	L= I Inle	' Round Culvert 76.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 675.75' / 673.60' S= 0.0283 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	679.50' 6.0'	'Horiz. Orifice/Grate C= 0.600 ited to weir flow at low heads		
#3 #4	Device 1 Device 1	675.75' 2.0'	'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads 'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		

Primary OutFlow Max=0.55 cfs @ 12.13 hrs HW=678.92' TW=674.19' (Dynamic Tailwater) **1=Culvert** (Passes 0.55 cfs of 1.25 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.18 cfs @ 8.46 fps)

4=Orifice/Grate (Orifice Controls 0.36 cfs @ 4.17 fps)

Summary for Pond RG69: Rain Garden 69

3,630 sf, 65.00% Impervious, Inflow Depth > 3.92" for 10YearMass event Inflow Area = Inflow = 0.38 cfs @ 12.07 hrs, Volume= 1,186 cf 0.33 cfs @ 12.12 hrs, Volume= 1,169 cf, Atten= 14%, Lag= 2.7 min Outflow = Discarded = 0.01 cfs @ 10.38 hrs, Volume= 675 cf Primary = 0.32 cfs @ 12.12 hrs, Volume= 494 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 678.19' @ 12.12 hrs Surf.Area= 210 sf Storage= 264 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 82.9 min (871.9 - 789.0)

Volume	Invert	Avail.Storage	Storage Description
#1	675.25'	27 cf	12.0" Round Pipe Storage Inside #2
			L= 35.0'
#2	675.25'	241 cf	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel
			630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids
#3	678.25'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch
	070 501	007 (53 cf Overall x 50.0% Voids
#4	678.50'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	675.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	677.25'	6.0" Round Culvert
			L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 677.25' / 677.00' S= 0.0096 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	679.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2		2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	677.75'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.01 cfs @ 10.38 hrs HW=675.29' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.32 cfs @ 12.12 hrs HW=678.19' TW=0.00' (Dynamic Tailwater)

- -2=Culvert (Passes 0.32 cfs of 0.69 cfs potential flow)
- **3=Orifice/Grate** (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.10 cfs @ 4.46 fps)

-5=Orifice/Grate (Orifice Controls 0.22 cfs @ 2.52 fps)

Summary for Pond RG70: Rain Garden 70

Inflow Area = 5,415 sf, 65.00% Impervious, Inflow Depth > 3.92" for 10YearMass event 0.57 cfs @ 12.07 hrs, Volume= Inflow = 1.770 cf 0.48 cfs @ 12.12 hrs, Volume= 1,680 cf, Atten= 16%, Lag= 2.9 min Outflow = Discarded = 0.04 cfs @ 12.12 hrs, Volume= 714 cf Primary = 0.44 cfs @ 12.12 hrs, Volume= 966 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.53' @ 12.12 hrs Surf.Area= 635 sf Storage= 301 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 45.4 min (834.4 - 789.0)

Volume	Invert	Avail.Storage	Storage Description
#1	671.25'	27 cf	12.0" Round Pipe Storage Inside #2
			L= 35.0'
#2	671.25'	241 cf	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel
			630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids
#3	674.25'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch
			53 cf Overall x 50.0% Voids
#4	674.50'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	671.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	673.25'	6.0" Round Culvert
			L= 20.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 673.25' / 673.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	675.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2		2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	673.75'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.12 hrs HW=674.53' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.44 cfs @ 12.12 hrs HW=674.53' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 0.44 cfs of 0.91 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.11 cfs @ 5.27 fps)

-5=Orifice/Grate (Orifice Controls 0.33 cfs @ 3.77 fps)

		Postdevelopment
AP5&6	Type III 24-hr	10YearMass Rainfall=5.05"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Sol	utions LLC	Page 29

Summary for Pond TD1: Trench Drain

Inflow A	rea =	21,545 sf, 65.00% Impervious, Inflow Depth > 3.92" for 10YearMass event				
Inflow	=	2.27 cfs @ 12.07 hrs, Volume= 7,041 cf				
Outflow	=	2.27 cfs @ 12.07 hrs, Volume= 7,039 cf, Atten= 0%, Lag= 0.1 min				
Primary		2.27 cfs @ 12.07 hrs, Volume= 7,039 cf				
		RG-5.1 : Rain Garden 5.1				
Routing	by Dyn-Sto	r-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs				
Peak Ele	ev= 674.34'	@ 12.42 hrs Surf.Area= 30 sf Storage= 37 cf				
•		time= 1.0 min calculated for 7,039 cf (100% of inflow)				
Center-o	of-Mass det.	. time= 0.8 min(789.8 - 789.0)				
Valuma	lover	t Avail Starage Starage Description				
Volume	Inver					
#1	673.10	60 cf 1.50'W x 20.00'L x 2.00'H Prismatoid				
Device	Routing	Invert Outlet Devices				
#1	Primary	673.10' 12.0" Round Culvert				
		L= 2.0' CPP, square edge headwall, Ke= 0.500				
		Inlet / Outlet Invert= 673.10' / 673.00' S= 0.0500 '/' Cc= 0.900				
		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf				
Primary OutFlow Max = 2.27 cfs @ 12.07 brs $HW=674.06'$ TW=673.44' (Dynamic Tailwater)						

Primary OutFlow Max=2.27 cfs @ 12.07 hrs HW=674.06' TW=673.44' (Dynamic Tailwater) -1=Culvert (Barrel Controls 2.27 cfs @ 3.76 fps)

Summary for Link AP5: AP5 - To Wetland A (A23-A32) / VP A1

Inflow Area =	180,650 sf, 19.26% Impervious,	Inflow Depth > 2.16"	for 10YearMass event
Inflow =	7.22 cfs @ 12.14 hrs, Volume=	32,541 cf	
Primary =	7.22 cfs @ 12.14 hrs, Volume=	32,541 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link AP6: AP6 - To Wetland B (off-site)

Inflow Area =	111,800 sf, 0.00% Impervious,	Inflow Depth > 1.81"	for 10YearMass event
Inflow =	4.34 cfs @ 12.15 hrs, Volume=		
Primary =	4.34 cfs @ 12.15 hrs, Volume=	16,896 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P5.1: To Wetland A (A23-A32) / VP A1

Runoff = 6.80 cfs @ 12.13 hrs, Volume= 23,539 cf, Depth> 3.33" Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN E	escription		
	34,145	70 V	Voods, Go	od, HSG C	
	38,725	77 V	Voods, Go	od, HSG D	
	2,390	80 >	75% Gras	s cover, Go	ood, HSG D
	8,015	74 >	75% Gras	s cover, Go	ood, HSG C
	1,535	98 L	Inconnecte	ed roofs, HS	SG C
	84,810	74 V	Veighted A	verage	
	83,275	g	8.19% Per	vious Area	
	1,535	1	.81% Impe	ervious Area	а
	1,535	1	00.00% Üı	nconnected	1
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.5	50	0.0750	0.11		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.8	150	0.0750	1.37		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
9.3	200	Total			

Summary for Subcatchment P5.2: Upgradient Lots 6-8

Runoff	=	0.84 cfs @	12.12 hrs,	Volume=	3,021 cf,	Depth> 1.70"
Routed	I to Pond	I IT-5/8 : Inter	ceptor Tren	ch		

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Area (sf)	CN	Description
300	61	>75% Grass cover, Good, HSG B
485	74	>75% Grass cover, Good, HSG C
19,455	55	Woods, Good, HSG B
1,085	70	Woods, Good, HSG C
21,325	56	Weighted Average
21,325		100.00% Pervious Area

	d by Mc		gineeering 362 © 202		Postdevelopment <i>Type III 24-hr 25YearMass Rainfall=6.18</i> " Printed 5/1/2023 Software Solutions LLC Page 31
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.7	50	0.1000	0.12	*	Sheet Flow,
0.9	85	0.1000	1.58		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.0	5	0.1000	2.21		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
7.6	140	Total			
			Summar	y for Sub	ocatchment P5.3: To RG5.1
Runoff Route	Runoff = 2.87 cfs @ 12.07 hrs, Volume= 9,009 cf, Depth> 5.02" Routed to Pond TD1 : Trench Drain				
			nod, UH=S s Rainfall=		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
А	rea (sf)	CN E	escription		
	21,545			s, 65% imp	, HSG C
	7,541 14,004	3	5.00% Per	vious Area pervious Are	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,
			Summ	ary for S	ubcatchment P5.4: Lot 4
Runoff Route	= ed to Pon		s @ 12.0 Rain Garde	7 hrs, Volu en 3	Ime= 2,710 cf, Depth> 4.80"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"					
А	rea (sf)	CN D	escription		
	6,030			s, 65% imp	. HSG C
	750			od, HSG C	
	6,780		Veighted A		
	2,861 3,920			vious Area pervious Are	
	3,920	5	1.01% IIII		σα
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Summary for Subcatchment P5.5: Lot 69

Runoff = 0.48 cfs @ 12.07 hrs, Volume= 1,518 cf, Depth> 5.02" Routed to Pond RG69 : Rain Garden 69

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Area (sf)	CN	Description					
3,630	90	90 1/8 acre lots, 65% imp, HSG C					
1,271		35.00% Pervious Area					
2,360		65.00% Impervious Area					
Tc Length (min) (feet)	Slop (ft/1	,	Capacity (cfs)	Description			
5.0				Direct Entry,			

Summary for Subcatchment P5.6: Upgradient of Amphibian Crossing

Runoff = 1.19 cfs @ 12.13 hrs, Volume= 4,163 cf, Depth> 2.21" Routed to Reach AC : Amphibian Crossing

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN [Description		
	1,860			,	ood, HSG C
	8,330	70 \	Voods, Go	od, HSG C	
	12,415	55 \	Voods, Go	od, HSG B	
	22,605	62 \	Veighted A	verage	
	22,605	100.00% Pervious Are			а
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
6.7	50	0.1000	0.12		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
1.6	150	0.1000	1.58		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.1	30	0.3000	3.83		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
8.4	230	Total			

Summary for Subcatchment P5.7: Lot 70

Runoff = 0.72 cfs @ 12.07 hrs, Volume= 2,264 cf, Depth> 5.02" Routed to Pond RG70 : Rain Garden 70

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN D	escription					
	5,415	90 1	00 1/8 acre lots, 65% imp, HSG C					
	1,895	3	5.00% Per	vious Area				
	3,520	6	65.00% Impervious Area					
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description			
5.0		· · · ·	· · ·		Direct Entry,			

Summary for Subcatchment P5.8: Lot 3

0.91 cfs @ 12.07 hrs, Volume= 2,852 cf, Depth> 5.02" Runoff = Routed to Pond RG2 : Rain Garden 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN I	Description					
	6,820	90 ⁻	1/8 acre lots, 65% imp, HSG C					
	2,387 4,433		35.00% Pervious Area 65.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P5.9: Lot 2

Runoff	=	1.03 cfs @	12.07 hrs,	Volume=	3,228 cf,	Depth> 5.02"
Routed	to Pond	d RG1 : Rain (Garden 1			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN [Description					
	7,720	90 1	1/8 acre lots, 65% imp, HSG C					
	2,702	3	35.00% Pervious Area					
	5,018	6	65.00% Impervious Area					
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

AP5&6

Summary for Subcatchment P6.1: To Wetland B (off-site)

Runoff = 4.95 cfs @ 12.13 hrs, Volume= 17,156 cf, Depth> 2.94" Routed to Link AP6 : AP6 - To Wetland B (off-site)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN [Description		
	6,465	74 >	75% Gras	s cover, Go	ood, HSG C
	1,875	55 V	Voods, Go	od, HSG B	
	61,615	70 V	Voods, Go	od, HSG C	
	69,955	70 V	Veighted A	verage	
	69,955	1	00.00% Pe	ervious Are	a
Тс	Length	Slope		Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.5	50	0.1600	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
0.3	45	0.2200	2.35		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
3.4	290	0.0800	1.41		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
9.2	385	Total			

Summary for Subcatchment P6.2: Upgradient Lots 1-5

Runoff = 1.94 cfs @ 12.16 hrs, Volume= 7,396 cf, Depth> 2.12" Routed to Pond IT-1/5 : Interceptor Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN [Description		
	2,130	74 >	75% Gras	s cover, Go	ood, HSG C
	24,405	55 \	Voods, Go	od, HSG B	
	15,310	70 \	Voods, Go	od, HSG C	
	41,845	61 \			
	41,845	100.00% Pervious Area			a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
8.2	50	0.0600	0.10		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
2.7	160	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
10.9	210	Total			

Summary for Reach AC: Amphibian Crossing

 Inflow Area =
 22,605 sf,
 0.00% Impervious,
 Inflow Depth >
 2.21"
 for
 25YearMass event

 Inflow =
 1.19 cfs @
 12.13 hrs,
 Volume=
 4,163 cf

 Outflow =
 1.19 cfs @
 12.14 hrs,
 Volume=
 4,158 cf,
 Atten= 0%,
 Lag= 0.6 min

 Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1
 VP A1
 VP
 VP
 VP

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Max. Velocity= 0.57 fps, Min. Travel Time= 0.9 min Avg. Velocity = 0.18 fps, Avg. Travel Time= 2.8 min

Peak Storage= 63 cf @ 12.14 hrs Average Depth at Peak Storage= 0.35', Surface Width= 6.00' Bank-Full Depth= 3.50' Flow Area= 21.0 sf, Capacity= 35.65 cfs

6.00' x 3.50' deep channel, n= 0.022 Earth, clean & straight Length= 30.0' Slope= 0.0003 '/' Inlet Invert= 675.83', Outlet Invert= 675.82'



Summary for Pond DMH1: Manhole

 Inflow Area =
 21,320 sf, 62.71% Impervious, Inflow Depth > 4.93" for 25YearMass event

 Inflow =
 1.76 cfs @
 12.12 hrs, Volume=
 8,755 cf

 Outflow =
 1.76 cfs @
 12.12 hrs, Volume=
 8,755 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 1.76 cfs @
 12.12 hrs, Volume=
 8,755 cf

 Routed to Pond RG-5.1 : Rain Garden 5.1
 8.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.61' @ 12.24 hrs Flood Elev= 675.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	671.50'	12.0" Round Culvert L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 671.50' / 671.00' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.55 cfs @ 12.12 hrs HW=674.27' TW=674.09' (Dynamic Tailwater) -1=Culvert (Outlet Controls 1.55 cfs @ 1.97 fps)

	Postdevelopment
AP5&6	Type III 24-hr 25YearMass Rainfall=6.18"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC Page 36

Summary for Pond DMH2: Manhole

Inflow Area = 13,600 sf, 61.42% Impervious, Inflow Depth > 4.89" for 25YearMass event Inflow 1.21 cfs @ 12.14 hrs, Volume= = 5.544 cf 1.21 cfs @ 12.14 hrs, Volume= 5,544 cf, Atten= 0%, Lag= 0.0 min Outflow = Primary = 1.21 cfs @ 12.14 hrs, Volume= 5,544 cf Routed to Pond DMH1 : Manhole Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.73' @ 12.24 hrs Flood Elev= 678.20' Device Routing Invert Outlet Devices Primary #1 673.50' 12.0" Round Culvert L= 80.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 673.50' / 671.60' S= 0.0237 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.03 cfs @ 12.14 hrs HW=674.54' TW=674.40' (Dynamic Tailwater) -1=Culvert (Outlet Controls 1.03 cfs @ 1.58 fps)

Summary for Pond IT-1/5: Interceptor Trench

 Inflow Area =
 41,845 sf, 0.00% Impervious, Inflow Depth > 2.12" for 25YearMass event

 Inflow =
 1.94 cfs @
 12.16 hrs, Volume=
 7,396 cf

 Outflow =
 1.68 cfs @
 12.23 hrs, Volume=
 7,361 cf, Atten= 13%, Lag= 4.3 min

 Primary =
 1.68 cfs @
 12.23 hrs, Volume=
 7,361 cf

 Routed to Link AP6 : AP6 - To Wetland B (off-site)
 7,361 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 678.33' @ 12.23 hrs Surf.Area= 750 sf Storage= 452 cf

Plug-Flow detention time= 7.6 min calculated for 7,358 cf (99% of inflow) Center-of-Mass det. time= 4.9 min (866.2 - 861.3)

Volume	Invert	Avail.Stora	ge Storage Description				
#1	677.00'	865	cf 3.00'W x 250.00'L x 3.00'H Prismatoid				
#2	677.00'	87	2,250 cf Overall - 87 cf Embedded = 2,163 cf x 40.0% Voids cf 8.0" Round Pipe Storage Inside #1 L= 250.0'				
		952	cf Total Available Storage				
Device	Routing	Invert (Dutlet Devices				
#1	Primary	L	3.0" Round Culvert = 220.0' CPP, square edge headwall, Ke= 0.500 nlet / Outlet Invert= 677.00' / 668.00' S= 0.0409 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf				
Primary	Primary OutFlow Max=1.68 cfs @ 12.23 hrs HW=678.33' TW=0.00' (Dynamic Tailwater)						

1=Culvert (Inlet Controls 1.68 cfs @ 4.81 fps)

Summary for Pond IT-5/8: Interceptor Trench

Inflow Area = 21,325 sf, 0.00% Impervious, Inflow Depth > 1.70" for 25YearMass event 0.84 cfs @ 12.12 hrs, Volume= Inflow 3.021 cf = 0.72 cfs @ 12.18 hrs, Volume= 3,006 cf, Atten= 14%, Lag= 3.4 min Outflow = Primary = 0.72 cfs @ 12.18 hrs, Volume= 3.006 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1 Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 680.83' @ 12.18 hrs Surf.Area= 465 sf Storage= 173 cf Plug-Flow detention time= 7.3 min calculated for 3,006 cf (99% of inflow) Center-of-Mass det. time= 4.5 min (876.7 - 872.2) Volume Invert Avail.Storage Storage Description #1 680.00' 546 cf 3.00'W x 155.00'L x 3.00'H Prismatoid 1,395 cf Overall - 30 cf Embedded = 1,365 cf x 40.0% Voids #2 680.00' 6.0" Round Pipe Storage Inside #1 30 cf L= 155.0' 576 cf Total Available Storage Device Routing **Outlet Devices** Invert 680.00' 6.0" Round Culvert #1 Primary L= 110.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 680.00' / 678.00' S= 0.0182 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf Primary OutFlow Max=0.72 cfs @ 12.18 hrs HW=680.83' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.72 cfs @ 3.67 fps)

Summary for Pond RG-5.1: Rain Garden 5.1

Inflow Area = 42,865 sf, 63.86% Impervious, Inflow Depth > 4.97" for 25YearMass event Inflow 4.50 cfs @ 12.08 hrs, Volume= 17,761 cf = Outflow 2.75 cfs @ 12.26 hrs, Volume= 16,151 cf, Atten= 39%, Lag= 10.8 min = 0.25 cfs @ 12.26 hrs, Volume= Discarded = 3.225 cf Primary = 2.50 cfs @ 12.26 hrs, Volume= 12,926 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Link AP6 : AP6 - To Wetland B (off-site)

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.41' @ 12.26 hrs Surf.Area= 4,352 sf Storage= 5,571 cf

Plug-Flow detention time= 112.9 min calculated for 16,144 cf (91% of inflow) Center-of-Mass det. time= 68.7 min (862.8 - 794.1)

Postdevelopment ... 6.18"

Volume	Invert	Avail.Sto	rage	Storage Description
#1	669.75'		47 cf	12.0" Round Pipe Storage Inside #3
#2	669.75'	1,12	23 cf	
#3	669.75'	1 1	04 cf	2,808 cf Overall x 40.0% Voids 24.00'W x 39.00'L x 3.00'H Soil Media and Gravel Impervious
110	000.70	1,1	0101	2,808 cf Overall - 47 cf Embedded = $2,761$ cf x 40.0% Voids
#4	672.75'	23	34 cf	24.00'W x 78.00'L x 0.25'H Mulch
				468 cf Overall x 50.0% Voids
#5	673.00'			24.00'W x 78.00'L x 2.00'H Ponding Z=2.0
		7,1	11 cf	Total Available Storage
Device	Routing	Invert	Outl	et Devices
#1	Discarded	672.75'		0 in/hr Exfiltration over Wetted area
#2	Primary	671.75'		" Round Culvert
			L= /	'5.0' CPP, square edge headwall, Ke= 0.500 : / Outlet Invert= 671.75' / 668.00' S= 0.0500 '/' Cc= 0.900
				0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	671.75'		Vert. Orifice/Grate X 3.00 C= 0.600
"0	Dovido L	011.10	-	ted to weir flow at low heads
#4	Device 2	674.25'		"Horiz. Orifice/Grate X 3.00 C= 0.600
				ted to weir flow at low heads
#5	Secondary	674.50'		l' long x 10.0' breadth Broad-Crested Rectangular Weir
				d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coe	f. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
Discard		lav-0 25 cf	ະ <i>ດ</i> 1	2.26 hrs HW=674.41' (Free Discharge)
	filtration (Exf			

-3=Orifice/Grate (Orifice Controls 0.51 cfs @ 7.73 fps)

-4=Orifice/Grate (Weir Controls 1.99 cfs @ 1.31 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=669.75' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG1: Rain Garden 1

Inflow Are	a =	7,720 sf, 65.00% Imperviou	s, Inflow Depth > 5.02" for 25YearMass event		
Inflow	=	1.03 cfs @ 12.07 hrs, Volume	= 3,228 cf		
Outflow	=	0.57 cfs @ 12.11 hrs, Volume	= 3,211 cf, Atten= 44%, Lag= 2.3 min		
Primary	=	0.57 cfs @ 12.11 hrs, Volume	= 3,211 cf		
Routed to Pond DMH1 : Manhole					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 675.47' @ 12.21 hrs Surf.Area= 1,025 sf Storage= 596 cf

Plug-Flow detention time= 35.5 min calculated for 3,209 cf (99% of inflow) Center-of-Mass det. time= 32.1 min (814.4 - 782.3)

Postdevelopment

Type III 24-hr 25YearMass Rainfall=6.18" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 39

Volume	Invert	Avail.Storage	Storage Description
#1	671.75'	47 cf	12.0" Round Pipe Storage Inside #2
#0	674 75	241 of	L= 60.0'
#2	671.75'	341 cf	5.00'W x 60.00'L x 3.00'H Soil Media and Gravel 900 cf Overall - 47 cf Embedded = 853 cf x 40.0% Voids
#3	674.75'	38 cf	
			75 cf Overall x 50.0% Voids
#4	675.00'	435 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		861 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	671.75' 6.0'	' Round Culvert
		L= 6	6.0' CPP, square edge headwall, Ke= 0.500
		Inle	t / Outlet Invert= 671.75' / 671.60' S= 0.0250 '/' Cc= 0.900
		n= (0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		'Horiz. Orifice/Grate C= 0.600
		Lim	ited to weir flow at low heads
#3	Device 1	671.75' 2.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	674.00' 4.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.56 cfs @ 12.11 hrs HW=675.35' TW=674.22' (Dynamic Tailwater)

1=Culvert (Passes 0.56 cfs of 1.00 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs) -3=Orifice/Grate (Orifice Controls 0.11 cfs @ 5.11 fps)

-4=Orifice/Grate (Orifice Controls 0.45 cfs @ 5.11 fps)

Summary for Pond RG2: Rain Garden 2

Inflow Are	a =	6,820 sf, 65.00% Impervious,	Inflow Depth > 5.02" for 25YearMass event		
Inflow	=	0.91 cfs @ 12.07 hrs, Volume=	2,852 cf		
Outflow	=	0.58 cfs @ 12.15 hrs, Volume=	2,841 cf, Atten= 36%, Lag= 4.5 min		
Primary	=	0.58 cfs @ 12.15 hrs, Volume=	2,841 cf		
Routed to Pond DMH2 : Manhole					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 677.12' @ 12.16 hrs Surf.Area= 1,019 sf Storage= 503 cf

Plug-Flow detention time= 20.8 min calculated for 2,841 cf (100% of inflow) Center-of-Mass det. time= 18.4 min (800.8 - 782.3)

Volume	Invert	Avail.Storage	Storage Description
#1	673.75'	43 cf	12.0" Round Pipe Storage Inside #2
			L= 55.0'
#2	673.75'	379 cf	6.00'W x 55.00'L x 3.00'H Soil Media and Gravel
			990 cf Overall - 43 cf Embedded = 947 cf x 40.0% Voids
#3	676.75'	41 cf	6.00'W x 55.00'L x 0.25'H Mulch
			83 cf Overall x 50.0% Voids
#4	677.00'	457 cf	6.00'W x 55.00'L x 1.00'H Ponding Z=2.0
		921 cf	Total Available Storage

AP5&6

AP5&6

Postdevelopment "Type III 24-hr 25YearMass Rainfall=6.18

> Printed 5/1/2023 Page 40

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	673.75'	6.0" Round Culvert
	-		L= 6.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 673.75' / 673.60' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	677.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	673.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	676.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.58 cfs @ 12.15 hrs HW=677.11' TW=674.55' (Dynamic Tailwater)

1=Culvert (Passes 0.58 cfs of 1.51 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.17 cfs @ 7.71 fps)

-4=Orifice/Grate (Orifice Controls 0.41 cfs @ 4.68 fps)

Summary for Pond RG3: Rain Garden 3

Inflow Are	a =	6,780 sf, 57.81% Impervious,	Inflow Depth > 4.80" for 25YearMass event		
Inflow	=	0.88 cfs @ 12.07 hrs, Volume=	2,710 cf		
Outflow	=	0.63 cfs @ 12.14 hrs, Volume=	2,703 cf, Atten= 28%, Lag= 4.2 min		
Primary	=	0.63 cfs @ 12.14 hrs, Volume=	2,703 cf		
Routed to Pond DMH2 : Manhole					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 679.24' @ 12.14 hrs Surf.Area= 671 sf Storage= 351 cf

Plug-Flow detention time= 12.1 min calculated for 2,703 cf (100% of inflow) Center-of-Mass det. time= 10.6 min (799.6 - 789.1)

Volume	Invert	Avail.Storage	Storage Description
#1	675.75'	27 cf	12.0" Round Pipe Storage Inside #2 L= 35.0'
#2	675.75'	241 cf	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel 630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids
#3	678.75'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch 53 cf Overall x 50.0% Voids
#4	679.00'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	L= Inle	' Round Culvert 76.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 675.75' / 673.60' S= 0.0283 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	679.50' 6.0'	'Horiz. Orifice/Grate C= 0.600 ited to weir flow at low heads
#3 #4	Device 1 Device 1	675.75' 2.0'	'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads 'Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.63 cfs @ 12.14 hrs HW=679.24' TW=674.53' (Dynamic Tailwater) **1=Culvert** (Passes 0.63 cfs of 1.25 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.19 cfs @ 8.89 fps)

-4=Orifice/Grate (Orifice Controls 0.44 cfs @ 4.99 fps)

Summary for Pond RG69: Rain Garden 69

3,630 sf, 65.00% Impervious, Inflow Depth > 5.02" for 25YearMass event Inflow Area = Inflow = 0.48 cfs @ 12.07 hrs, Volume= 1,518 cf 0.42 cfs @ 12.11 hrs, Volume= 1,462 cf, Atten= 13%, Lag= 2.6 min Outflow = Discarded = 0.02 cfs @ 12.06 hrs, Volume= 712 cf Primary = 0.40 cfs @ 12.11 hrs, Volume= 749 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 678.39' @ 12.11 hrs Surf.Area= 420 sf Storage= 283 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 59.7 min (842.0 - 782.3)

Volume	Invert	Avail.Storage	Storage Description
#1	675.25'	27 cf	12.0" Round Pipe Storage Inside #2
#2	675.25'	241 cf	L= 35.0' 6.00'W x 35.00'L x 3.00'H Soil Media and Gravel
#3	678.25'	26 cf	630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids 6.00'W x 35.00'L x 0.25'H Mulch
#4	678.50'	297 cf	53 cf Overall x 50.0% Voids 6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	675.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	677.25'	6.0" Round Culvert
			L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 677.25' / 677.00' S= 0.0096 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	679.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2	677.25'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	677.75'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.02 cfs @ 12.06 hrs HW=678.28' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.40 cfs @ 12.11 hrs HW=678.39' TW=0.00' (Dynamic Tailwater)

-2=Culvert (Passes 0.40 cfs of 0.79 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.11 cfs @ 4.95 fps)

-5=Orifice/Grate (Orifice Controls 0.29 cfs @ 3.31 fps)

Summary for Pond RG70: Rain Garden 70

Inflow Area = 5,415 sf, 65.00% Impervious, Inflow Depth > 5.02" for 25YearMass event 0.72 cfs @ 12.07 hrs, Volume= Inflow = 2.264 cf 0.54 cfs @ 12.14 hrs, Volume= 2,135 cf, Atten= 25%, Lag= 3.9 min Outflow = Discarded = 0.04 cfs @ 12.14 hrs, Volume= 760 cf Primary = 0.50 cfs @ 12.14 hrs, Volume= 1.375 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.72' @ 12.14 hrs Surf.Area= 668 sf Storage= 346 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 31.3 min (813.7 - 782.3)

Volume	Invert	Avail.Storage	Storage Description
#1	671.25'	27 cf	12.0" Round Pipe Storage Inside #2
			L= 35.0'
#2	671.25'	241 cf	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel
			630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids
#3	674.25'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch
			53 cf Overall x 50.0% Voids
#4	674.50'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage
			•

Device	Routing	Invert	Outlet Devices
#1	Discarded	671.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	673.25'	6.0" Round Culvert
			L= 20.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 673.25' / 673.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	675.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2		2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	673.75'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.14 hrs HW=674.72' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.50 cfs @ 12.14 hrs HW=674.72' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 0.50 cfs of 0.99 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.12 cfs @ 5.68 fps)

-5=Orifice/Grate (Orifice Controls 0.38 cfs @ 4.33 fps)

	Postdevelopment
AP5&6	Type III 24-hr 25YearMass Rainfall=6.18"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC Page 43

Summary for Pond TD1: Trench Drain

Inflow Area		21,545 sf, 65.00% Impervious, Inflow Depth > 5.02" for 25YearMass event								
Inflow = Outflow =		2.87 cfs @ 12.07 hrs, Volume= 9,009 cf 2.81 cfs @ 12.08 hrs, Volume= 9,006 cf, Atten= 2%, Lag= 0.5 min								
Primary :										
	Routed to Pond RG-5.1 : Rain Garden 5.1									
			e Span= 0.00-24.00 hrs, dt= 0.01 hrs							
Peak Elev=	674.52' @ 1	2.23 hrs Su	rf.Area= 30 sf Storage= 42 cf							
Diver Flowed	latantian tina		louisted for 0.002 of (1000/ of inflow)							
U U			lculated for 9,003 cf (100% of inflow) 83.1 - 782.3)							
Center-or-In		== 0.0 mm (<i>1</i>	65.1 - 762.5)							
Volume	Invert	Avail.Storage	e Storage Description							
#1	673.10'	60 c	of 1.50'W x 20.00'L x 2.00'H Prismatoid							
Device Ro	outing	Invert O	utlet Devices							
	outing imary	673.10' 1 2	2.0" Round Culvert							
-	<u> </u>	673.10' 12 L=	2.0" Round Culvert = 2.0' CPP, square edge headwall, Ke= 0.500							
-	<u> </u>	673.10' 12 L= In	2.0" Round Culvert = 2.0' CPP, square edge headwall, Ke= 0.500 let / Outlet Invert= 673.10' / 673.00' S= 0.0500 '/' Cc= 0.900							
-	<u> </u>	673.10' 12 L= In	2.0" Round Culvert = 2.0' CPP, square edge headwall, Ke= 0.500							

Primary OutFlow Max=2.65 cfs @ 12.08 hrs HW=674.36' TW=673.87' (Dynamic Tailwater) -1=Culvert (Inlet Controls 2.65 cfs @ 3.37 fps)

Summary for Link AP5: AP5 - To Wetland A (A23-A32) / VP A1

Inflow Area =	180,650 sf, 19.26% Impervious,	Inflow Depth > 3.04"	for 25YearMass event
Inflow =	10.22 cfs @ 12.19 hrs, Volume=	45,754 cf	
Primary =	10.22 cfs @ 12.19 hrs, Volume=	45,754 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link AP6: AP6 - To Wetland B (off-site)

Inflow Area =	111,800 sf, 0.00% Impervious,	Inflow Depth > 2.63"	for 25YearMass event
Inflow =	6.35 cfs @ 12.14 hrs, Volume=	24,517 cf	
Primary =	6.35 cfs @ 12.14 hrs, Volume=	24,517 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P5.1: To Wetland A (A23-A32) / VP A1

Runoff = 9.90 cfs @ 12.13 hrs, Volume= 34,310 cf, Depth> 4.85" Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN E	CN Description						
	34,145	70 V	70 Woods, Good, HSG C						
	38,725	77 V	Woods, Good, HSG D						
	2,390	80 >	75% Gras	s cover, Go	ood, HSG D				
	8,015	74 >	75% Gras	s cover, Go	ood, HSG C				
	1,535	98 L	Inconnecte	ed roofs, HS	SG C				
	84,810	74 V	Veighted A	verage					
	83,275	g	8.19% Per	vious Area					
	1,535	1	.81% Impe	ervious Area	а				
	1,535	1	00.00% Üı	nconnected	1				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
7.5	50	0.0750	0.11		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.00"				
1.8	150	0.0750	1.37		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
9.3	200	Total							

Summary for Subcatchment P5.2: Upgradient Lots 6-8

Runoff = 1.49 cfs @ 12.12 hrs, Volume= 5,045 cf, Depth> 2.84" Routed to Pond IT-5/8 : Interceptor Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Area (sf)	CN	Description
300	61	>75% Grass cover, Good, HSG B
485	74	>75% Grass cover, Good, HSG C
19,455	55	Woods, Good, HSG B
1,085	70	Woods, Good, HSG C
21,325	56	Weighted Average
21,325		100.00% Pervious Area

Prepare	AP5&6Type III 24-hr100YearMass Rainfall=7.93"Prepared by Mcclure EngineeeringPrinted 5/1/2023HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLCPage 45						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.7	50	0.1000	0.12		Sheet Flow,		
0.9	85	0.1000	1.58		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps		
0.0	5	0.1000	2.21		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
7.6	140	Total					
			Summar	y for Sub	ocatchment P5.3: To RG5.1		
Runoff Route	= ed to Pon		s @ 12.0 Trench Dra		me= 12,086 cf, Depth> 6.73"		
			hod, UH=S ss Rainfall		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs		
Δ	rea (sf)	CN E	Description				
	21,545			s, 65% imp	HSG C		
	7,541 14,004	3	5.00% Per	vious Area pervious Are			
(min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	· · · · · · · · · · · · · · · · · · ·		
5.0					Direct Entry,		
			Summ	ary for S	ubcatchment P5.4: Lot 4		
Runoff Route	= ed to Pon		s @ 12.0 Rain Garde	7 hrs, Volu en 3	me= 3,670 cf, Depth> 6.49"		
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"							
А	rea (sf)	CN E	Description				
	6,030			s, 65% imp	, HSG C		
	750			od, HSG C			
	6,780		Veighted A				
	2,861 3,920		-	vious Area pervious Are			
	0,020						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry,		

Summary for Subcatchment P5.5: Lot 69

Runoff = 0.64 cfs @ 12.07 hrs, Volume= 2,036 cf, Depth> 6.73" Routed to Pond RG69 : Rain Garden 69

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Α	rea (sf)	CN	CN Description						
	3,630	90	1/8 acre lot	s, 65% imp	p, HSG C				
	1,271		35.00% Pervious Area						
	2,360		65.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)					
5.0					Direct Entry,				

Summary for Subcatchment P5.6: Upgradient of Amphibian Crossing

6,585 cf, Depth> 3.50"

Runoff = 1.94 cfs @ 12.12 hrs, Volume= Routed to Reach AC : Amphibian Crossing

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN [CN Description					
	1,860		74 >75% Grass cover, Good, HSG C					
	8,330	70 \	Voods, Go	od, HSG C				
	12,415	55 \	Voods, Go	od, HSG B				
	22,605	62 \	Veighted A	verage				
	22,605			ervious Are	а			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•			
6.7	50	0.1000	0.12		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
1.6	150	0.1000	1.58		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.1	30	0.3000	3.83		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
8.4	230	Total						

Summary for Subcatchment P5.7: Lot 70

Runoff = 0.95 cfs @ 12.07 hrs, Volume= 3,038 cf, Depth> 6.73" Routed to Pond RG70 : Rain Garden 70

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN	N Description						
	5,415	90	0 1/8 acre lots, 65% imp, HSG C						
	1,895 3,520	35.00% Pervious Area 65.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				

Direct Entry,

Summary for Subcatchment P5.8: Lot 3

1.20 cfs @ 12.07 hrs, Volume= 3,826 cf, Depth> 6.73" Runoff = Routed to Pond RG2 : Rain Garden 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Area	(sf)	CN D	Description						
6,	820	90 1	/8 acre lots	s, 65% imp,	, HSG C				
4,	387 433 ength	-		vious Area pervious Are Capacity					
	feet)	(ft/ft)	(ft/sec)	(cfs)	Description				
5.0					Direct Entry,				

Summary for Subcatchment P5.9: Lot 2

Runoff	=	1.36 cfs @	12.07 hrs,	Volume=	4,331 cf,	Depth>	6.73"
Routed	l to Pond	d RG1 : Rain (Garden 1			•	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Α	rea (sf)	CN E	Description						
	7,720	90 1	1/8 acre lots, 65% imp, HSG C						
	2,702	3	5.00% Per	vious Area					
	5,018	6	65.00% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry,				

AP5&6

5.0

Prepared by Mcclure Engineeering

Summary for Subcatchment P6.1: To Wetland B (off-site)

Runoff = 7.44 cfs @ 12.13 hrs, Volume= 25,628 cf, Depth> 4.40" Routed to Link AP6 : AP6 - To Wetland B (off-site)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Α	rea (sf)	CN [Description						
	6,465	74 >	>75% Grass cover, Good, HSG C						
	1,875	55 \	Noods, Go	od, HSG B					
	61,615	70 \	Noods, Go	od, HSG C					
	69,955	70 \	Neighted A	verage					
	69,955		100.00% Pe	ervious Are	а				
Тс	Length	Slope		Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.5	50	0.1600	0.15		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.00"				
0.3	45	0.2200	2.35		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
3.4	290	0.0800	1.41		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
9.2	385	Total							

Summary for Subcatchment P6.2: Upgradient Lots 1-5

Runoff = 3.19 cfs @ 12.15 hrs, Volume= 11,797 cf, Depth> 3.38" Routed to Pond IT-1/5 : Interceptor Trench

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Α	rea (sf)	CN [Description						
	2,130	74 >	>75% Gras	s cover, Go	ood, HSG C				
	24,405	55 \	Woods, Good, HSG B						
	15,310	70 \	Woods, Good, HSG C						
	41,845	61 \	Neighted A	verage					
	41,845		100.00% Pe	ervious Are	а				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
8.2	50	0.0600	0.10		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.00"				
2.7	160	0.0400	1.00		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
10.9	210	Total							

		Postdevelopment
AP5&6	Type III 24-hr	100YearMass Rainfall=7.93"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software	Solutions LLC	Page 49

Summary for Reach AC: Amphibian Crossing

 Inflow Area =
 22,605 sf,
 0.00% Impervious,
 Inflow Depth >
 3.50" for
 100YearMass event

 Inflow =
 1.94 cfs @
 12.12 hrs,
 Volume=
 6,585 cf

 Outflow =
 1.93 cfs @
 12.13 hrs,
 Volume=
 6,579 cf,
 Atten= 0%,
 Lag= 0.5 min

 Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1
 VP A1
 Atten
 <

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Max. Velocity= 0.68 fps, Min. Travel Time= 0.7 min Avg. Velocity = 0.21 fps, Avg. Travel Time= 2.4 min

Peak Storage= 85 cf @ 12.13 hrs Average Depth at Peak Storage= 0.47', Surface Width= 6.00' Bank-Full Depth= 3.50' Flow Area= 21.0 sf, Capacity= 35.65 cfs

6.00' x 3.50' deep channel, n= 0.022 Earth, clean & straight Length= 30.0' Slope= 0.0003 '/' Inlet Invert= 675.83', Outlet Invert= 675.82'



Summary for Pond DMH1: Manhole

21,320 sf, 62.71% Impervious, Inflow Depth > 6.63" Inflow Area = for 100YearMass event Inflow 2.37 cfs @ 12.13 hrs, Volume= 11,786 cf = 2.37 cfs @ 12.13 hrs, Volume= Outflow = 11,786 cf, Atten= 0%, Lag= 0.0 min = 2.37 cfs @ 12.13 hrs, Volume= 11,786 cf Primary Routed to Pond RG-5.1 : Rain Garden 5.1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.94' @ 12.14 hrs Flood Elev= 675.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	671.50'	12.0" Round Culvert L= 50.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 671.50' / 671.00' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.35 cfs @ 12.13 hrs HW=674.94' TW=674.51' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 2.35 cfs @ 2.99 fps)

		Postdevelopment
AP5&6	Type III 24-hr	100YearMass Rainfall=7.93"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software	Solutions LLC	Page 50

Summary for Pond DMH2: Manhole

Inflow Area = 13,600 sf, 61.42% Impervious, Inflow Depth > 6.60" for 100YearMass event Inflow 1.52 cfs @ 12.14 hrs, Volume= = 7.476 cf 1.52 cfs @ 12.14 hrs, Volume= 7,476 cf, Atten= 0%, Lag= 0.0 min Outflow = Primary = 1.52 cfs @ 12.14 hrs, Volume= 7,476 cf Routed to Pond DMH1 : Manhole Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 675.17' @ 12.15 hrs Flood Elev= 678.20' Device Routing Invert Outlet Devices Primary #1 673.50' 12.0" Round Culvert L= 80.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 673.50' / 671.60' S= 0.0237 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.48 cfs @ 12.14 hrs HW=675.16' TW=674.94' (Dynamic Tailwater) -1=Culvert (Outlet Controls 1.48 cfs @ 1.88 fps)

Summary for Pond IT-1/5: Interceptor Trench

 Inflow Area =
 41,845 sf, 0.00% Impervious, Inflow Depth > 3.38" for 100YearMass event

 Inflow =
 3.19 cfs @
 12.15 hrs, Volume=
 11,797 cf

 Outflow =
 2.54 cfs @
 12.25 hrs, Volume=
 11,754 cf, Atten= 20%, Lag= 5.8 min

 Primary =
 2.54 cfs @
 12.25 hrs, Volume=
 11,754 cf

 Routed to Link AP6 : AP6 - To Wetland B (off-site)
 11,754 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 679.67' @ 12.25 hrs Surf.Area= 750 sf Storage= 854 cf

Plug-Flow detention time= 6.9 min calculated for 11,754 cf (100% of inflow) Center-of-Mass det. time= 4.7 min (852.2 - 847.4)

Volume	Invert	Avail.Storag	e Storage Description				
#1	677.00'	865 0	of 3.00'W x 250.00'L x 3.00'H Prismatoid				
#2	677.00'	87 0	2,250 cf Overall - 87 cf Embedded = 2,163 cf x 40.0% Voids f 8.0" Round Pipe Storage Inside #1 L= 250.0'				
		952 0	of Total Available Storage				
Device	Routing	Invert O	utlet Devices				
#1	Primary	L= In	0" Round Culvert = 220.0' CPP, square edge headwall, Ke= 0.500 let / Outlet Invert= 677.00' / 668.00' S= 0.0409 '/' Cc= 0.900 = 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf				
Primary	Primary OutFlow Max=2.54 cfs @ 12.25 hrs HW=679.67' TW=0.00' (Dynamic Tailwater)						

1=Culvert (Barrel Controls 2.54 cfs @ 7.28 fps)

		Postdevelopment
AP5&6	Type III 24-hr	100YearMass Rainfall=7.93"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software	Solutions LLC	Page 51

Summary for Pond IT-5/8: Interceptor Trench

Inflow Area = 21,325 sf, 0.00% Impervious, Inflow Depth > 2.84" for 100YearMass event 1.49 cfs @ 12.12 hrs, Volume= Inflow = 5.045 cf 0.96 cfs @ 12.24 hrs, Volume= 5,026 cf, Atten= 36%, Lag= 7.5 min Outflow = Primary = 0.96 cfs @ 12.24 hrs, Volume= 5.026 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1 Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 682.27' @ 12.24 hrs Surf.Area= 465 sf Storage= 441 cf Plug-Flow detention time= 7.0 min calculated for 5,026 cf (100% of inflow) Center-of-Mass det. time= 4.9 min (861.1 - 856.2) Avail.Storage Storage Description Volume Invert #1 680.00' 546 cf 3.00'W x 155.00'L x 3.00'H Prismatoid 1,395 cf Overall - 30 cf Embedded = 1,365 cf x 40.0% Voids #2 6.0" Round Pipe Storage Inside #1 680.00' 30 cf L= 155.0' 576 cf Total Available Storage Device Routing Invert Outlet Devices #1 680.00' 6.0" Round Culvert Primary L= 110.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 680.00' / 678.00' S= 0.0182 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf Primary OutFlow Max=0.96 cfs @ 12.24 hrs HW=682.27' TW=0.00' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.96 cfs @ 4.88 fps) Summary for Pond RG-5.1: Rain Garden 5.1

42,865 sf, 63.86% Impervious, Inflow Depth > 6.68" for 100YearMass event Inflow Area = Inflow 5.76 cfs @ 12.09 hrs, Volume= 23.870 cf = Outflow 5.04 cfs @ 12.15 hrs, Volume= 22,244 cf, Atten= 13%, Lag= 3.3 min = 0.25 cfs @ 12.15 hrs, Volume= Discarded = 3.987 cf Primary = 4.73 cfs @ 12.15 hrs, Volume= 18,250 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1 0.05 cfs @ 12.15 hrs, Volume= Secondary = 7 cf Routed to Link AP6 : AP6 - To Wetland B (off-site)

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 674.52' @ 12.15 hrs Surf.Area= 4,399 sf Storage= 5,833 cf

Plug-Flow detention time= 96.8 min calculated for 22,235 cf (93% of inflow) Center-of-Mass det. time= 61.1 min (847.2 - 786.1)

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Tyuloca	D® 10.20-2g s/	11 03362 @ 2	2022 F	IydroCAD Software Solutions LLC Page 52
Volume	Invert	Avail.Sto	rage	Storage Description
#1	669.75'	4	47 cf	12.0" Round Pipe Storage Inside #3 L= 60.0'
#2	669.75'	1,12	23 cf	24.00'W x 39.00'L x 3.00'H Soil Media and Gravel Impervious 2,808 cf Overall x 40.0% Voids
#3	669.75'	1,10	04 cf	24.00'W x 39.00'L x 3.00'H Soil Media and Gravel Impervious 2,808 cf Overall - 47 cf Embedded = 2,761 cf x 40.0% Voids
#4	672.75'	23	34 cf	
#5	673.00'	4,60	03 cf	24.00'W x 78.00'L x 2.00'H Ponding Z=2.0
Device	Routing	7,1 ⁻ Invert		Total Available Storage et Devices
#1 #2	Discarded Primary	672.75' 671.75'	12.0 L= 7 Inlet	0 in/hr Exfiltration over Wetted area " Round Culvert 5.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 671.75' / 668.00' S= 0.0500 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	671.75'	2.0"	Vert. Orifice/Grate X 3.00 C= 0.600 ted to weir flow at low heads
#4	Device 2	674.25'	-	" Horiz. Orifice/Grate X 3.00 C= 0.600 ted to weir flow at low heads
#5	Secondary	674.50'	Hea	' long x 10.0' breadth Broad-Crested Rectangular Weir d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 f. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=4.73 cfs @ 12.15 hrs HW=674.52' TW=0.00' (Dynamic Tailwater) **2=Culvert** (Passes 4.73 cfs of 5.69 cfs potential flow) -3=Orifice/Grate (Orifice Controls 0.52 cfs @ 7.89 fps) -4=Orifice/Grate (Weir Controls 4.21 cfs @ 1.68 fps)

Secondary OutFlow Max=0.05 cfs @ 12.15 hrs HW=674.52' TW=0.00' (Dynamic Tailwater) 5=Broad-Crested Rectangular Weir (Weir Controls 0.05 cfs @ 0.31 fps)

Summary for Pond RG1: Rain Garden 1

Inflow Are	a =	7,720 sf	, 65.00% Impervious,	Inflow Depth > 6.	.73" for 100YearMass event
Inflow	=	1.36 cfs @	12.07 hrs, Volume=	4,331 cf	
Outflow	=	0.89 cfs @	12.11 hrs, Volume=	4,310 cf,	Atten= 34%, Lag= 2.5 min
Primary	=	0.89 cfs @	12.11 hrs, Volume=	4,310 cf	
Routed	I to Pond	d DMH1 : Mar	hole		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 675.74' @ 12.16 hrs Surf.Area= 1,102 sf Storage= 723 cf

Plug-Flow detention time= 33.3 min calculated for 4,308 cf (99% of inflow) Center-of-Mass det. time= 30.2 min (804.9 - 774.8)

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 53

Volume	Invert	Avail.Storage	Storage Description
#1	671.75'	47 cf	12.0" Round Pipe Storage Inside #2 L= 60.0'
#2	671.75'	341 cf	
#3	674.75'	38 cf	5.00'W x 60.00'L x 0.25'H Mulch 75 cf Overall x 50.0% Voids
#4	675.00'	435 cf	5.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		861 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	L= (Inle	" Round Culvert 6.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 671.75' / 671.60' S= 0.0250 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	675.50' 6.0 '	" Horiz. Orifice/Grate C= 0.600 ited to weir flow at low heads
#3 #4	Device 1 Device 1	-	Vert. Orifice/GrateC= 0.600Limited to weir flow at low headsVert. Orifice/GrateC= 0.600Limited to weir flow at low heads

Primary OutFlow Max=0.85 cfs @ 12.11 hrs HW=675.69' TW=674.89' (Dynamic Tailwater)

1=Culvert (Inlet Controls 0.85 cfs @ 4.31 fps)

2=Orifice/Grate (Passes < 0.41 cfs potential flow)
 3=Orifice/Grate (Passes < 0.09 cfs potential flow)

-4=Orifice/Grate (Passes < 0.38 cfs potential flow)

Summary for Pond RG2: Rain Garden 2

Inflow Are	a =	6,820 sf, 65.00% Impervio	us, Inflow Depth > 6.73" for 100YearMass event		
Inflow	=	1.20 cfs @ 12.07 hrs, Volum	e= 3,826 cf		
Outflow	=	0.66 cfs @ 12.18 hrs, Volum	e= 3,814 cf, Atten= 45%, Lag= 6.6 min		
Primary	=	0.66 cfs @ 12.18 hrs, Volum	e= 3,814 cf		
Routed to Pond DMH2 : Manhole					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 677.52' @ 12.18 hrs Surf.Area= 1,120 sf Storage= 666 cf

Plug-Flow detention time= 20.5 min calculated for 3,814 cf (100% of inflow) Center-of-Mass det. time= 18.4 min (793.2 - 774.8)

Volume	Invert	Avail.Storage	Storage Description
#1	673.75'	43 cf	12.0" Round Pipe Storage Inside #2
#0		070 -{	L= 55.0'
#2	673.75'	379 CT	6.00'W x 55.00'L x 3.00'H Soil Media and Gravel 990 cf Overall - 43 cf Embedded = 947 cf x 40.0% Voids
#3	676,75'	41 cf	6.00'W x 55.00'L x 0.25'H Mulch
<i></i> 0	0/0./0		83 cf Overall x 50.0% Voids
#4	677.00'	457 cf	6.00'W x 55.00'L x 1.00'H Ponding Z=2.0
		921 cf	Total Available Storage

AP5&6

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

Page 54

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g_s/n 03362_© 2022 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	673.75'	6.0" Round Culvert
	•		L= 6.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 673.75' / 673.60' S= 0.0250 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	677.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	673.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	676.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.66 cfs @ 12.18 hrs HW=677.52' TW=675.13' (Dynamic Tailwater)

1=Culvert (Passes 0.66 cfs of 1.46 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.01 cfs @ 0.40 fps)

-3=Orifice/Grate (Orifice Controls 0.16 cfs @ 7.44 fps)

-4=Orifice/Grate (Orifice Controls 0.49 cfs @ 5.59 fps)

Summary for Pond RG3: Rain Garden 3

Inflow Are	a =	6,780 sf, 57.81% Impervious, Inflo	w Depth > 6.49" for 100YearMass event	
Inflow	=	1.17 cfs @ 12.07 hrs, Volume=	3,670 cf	
Outflow	=	0.88 cfs @ 12.14 hrs, Volume=	3,662 cf, Atten= 25%, Lag= 3.9 min	
Primary	=	0.88 cfs @ 12.14 hrs, Volume=	3,662 cf	
Routed to Pond DMH2 : Manhole				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 679.60' @ 12.14 hrs Surf.Area= 735 sf Storage= 452 cf

Plug-Flow detention time= 11.9 min calculated for 3,662 cf (100% of inflow) Center-of-Mass det. time= 10.6 min (791.5 - 780.9)

Volume	Invert	Avail.Storage	Storage Description
#1	675.75'	27 cf	12.0" Round Pipe Storage Inside #2 L= 35.0'
#2	675.75'	241 cf	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel
#3	678.75'	26 cf	630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids 6.00'W x 35.00'L x 0.25'H Mulch 53 cf Overall x 50.0% Voids
#4	679.00'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	675.75' 6.0 '	" Round Culvert
		Inle	76.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 675.75' / 673.60' S= 0.0283 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	679.50' 6.0 '	Horiz. Orifice/Grate C= 0.600 ited to weir flow at low heads
#3 #4	Device 1 Device 1	675.75' 2.0 '	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

AP5&6

Primary OutFlow Max=0.88 cfs @ 12.14 hrs HW=679.60' TW=675.16' (Dynamic Tailwater) **1=Culvert** (Passes 0.88 cfs of 1.21 cfs potential flow)

-2=Orifice/Grate (Weir Controls 0.17 cfs @ 1.05 fps) -3=Orifice/Grate (Orifice Controls 0.20 cfs @ 9.35 fps)

-4=Orifice/Grate (Orifice Controls 0.50 cfs @ 5.77 fps)

Summary for Pond RG69: Rain Garden 69

3,630 sf, 65.00% Impervious, Inflow Depth > 6.73" for 100YearMass event Inflow Area = Inflow = 0.64 cfs @ 12.07 hrs, Volume= 2,036 cf 0.51 cfs @ 12.13 hrs, Volume= Outflow = 1,933 cf, Atten= 21%, Lag= 3.4 min Discarded = 0.04 cfs @ 12.13 hrs, Volume= 766 cf 0.47 cfs @ 12.13 hrs, Volume= 1.167 cf Primary = Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 678.61' @ 12.13 hrs Surf.Area= 649 sf Storage= 320 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 39.3 min (814.0 - 774.8)

Volume	Invert	Avail.Storage	Storage Description
#1	675.25'	27 cf	12.0" Round Pipe Storage Inside #2
<i>щ</i> о		044 of	L= 35.0'
#2	675.25'	24 I CI	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel 630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids
#3	678.25'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch
			53 cf Overall x 50.0% Voids
#4	678.50'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	675.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	677.25'	6.0" Round Culvert
			L= 26.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 677.25' / 677.00' S= 0.0096 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	679.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2		
#5	Device 2	677.75'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.13 hrs HW=678.61' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.47 cfs @ 12.13 hrs HW=678.61' TW=0.00' (Dynamic Tailwater)

-2=Culvert (Passes 0.47 cfs of 0.88 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.12 cfs @ 5.45 fps)

-5=Orifice/Grate (Orifice Controls 0.35 cfs @ 4.02 fps)

Summary for Pond RG70: Rain Garden 70

Inflow Area = 5,415 sf, 65.00% Impervious, Inflow Depth > 6.73" for 100YearMass event Inflow 0.95 cfs @ 12.07 hrs, Volume= = 3.038 cf 0.66 cfs @ 12.15 hrs, Volume= Outflow 2,870 cf, Atten= 31%, Lag= 4.5 min = Discarded = 0.04 cfs @ 12.15 hrs, Volume= 823 cf Primary = 0.62 cfs @ 12.15 hrs, Volume= 2,047 cf Routed to Link AP5 : AP5 - To Wetland A (A23-A32) / VP A1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 675.04' @ 12.15 hrs Surf.Area= 723 sf Storage= 432 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 21.0 min (795.8 - 774.8)

Volume	Invert	Avail.Storage	Storage Description
#1	671.25'	27 cf	12.0" Round Pipe Storage Inside #2
			L= 35.0'
#2	671.25'	241 cf	6.00'W x 35.00'L x 3.00'H Soil Media and Gravel
			630 cf Overall - 27 cf Embedded = 603 cf x 40.0% Voids
#3	674.25'	26 cf	6.00'W x 35.00'L x 0.25'H Mulch
			53 cf Overall x 50.0% Voids
#4	674.50'	297 cf	6.00'W x 35.00'L x 1.00'H Ponding Z=2.0
		592 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Discarded	671.25'	2.410 in/hr Exfiltration over Surface area
#2	Primary	673.25'	6.0" Round Culvert
			L= 20.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 673.25' / 673.00' S= 0.0125 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Device 2	675.00'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#4	Device 2	673.25'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	673.75'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.04 cfs @ 12.15 hrs HW=675.04' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.62 cfs @ 12.15 hrs HW=675.04' TW=0.00' (Dynamic Tailwater)

-2=Culvert (Passes 0.62 cfs of 1.11 cfs potential flow)

-3=Orifice/Grate (Weir Controls 0.04 cfs @ 0.62 fps)

-4=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.28 fps)

-5=Orifice/Grate (Orifice Controls 0.44 cfs @ 5.09 fps)

		Postdevelopment
AP5&6	Type III 24-hr	100YearMass Rainfall=7.93"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software	Solutions LLC	Page 57

Summary for Pond TD1: Trench Drain

Inflow = Outflow = Primary =	Inflow Area = 21,545 sf, 65.00% Impervious, Inflow Depth > 6.73" for 100YearMass event Inflow = 3.79 cfs @ 12.07 hrs, Volume= 12,086 cf Outflow = 3.84 cfs @ 12.07 hrs, Volume= 12,084 cf, Atten= 0%, Lag= 0.0 min Primary = 3.84 cfs @ 12.07 hrs, Volume= 12,084 cf Routed to Pond RG-5.1 : Rain Garden 5.1 12								
Routing by I	Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs								
			Area= 30 sf Storage= 60 cf						
-	letention time= (lass det. time= (ulated for 12,079 cf (100% of inflow)						
Center-or-In		0.7 mm (770	5.4 - 774.0)						
Volume	Invert Av	vail.Storage	Storage Description						
#1	673.10'	60 cf	1.50'W x 20.00'L x 2.00'H Prismatoid						
Device Ro	outing	Invert Outl	let Devices						
#1 Pri	imary 6 ⁻)" Round Culvert						
	L= 2.0' CPP, square edge headwall, Ke= 0.500								
	Inlet / Outlet Invert= 673.10' / 673.00' S= 0.0500 '/' Cc= 0.900								
	n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf								
Brimary OutElow Max-2 71 of $(20, 12, 0.7)$ http://www.example.com/article/									

Primary OutFlow Max=3.71 cfs @ 12.07 hrs HW=675.30' TW=674.33' (Dynamic Tailwater) -1=Culvert (Inlet Controls 3.71 cfs @ 4.73 fps)

Summary for Link AP5: AP5 - To Wetland A (A23-A32) / VP A1

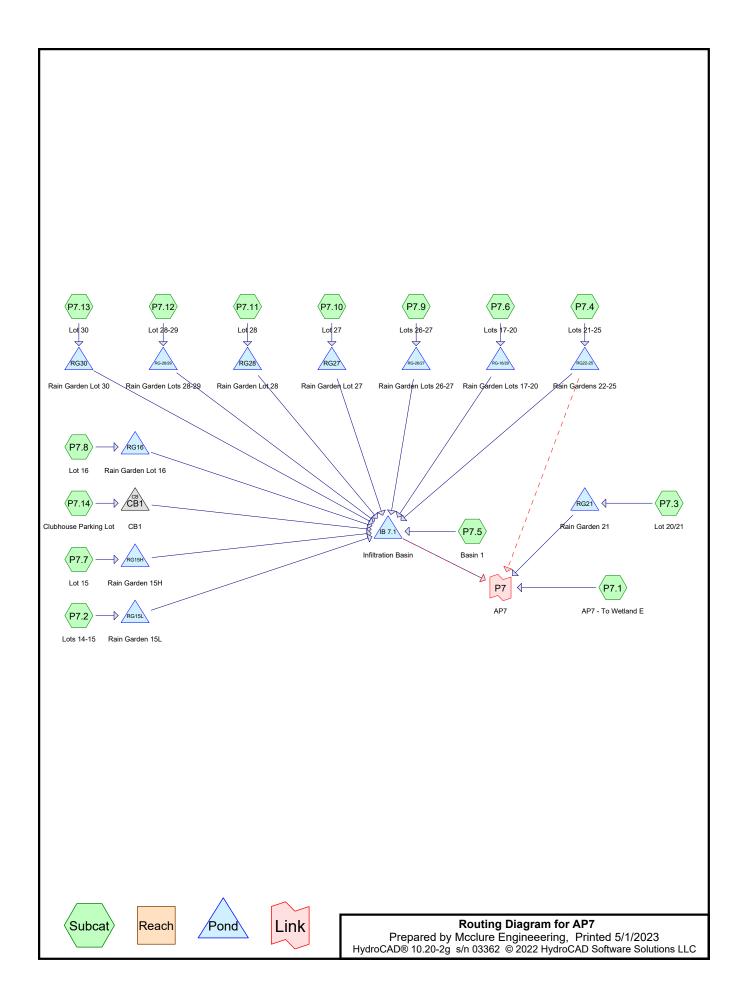
Inflow Are	a =	180,650 sf, 19.26% Impervious, Inflow Depth > 4.48"	for 100YearMass event
Inflow	=	18.49 cfs @ 12.14 hrs, Volume= 67,380 cf	
Primary	=	18.49 cfs @ 12.14 hrs, Volume= 67,380 cf, Atten	= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Link AP6: AP6 - To Wetland B (off-site)

Inflow Area	ı =	111,800 sf,	0.00% Imp	pervious,	Inflow Depth >	4.01"	for 100YearMass event
Inflow	=	9.58 cfs @ 1	12.14 hrs, Ň	/olume=	37,388 c	f	
Primary	=	9.58 cfs @ 1	12.14 hrs, V	/olume=	37,388 c	f, Atten	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs



Summary for Subcatchment P7.1: AP7 - To Wetland E

Runoff = 0.28 cfs @ 12.34 hrs, Volume= 2,264 cf, Depth> 0.29" Routed to Link P7 : AP7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Ar	rea (sf)	CN /	Adj Desc	ription						
	78,255	55	Woo	Woods, Good, HSG B						
	13,515	61	>75%	>75% Grass cover, Good, HSG B						
	1,560	98	Unco	Unconnected roofs, HSG B						
9	93,330	57								
9	91,770			3% Perviou						
	1,560			% Impervio						
	1,560		100.	00% Uncor	nnected					
т.	1	0	V/.1	0	Description					
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
4.3	50	0.3000	0.19		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 3.00"					
0.2	25	0.3000	2.74		Shallow Concentrated Flow,					
				Woodland Kv= 5.0 fps						
4.1	245	0.0400	1.00		Shallow Concentrated Flow,					
					Woodland Kv= 5.0 fps					
8.6	320	Total								

Summary for Subcatchment P7.10: Lot 27

Runoff	=	0.36 cfs @	12.14 hrs,	Volume=	1,287 cf,	Depth>	1.43"
Routed	I to Pond	RG27 : Rair	Garden Lo	t 27			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

	Area (sf)	CN	Description					
*	3,845	90	Residential Lots, 65% imp, HSG C					
	2,045	70	Woods, Good, HSG C					
	940	89	Gravel roads, HSG C					
	3,970	74	>75% Grass cover, Good, HSG C					
	10,800	80	Weighted Average					
	8,301		76.86% Pervious Area					
	2,499		23.14% Impervious Area					

			gineeering		Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023		
HydroCA	D® 10.20-	-2g_s/n 03	362 © 202	2 HydroCAL) Software Solutions LLC Page 3		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
9.7	50	0.0400	0.09		Sheet Flow,		
0.2	35	0.2000	3.13		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
9.9	85	Total					
			Summa	ry for Su	bcatchment P7.11: Lot 28		
Runoff Route	= ed to Pon			7 hrs, Volu den Lot 28	Ime= 2,534 cf, Depth> 1.06"		
			nod, UH=S Rainfall=3		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs		
А	rea (sf)	CN D	escription				
*	4,250				imp, HSG C		
	18,600	70 V	Voods, Go	od, HSG C			
	960		Gravel road				
	4,855 28,665		Veighted A		ood, HSG C		
	25,903			rvious Area			
	2,763			ervious Area			
Tc	Length	Slope	Velocity	Capacity	Description		
<u>(min)</u> 9.7	<u>(feet)</u> 50	(ft/ft) 0.0400	(ft/sec) 0.09	(cfs)	Shoot Flow		
9.7	50	0.0400	0.09		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00"		
1.7	100	0.0400	1.00		Shallow Concentrated Flow, Woodland Kv= 5.0 fps		
0.2	30	0.2000	3.13		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps		
11.6	180	Total					
Summary for Subcatchment P7.12: Lot 28-29							
Runoff Route	= ed to Pon			7 hrs, Volu Garden Lots			
			nod, UH=S Rainfall=3		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs		
Δ	vrea (sf)	CN D	escription				

	Area (sf)	CN	Description
*	17,105	90	Residential Lots, 65% imp, HSG C
	5,987		35.00% Pervious Area
	11,118		65.00% Impervious Area

	Postdevelopment
AP7	Type III 24-hr 2YearMass Rainfall=3.24"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Sol	lutions LLC Page 4
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
5.0 Direct Entr	n /
5.0 Direct Entr	y,
Summary for Subcatchme	nt P7.13: Lot 30
•	
Runoff = 0.41 cfs @ 12.07 hrs, Volume=	1,238 cf, Depth> 2.20"
Routed to Pond RG30 : Rain Garden Lot 30	.,
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time	e Span= 0.00-24.00 hrs. dt= 0.01 hrs
Type III 24-hr 2YearMass Rainfall=3.24"	
·)[- · · · _ · · · · _ · · · · · · · · · ·	
Area (sf) CN Description	
* 6,740 90 Residential Lots, 65% imp, HSG C	
2,359 35.00% Pervious Area	
4,381 65.00% Impervious Area	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
	n/
5.0 Direct Entr	ſ y ,
Summary for Subcatchment P7.14:	Clubhouse Parking Lot
Runoff = 0.57 cfs @ 12.07 hrs, Volume=	1,776 cf, Depth> 2.58"
Routed to Pond CB1 : CB1	
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time	a Span = 0.00-24.00 brs. dt = 0.01 brs.
Type III 24-hr 2YearMass Rainfall=3.24"	c opan- 0.00-24.00 ms, dt- 0.01 ms
1 yp= 111 24-111 21 carinass 1 all 11d11-3.24	
Area (sf) CN Description	
1,410 74 >75% Grass cover, Good, HSG C	
750 08 Poofs HSC C	

	1,410	74 >	>75% Grass cover, Good, HSG C						
	750	98 F	Roofs, HSG	i C					
	6,100	98 F	Paved parking, HSG C						
	8,260	94 V	Weighted Average						
	1,410	1	17.07% Pervious Area						
	6,850	8	82.93% Impervious Area						
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	I				
(11111)		(1010)	(10360)	(013)					

5.0

Direct Entry,

Summary for Subcatchment P7.2: Lots 14-15

noff = 0.50 cfs @ 12.07 hrs, Volume= Routed to Pond RG15L : Rain Garden 15L Runoff 1,504 cf, Depth> 2.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023 plutions LLC Page 5

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

	А	rea (sf)	CN [Description							
*		8,190	90 F	Residential Lots, 65% imp, HSG C							
		2,867	3	35.00% Pervious Area							
		5,324	6	65.00% Impervious Area							
	Тс	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry,					

Summary for Subcatchment P7.3: Lot 20/21

Runoff = 0.40 cfs @ 12.07 hrs, Volume= 1,181 cf, Depth> 1.79" Routed to Pond RG21 : Rain Garden 21

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN E	Description							
	7,915	85 1	85 1/8 acre lots, 65% imp, HSG B							
	2,770	3	35.00% Pervious Area							
	5,145	6	65.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					

Summary for Subcatchment P7.4: Lots 21-25

Runoff = 2.88 cfs @ 12.18 hrs, Volume= 11,346 cf, Depth> 1.36" Routed to Pond RG22-25 : Rain Gardens 22-25

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

	Area (sf)	CN	Description
	38,735	90	1/8 acre lots, 65% imp, HSG C
	11,360	85	1/8 acre lots, 65% imp, HSG B
	49,505	70	Woods, Good, HSG C
*	320	74	Grass Paver, Good, HSG C
	99,920 67,358	79	Weighted Average 67.41% Pervious Area
	32,562		32.59% Impervious Area

AP7 Prepare	AP7 Prepared by Mcclure Engineeering Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023											
					Software Solutions LLC Page 6							
Tc	Length	Slope	Velocity	Capacity	Description							
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
9.7	50	0.0400	0.09		Sheet Flow,							
					Woods: Light underbrush n= 0.400 P2= 3.00"							
0.8	50	0.0400	1.00		Shallow Concentrated Flow,							
					Woodland Kv= 5.0 fps							
2.6	230	0.0900	1.50		Shallow Concentrated Flow,							
					Woodland Kv= 5.0 fps							
13.1	330	Total										

Summary for Subcatchment P7.5: Basin 1

Runoff	=	1.30 cfs @	12.11 hrs,	Volume=	4,573 cf,	Depth>	0.85"
Routed	l to Pond	l IB 7.1 : Infiltr	ration Basin				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

A	rea (sf)	CN E	Description		
	13,590	98 V	Vater Surfa	ace, HSG B	
	1,860	98 F	aved park	ing, HSG C)
	1,425	98 F	aved park	ing, HSG B	
	28,270	55 V	Voods, Go	od, HSG B	
	8,160	74 >	75% Gras	s cover, Go	ood, HSG C
	750	98 F	Roofs, HSG	G C	
	10,520	61 >	75% Gras	s cover, Go	ood, HSG B
	64,575	70 V	Veighted A	verage	
	46,950	7	2.71% Per	vious Area	
	17,625	2	7.29% Imp	ervious Are	ea
			-		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
5.7	50	0.1500	0.15		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
0.9	110	0.1500	1.94		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
6.6	160	Total			

Summary for Subcatchment P7.6: Lots 17-20

Runoff	=	1.32 cfs @	12.07 hrs,	Volume=	3,943 cf,	Depth>	1.95"
Routed	I to Pond	d RG-16/20 : F	Rain Garder	n Lots 17-20			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

Page 7

AP7

 Prepared by Mcclure Engineeering
 Printed

 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC
 Printed

		.						
A	rea (sf)	CN	Description					
	11,200	90	1/8 acre lots	s, 65% imp	, HSG C			
	13,081	85	1/8 acre lots	s, 65% imp	, HSG B			
	24,281	87	Weighted A	verage				
	8,498		35.00% Per		l			
	15,783		65.00% Imp	pervious Are	ea			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
5.0					Direct Entry,			
			Summa	arv for Su	ubcatchment P7.7: Lot 15			
			•••••	,				
Runoff	=	0.64.0	cfs @ 12.0	7 hrs Volu	ume= 1,928 cf, Depth> 2.20"			
	ed to Pond		H : Rain Ga					
rtoutt								
Runoff b	Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs							
Type III 24-hr 2YearMass Rainfall=3.24"								
.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,								
А	rea (sf)	CN	Description					
*	10 500	90		Lots 65%	imp HSG C			

	<u> </u>	rea (SI)		rescription					
*		10,500	90 F	Residential Lots, 65% imp, HSG C					
		3,675	3	35.00% Pervious Area					
		6,825	6	65.00% Impervious Area					
	Тс	Longth	Slope	Velocity	Capacity	Description			
	(min)	Length (feet)	(ft/ft)	(ft/sec)	(cfs)	Description			
	5.0	, , , , , , , , , , , , , , , , , , ,			, <i>, , ,</i>	Direct Entry,			

Summary for Subcatchment P7.8: Lot 16

Runoff = 0.41 cfs @ 12.07 hrs, Volume= 1,239 cf, Depth> 2.20" Routed to Pond RG16 : Rain Garden Lot 16

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

	A	rea (sf)	CN [Description						
*		6,745	90 F	Residential Lots, 65% imp, HSG C						
		2,361		35.00% Pervious Area						
		4,384	6	65.00% Impervious Area						
	Тс	Length	Slope	Velocity	Capacity	Description				
((min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	5.0					Direct Entry,				

Summary for Subcatchment P7.9: Lots 26-27

Runoff = 0.83 cfs @ 12.07 hrs, Volume= 2,499 cf, Depth> 2.20" Routed to Pond RG-26/27 : Rain Garden Lots 26-27

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Area (s	f) CN	Description								
* 13,60	5 90	90 Residential Lots, 65% imp, HSG C								
4,76	2	35.00% Pervious Area								
8,84	3	65.00% Impervious Area								
Tc Leng (min) (fee			Capacity (cfs)	Description						
5.0				Direct Entry,						
		_								

Summary for Pond CB1: CB1

 Inflow Area =
 8,260 sf, 82.93% Impervious, Inflow Depth >
 2.58" for 2YearMass event

 Inflow =
 0.57 cfs @
 12.07 hrs, Volume=
 1,776 cf

 Outflow =
 0.57 cfs @
 12.07 hrs, Volume=
 1,776 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.57 cfs @
 12.07 hrs, Volume=
 1,776 cf

 Routed to Pond IB 7.1 : Infiltration Basin
 1,776 cf
 1,776 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 727.39' @ 12.07 hrs Flood Elev= 730.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	727.00'	12.0" Round Culvert L= 40.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 727.00' / 726.60' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.57 cfs @ 12.07 hrs HW=727.39' TW=704.54' (Dynamic Tailwater) -1=Culvert (Barrel Controls 0.57 cfs @ 2.97 fps)

Summary for Pond IB 7.1: Infiltration Basin

Inflow Area =	299,386 sf	, 39.73% Impervious	Inflow Depth > 1.27"	for 2YearMass event
Inflow =	6.21 cfs @	12.15 hrs, Volume=	31,600 cf	
Outflow =	1.59 cfs @	13.05 hrs, Volume=	27,560 cf, Atter	n= 74%, Lag= 53.8 min
Discarded =	0.46 cfs @	13.05 hrs, Volume=	18,950 cf	
Primary =	1.13 cfs @	13.05 hrs, Volume=	8,610 cf	
Routed to Link	P7 : AP7			
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Routed to Link	P7 : AP7			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 705.98' @ 13.05 hrs Surf.Area= 8,194 sf Storage= 12,575 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 123.1 min (961.2 - 838.1)

Volume	Invert	Avail.Sto	orage	Storage Description		
#1	704.00'	58,8	43 cf	Custom Stage Data	a (Irregular)Listed	below (Recalc)
Elevatio (fee		f.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
704.0 706.0 708.0 710.0)0)0)0 1	4,675 8,235 1,620	575.0 615.0 650.0 680.0	0 12,743 19,758 26,342	0 12,743 32,501 58,843	4,675 8,645 12,389 15,831
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	704.00'		0 in/hr Exfiltration o		
#2	Secondary	709.00'	Head	' long x 10.0' bread d (feet) 0.20 0.40 0 f. (English) 2.49 2.50	.60 0.80 1.00 1.2	0 1.40 1.60
#3	Primary	704.00'	L= 3 Inlet	" Round Culvert 5.0' CPP, square ec / Outlet Invert= 704.0 .013 Corrugated PE	00' / 704.00' S= 0.	0000 '/' Cc= 0.900
#4	Device 3	709.00'	24.0	" Horiz. Orifice/Grat	e C= 0.600	
#5	Device 3	704.00'	Cus t Head	tom Weir/Orifice, Cv d (feet) 0.00 1.40 1	*= 2.62 (C= 3.28) .40 2.00 3.00 3.2	5 3.25 4.00 4.00 5.00 5 0.50 0.50 0.66 0.66
#6	Device 3	705.40'				to weir flow at low heads

Discarded OutFlow Max=0.46 cfs @ 13.05 hrs HW=705.98' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.46 cfs)

Primary OutFlow Max=1.13 cfs @ 13.05 hrs HW=705.98' TW=0.00' (Dynamic Tailwater)

-3=Culvert (Passes 1.13 cfs of 6.89 cfs potential flow)

4=Orifice/Grate (Controls 0.00 cfs)

-5=Custom Weir/Orifice (Weir Controls 0.36 cfs @ 2.49 fps)

-6=Orifice/Grate (Orifice Controls 0.77 cfs @ 2.59 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=704.00' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG-16/20: Rain Garden Lots 17-20

Inflow Area	a =	24,281 sf,	65.00% Impervious,	Inflow Depth > 1.95"	for 2YearMass event	
Inflow	=	1.32 cfs @	12.07 hrs, Volume=	3,943 cf		
Outflow	=	0.54 cfs @	12.29 hrs, Volume=	3,916 cf, Atte	en= 59%, Lag= 12.8 min	
Primary	=	0.54 cfs @	12.29 hrs, Volume=	3,916 cf		
Routed to Pond IB 7.1 : Infiltration Basin						

	Postdevelopment
AP7	Type III 24-hr 2YearMass Rainfall=3.24"
Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Se	olutions LLC Page 10

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 725.98' @ 12.29 hrs Surf.Area= 960 sf Storage= 777 cf

Plug-Flow detention time= 18.8 min calculated for 3,916 cf (99% of inflow) Center-of-Mass det. time= 14.6 min (831.5 - 816.8)

Volume	Invert	Avail.Storage	Storage Description
#1	724.25'	188 cf	12.0" Round Pipe Storage x 4 Inside #2
			L= 60.0'
#2	724.25'	1,077 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 4
			2,880 cf Overall - 188 cf Embedded = 2,692 cf x 40.0% Voids
#3	727.25'	120 cf	4.00'W x 60.00'L x 0.25'H Mulch × 4
			240 cf Overall x 50.0% Voids
#4	727.50'	1,493 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 4
		2,878 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	724.25'	6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 724.25' / 724.25' S= 0.0000 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	728.00'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	724.25'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	726.50'	4.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=0.54 cfs @ 12.29 hrs HW=725.98' TW=705.25' (Dynamic Tailwater)

-1=Culvert (Passes 0.54 cfs of 4.60 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.54 cfs @ 6.17 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond RG-26/27: Rain Garden Lots 26-27

Inflow Are	a =	13,605 sf, 65.00% Impervious	, Inflow Depth > 2.20" for 2YearMass event			
Inflow	=	0.83 cfs @ 12.07 hrs, Volume=	= 2,499 cf			
Outflow	=	0.36 cfs @ 12.25 hrs, Volume=	= 2,488 cf, Atten= 56%, Lag= 10.6 min			
Primary	=	0.36 cfs @ 12.25 hrs, Volume=	= 2,488 cf			
Routed to Pond IB 7.1 : Infiltration Basin						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 746.60' @ 12.25 hrs Surf.Area= 480 sf Storage= 508 cf

Plug-Flow detention time= 18.0 min calculated for 2,487 cf (100% of inflow) Center-of-Mass det. time= 15.4 min (820.3 - 805.0)

Type III 24-hr 2YearMass Rainfall=3.24"

Printed 5/1/2023 Page 11

Volume	Invert	Avail.Stora	age Storage Description
#1	744.25'	94	4 cf 12.0" Round Pipe Storage x 2 Inside #2 L= 60.0'
#2	744.25'	538	8 cf 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,440 cf Overall - 94 cf Embedded = 1,346 cf x 40.0% Voids
#3	747.25'	60	0 cf 4.00'W x 60.00'L x 0.25'H Mulch x 2 120 cf Overall x 50.0% Voids
#4	747.50'	747	7 cf 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,439	9 cf Total Available Storage
Device	Routing	Invert	Outlet Devices
#1	Primary		6.0" Round Culvert X 2.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 744.25' / 744.15' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	748.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads
#3	Device 1	744.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads
#4	Device 1	746.50'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600

Limited to weir flow at low heads

Primary OutFlow Max=0.36 cfs @ 12.25 hrs HW=746.60' TW=705.14' (Dynamic Tailwater)

1=Culvert (Passes 0.36 cfs of 2.74 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.32 cfs @ 7.25 fps)

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

-4=Orifice/Grate (Orifice Controls 0.05 cfs @ 1.07 fps)

Summary for Pond RG-28/29: Rain Garden Lots 28-29

 Inflow Area =
 17,105 sf, 65.00% Impervious, Inflow Depth > 2.20" for 2YearMass event

 Inflow =
 1.04 cfs @
 12.07 hrs, Volume=
 3,141 cf

 Outflow =
 0.71 cfs @
 12.15 hrs, Volume=
 3,129 cf, Atten= 32%, Lag= 4.7 min

 Primary =
 0.71 cfs @
 12.15 hrs, Volume=
 3,129 cf

 Routed to Pond IB 7.1 : Infiltration Basin
 3,129 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 742.37' @ 12.15 hrs Surf.Area= 480 sf Storage= 559 cf

Plug-Flow detention time= 16.8 min calculated for 3,129 cf (100% of inflow) Center-of-Mass det. time= 14.4 min (819.3 - 805.0)

AP7

Type III 24-hr 2YearMass Rainfall=3.24"

Printed 5/1/2023 Page 12

HydroCAD® 10.20-2g	s/n 03362	© 2022 HydroCAD Software Solutions LLC	;

Volume	Invert	Avail.Stor	rage	Storage Description
#1	739.75'	ç	94 cf	12.0" Round Pipe Storage x 2 Inside #2
#2	739.75'	53	38 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,440 cf Overall - 94 cf Embedded = 1,346 cf x 40.0% Voids
#3	742.75'	6	60 cf	
#4	743.00'	74	17 cf	120 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
				Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Primary	739.75'	6.0"	Round Culvert X 2.00
				0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 739.75' / 739.65' S= 0.0100 '/' Cc= 0.900
				.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	743.50'		Horiz. Orifice/Grate X 2.00 C= 0.600
		700 751		ted to weir flow at low heads
#3	Device 1	739.75'	-	Vert. Orifice/Grate X 2.00 C= 0.600
	During 4	740.00		ted to weir flow at low heads
#4	Device 1	742.00'	-	Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads

Primary OutFlow Max=0.71 cfs @ 12.15 hrs HW=742.37' TW=704.81' (Dynamic Tailwater)

-1=Culvert (Passes 0.71 cfs of 2.91 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.33 cfs @ 7.67 fps)

-4=Orifice/Grate (Orifice Controls 0.38 cfs @ 2.16 fps)

Summary for Pond RG15H: Rain Garden 15H

10,500 sf, 65.00% Impervious, Inflow Depth > 2.20" for 2YearMass event Inflow Area = Inflow = 0.64 cfs @ 12.07 hrs, Volume= 1,928 cf Outflow 0.47 cfs @ 12.14 hrs, Volume= 1,807 cf, Atten= 26%, Lag= 4.0 min = Discarded = 0.04 cfs @ 12.07 hrs, Volume= 778 cf 0.43 cfs @ 12.14 hrs, Volume= Primary = 1,029 cf Routed to Pond IB 7.1 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 718.63' @ 12.14 hrs Surf.Area= 720 sf Storage= 378 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 45.1 min (850.1 - 805.0)

AP7

Type III 24-hr 2YearMass Rainfall=3.24" 1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 13

Volume	Invert	Avail.Storage	Storage Description
#1	715.25'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	715.25'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	718.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	718.50'	240 cf	4.00'W x 60.00'L x 1.00'H Ponding
		586 cf	
		000 01	
Device	Routing	Invert Out	tlet Devices
#1	Primary		" Round Culvert
#1	Filliary	••••	
			10.0' CPP, square edge headwall, Ke= 0.500
		Inle	t / Outlet Invert= 717.25' / 717.15' S= 0.0100 '/' Cc= 0.900
		n=	0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1		"Horiz. Orifice/Grate C= 0.600
		Lim	ited to weir flow at low heads
#3	Device 1	717.25' 3.0	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1		"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded		10 in/hr Exfiltration over Surface area
#5	Discarded	/10.20 2.4	IV III/III EXIIItation over Sunace died

Discarded OutFlow Max=0.04 cfs @ 12.07 hrs HW=718.51' (Free Discharge) -5=Exfiltration (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.43 cfs @ 12.14 hrs HW=718.63' TW=704.77' (Dynamic Tailwater)

-1=Culvert (Passes 0.43 cfs of 1.72 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.26 cfs @ 5.40 fps)

-4=Orifice/Grate (Orifice Controls 0.17 cfs @ 3.42 fps)

Summary for Pond RG15L: Rain Garden 15L

Inflow Area =	8,190 sf, 65.00% Impervio	us, Inflow Depth > 2.20" for 2YearMass event					
Inflow =	0.50 cfs @ 12.07 hrs, Volume	e= 1,504 cf					
Outflow =	0.41 cfs @ 12.13 hrs, Volum	e= 1,435 cf, Atten= 18%, Lag= 3.2 min					
Discarded =	0.03 cfs @ 12.08 hrs, Volum	e= 746 cf					
Primary =	0.38 cfs @ 12.13 hrs, Volum	e= 689 cf					
Routed to Pond IB 7.1 : Infiltration Basin							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 709.84' @ 12.13 hrs Surf.Area= 480 sf Storage= 327 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 68.5 min (873.5 - 805.0)

AP7

Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 14

Volume	Invert	Avail.Storage	Storage Description
#1	706.75'	47 cf	
#2	706.75'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	709.75'	30 cf	
#4	710.00'	240 cf	
		586 cf	Total Available Storage
Device	Routing	Invert Out	tlet Devices
#1	Primary		" Round Culvert
			10.0' CPP, square edge headwall, Ke= 0.500
			et / Outlet Invert= 708.75' / 708.65' S= 0.0100 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		"Horiz. Orifice/Grate C= 0.600
			ited to weir flow at low heads
#3	Device 1	708.75' 2.0	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	709.25' 4.0	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	706.75' 2.4	10 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.03 cfs @ 12.08 hrs HW=709.76' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.38 cfs @ 12.13 hrs HW=709.84' TW=704.72' (Dynamic Tailwater)

-1=Culvert (Passes 0.38 cfs of 0.87 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.11 cfs @ 4.84 fps)

-4=Orifice/Grate (Orifice Controls 0.27 cfs @ 3.14 fps)

Summary for Pond RG16: Rain Garden Lot 16

Inflow Area =	6,745 sf, 65.00% Impervious	, Inflow Depth > 2.20" for 2YearMass event
Inflow =	0.41 cfs @ 12.07 hrs, Volume=	1,239 cf
Outflow =	0.30 cfs @ 12.14 hrs, Volume=	1,209 cf, Atten= 26%, Lag= 4.1 min
Discarded =	0.01 cfs @ 10.84 hrs, Volume=	727 cf
Primary =	0.29 cfs @ 12.14 hrs, Volume=	482 cf
Routed to Pond	d IB 7.1 : Infiltration Basin	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 733.13' @ 12.14 hrs Surf.Area= 240 sf Storage= 305 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 90.9 min (895.8 - 805.0)

Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 15

\ <i>(</i> = 1,	1		Change Description
Volume	Invert	Avall.Storage	Storage Description
#1	730.25'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	730.25'	269 cf	
π ∠	100.20	200 01	720 cf Overall - 47 cf Embedded = 673 cf \times 40.0% Voids
що	700 051	20 of	
#3	733.25'	30 cf	
			60 cf Overall x 50.0% Voids
#4	733.50'	240 cf	4.00'W x 60.00'L x 1.00'H Ponding
		586 cf	Total Available Storage
			· · · · · · · · · · · · · · · · · · ·
Device	Routing	Invert Out	let Devices
-	0		
#1	Primary	••••	' Round Culvert
		L= 6	5.0' CPP, square edge headwall, Ke= 0.500
		Inle	t / Outlet Invert= 732.25' / 732.00' S= 0.0417 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		'Horiz. Orifice/Grate C= 0.600
π∠	Device		
	— · · ·		ited to weir flow at low heads
#3	Device 1	-	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	732.75' 4.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	730.25' 2.4 1	0 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.01 cfs @ 10.84 hrs HW=730.29' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.29 cfs @ 12.14 hrs HW=733.13' TW=704.78' (Dynamic Tailwater)

—1=Culvert (Passes 0.29 cfs of 0.75 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.09 cfs @ 4.30 fps)

-4=Orifice/Grate (Orifice Controls 0.20 cfs @ 2.24 fps)

Summary for Pond RG21: Rain Garden 21

Inflow Area =	7,915 sf	, 65.00% Impervious,	Inflow Depth > 1.79"	for 2YearMass event
Inflow =	0.40 cfs @	12.07 hrs, Volume=	1,181 cf	
Outflow =	0.26 cfs @	12.16 hrs, Volume=	1,146 cf, Atten	= 34%, Lag= 5.0 min
Discarded =	0.01 cfs @	11.26 hrs, Volume=	684 cf	
Primary =	0.25 cfs @	12.16 hrs, Volume=	462 cf	
Routed to Link	P7 : AP7			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 727.56' @ 12.16 hrs Surf.Area= 240 sf Storage= 289 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 89.9 min (914.0 - 824.0)

Type III 24-hr 2YearMass Rainfall=3.24" Printed 5/1/2023

Page 16

		E l'oumaoo l'ium
Prepared by Mcclure Engineeering		Printeo
HvdroCAD® 10.20-2g s/n 03362 © 2022 HvdroCAD Software Sol	utions LLC	

Volume	Invert	Avail.Storage	Storage Description
#1	724.75'	31 cf	12.0" Round Pipe Storage Inside #2
			L= 40.0'
#2	724.75'	275 cf	6.00'W x 40.00'L x 3.00'H Soil Media and Gravel
	•		720 cf Overall - 31 cf Embedded = 689 cf \times 40.0% Voids
#3	727.75'	30 cf 6.00'W x 40.00'L x 0.25'H Mulch	
110	121.10	00 01	60 cf Overall x 50.0% Voids
#4	728.00'	337 of	6.00'W x 40.00'L x 1.00'H Ponding Z=2.0
	720.00		
		674 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary	726.75' 6.0'	' Round Culvert
	J		10.0' CPP, square edge headwall, Ke= 0.500
			t / Outlet Invert= 726.75' / 726.65' S= 0.0100 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		Horiz. Orifice/Grate C= 0.600
#2	Device I		
			ited to weir flow at low heads
#3	Device 1	-	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	727.25' 4.0'	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	724.75' 2.4 '	I0 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.01 cfs @ 11.26 hrs HW=724.79' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.25 cfs @ 12.16 hrs HW=727.56' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 0.25 cfs of 0.67 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.09 cfs @ 4.10 fps)

-4=Orifice/Grate (Orifice Controls 0.16 cfs @ 1.89 fps)

Summary for Pond RG22-25: Rain Gardens 22-25

Inflow Area =	99,920 sf, 3	2.59% Impervious,	Inflow Depth > 1.36" for 2YearMass eve	ent
Inflow =	2.88 cfs @ 12	2.18 hrs, Volume=	11,346 cf	
Outflow =	2.38 cfs @ 12	2.29 hrs, Volume=	11,303 cf, Atten= 17%, Lag= 6.1 mi	in
Primary =	1.78 cfs @ 12	2.29 hrs, Volume=	8,477 cf	
Routed to Pond	d IB 7.1 : Infiltrati	ion Basin		
Secondary =	0.59 cfs @ 12	2.29 hrs, Volume=	2,826 cf	
Routed to Link	P7 : AP7			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 726.45' @ 12.29 hrs Surf.Area= 960 sf Storage= 767 cf

Plug-Flow detention time= 6.5 min calculated for 11,298 cf (100% of inflow) Center-of-Mass det. time= 4.2 min (854.3 - 850.1)

Postdevelopment AP7 Type III 24-hr 2YearMass Rainfall=3.24" Prepared by Mcclure Engineeering Printed 5/1/2023 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 17 Avail.Storage Storage Description Volume Invert #1 724.75' 12.0" Round Pipe Storage x 4 Inside #2 188 cf L = 60.0'#2 724.75' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 4 1,077 cf 2,880 cf Overall - 188 cf Embedded = 2,692 cf \times 40.0% Voids #3 727.75 120 cf 4.00'W x 60.00'L x 0.25'H Mulch x 4 240 cf Overall x 50.0% Voids #4 728.00' 1,493 cf 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 4 2,878 cf Total Available Storage Device Routing Invert Outlet Devices #1 Primary 724.75' 8.0" Round Culvert X 3.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 724.75' / 724.75' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf #2 Secondary 724.75 8.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 724.75' / 724.75' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf 6.0" Horiz. Orifice/Grate X 6.00 C= 0.600 #3 Device 1 728.50' Limited to weir flow at low heads #4 Device 2 728.50 6.0" Horiz. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads #5 Device 1 724.75' 3.0" Vert. Orifice/Grate X 6.00 C= 0.600 Limited to weir flow at low heads #6 Device 2 724.75' 3.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads #7 Device 1 727.25' 3.0" Vert. Orifice/Grate X 6.00 C= 0.600 Limited to weir flow at low heads #8 Device 2 727.25' 3.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads Primary OutFlow Max=1.78 cfs @ 12.29 hrs HW=726.45' TW=705.24' (Dynamic Tailwater) **1=Culvert** (Passes 1.78 cfs of 5.90 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -5=Orifice/Grate (Orifice Controls 1.78 cfs @ 6.05 fps)

-7=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.59 cfs @ 12.29 hrs HW=726.45' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 0.59 cfs of 1.97 cfs potential flow) -4=Orifice/Grate (Controls 0.00 cfs)

-6=Orifice/Grate (Orifice Controls 0.59 cfs @ 6.05 fps)

-8=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond RG27: Rain Garden Lot 27

Inflow Area = 10,800 sf, 23.14% Impervious, Inflow Depth > 1.43" for 2YearMass event Inflow 0.36 cfs @ 12.14 hrs, Volume= 1.287 cf = 0.19 cfs @ 12.38 hrs, Volume= Outflow = 1,280 cf, Atten= 48%, Lag= 14.3 min Primary = 0.19 cfs @ 12.38 hrs, Volume= 1.280 cf Routed to Pond IB 7.1 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 757.11' @ 12.38 hrs Surf.Area= 240 sf Storage= 255 cf

Plug-Flow detention time= 17.9 min calculated for 1,280 cf (99% of inflow) Center-of-Mass det. time= 15.0 min (859.4 - 844.4)

Volume	Invert	Avail.Storage	Storage Description
#1	754.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	754.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	757.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	758.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices		
#1	Primary	754.75'	6.0" Round Culvert		
			L= 36.0' CPP, square edge headwall, Ke= 0.500		
			Inlet / Outlet Invert= 754.75' / 753.50' S= 0.0347 '/' Cc= 0.900		
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	758.50'	6.0" Horiz. Orifice/Grate C= 0.600		
			Limited to weir flow at low heads		
#3	Device 1	754.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#4	Device 1	757.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		

Primary OutFlow Max=0.19 cfs @ 12.38 hrs HW=757.11' TW=705.48' (Dynamic Tailwater)

-1=Culvert (Passes 0.19 cfs of 1.33 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.16 cfs @ 7.26 fps)

-4=Orifice/Grate (Orifice Controls 0.03 cfs @ 1.12 fps)

Summary for Pond RG28: Rain Garden Lot 28

Inflow Are	a =	28,665 sf,	9.64% Impervious,	Inflow Depth > 1.06" for 2YearMass event
Inflow	=	0.64 cfs @	12.17 hrs, Volume=	2,534 cf
Outflow	=	0.53 cfs @	12.27 hrs, Volume=	2,527 cf, Atten= 18%, Lag= 5.7 min
Primary	=	0.53 cfs @	12.27 hrs, Volume=	2,527 cf
Routed	to Pond	d IB 7.1 : Infiltr	ation Basin	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 755.13' @ 12.27 hrs Surf.Area= 240 sf Storage= 161 cf

Plug-Flow detention time= 5.1 min calculated for 2,527 cf (100% of inflow) Center-of-Mass det. time= 3.5 min (868.2 - 864.7)

Type III 24-hr 2YearMass Rainfall=3.24"

Printed 5/1/2023 Page 19

		HydroCAD® 10.20-2g	s/n 03362 © 2	022 HydroCAD	Software Solutions LLC	
--	--	--------------------	---------------	--------------	------------------------	--

Volume	Invert	Avail.Stor	age	Storage Description
#1				12.0" Round Pipe Storage Inside #2
#2	753.75'	26		L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	756.75'	3	-	4.00'W x 60.00'L x 0.25'H Mulch
#4	757.00'	37		60 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		72	20 cf	Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1	Primary	753.75'	L= 30 Inlet /	Round Culvert 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 753.75' / 753.50' S= 0.0083 '/' Cc= 0.900 013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	757.50'	6.0" I	Horiz. Orifice/Grate X 2.00 C= 0.600
#3	Device 1	753.75'	3.0" \	ed to weir flow at low heads Vert. Orifice/Grate X 2.00 C= 0.600 ed to weir flow at low heads
#4	Device 1	756.25'	-	Vert. Orifice/Grate X 2.00 C= 0.600 ed to weir flow at low heads

Primary OutFlow Max=0.53 cfs @ 12.27 hrs HW=755.13' TW=705.19' (Dynamic Tailwater) **1=Culvert** (Passes 0.53 cfs of 1.56 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.53 cfs @ 5.39 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond RG30: Rain Garden Lot 30

Inflow Are	a =	6,740 sf, 65.00% Impervious,	Inflow Depth > 2.20" for 2YearMass event
Inflow	=	0.41 cfs @ 12.07 hrs, Volume=	1,238 cf
Outflow	=	0.18 cfs @ 12.26 hrs, Volume=	1,233 cf, Atten= 57%, Lag= 11.2 min
Primary	=	0.18 cfs @ 12.26 hrs, Volume=	1,233 cf
Routed	to Pond	IB 7.1 : Infiltration Basin	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 730.09' @ 12.26 hrs Surf.Area= 240 sf Storage= 252 cf

Plug-Flow detention time= 18.1 min calculated for 1,232 cf (100% of inflow) Center-of-Mass det. time= 15.4 min (820.4 - 805.0)

AP7

Postdevelopment Type III 24-hr 2YearMass Rainfall=3.24"

Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 20

Volume	Invert	Avail.Storage	Storage Description
#1	U		
#2	727.75'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	730.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
#4	60 cf Overall x 50.0% Voids 731.00' 373 cf 4.00'W x 60.00'L x 1.00'H Ponding Z=2. (•••••••••••••••••••••••••••••••••••••••
		720 cf	Total Available Storage
Device	Routing	Invert Out	tlet Devices
#1	Primary	L= Inle	" Round Culvert 10.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 727.75' / 727.65' S= 0.0100 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	731.50' 6.0 '	"Horiz. Orifice/Grate C= 0.600
#3 #4	Device 1 Device 1	727.75' 2.0 '	ited to weir flow at low heads "Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads "Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
			26 hrs HW=730.09' TW=705.16' (Dynamic Tailwater)

1=Culvert (Passes 0.18 cfs of 1.37 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs) **3=Orifice/Grate** (Orifice Controls 0.16 cfs @ 7.23 fps)

-4=Orifice/Grate (Orifice Controls 0.02 cfs @ 0.99 fps)

Summary for Link P7: AP7

Inflow Area =	400,631 sf, 31.37% Impervious,	Inflow Depth > 0.42"	for 2YearMass event
Inflow =	1.40 cfs @ 12.72 hrs, Volume=	14,162 cf	
Primary =	1.40 cfs @ 12.72 hrs, Volume=	14,162 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P7.1: AP7 - To Wetland E

Runoff = 2.02 cfs @ 12.14 hrs, Volume= 8,280 cf, Depth> 1.06" Routed to Link P7 : AP7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Ar	rea (sf)	CN /	Adj Desc	ription		
	78,255	55	Woo	ds, Good, I	HSG B	
	13,515	61	>75%	6 Grass co	ver, Good, HSG B	
	1,560	98	Unco	onnected ro	oofs, HSG B	
9	93,330	57	56 Weig	hted Avera	age, UI Adjusted	
9	91,770			3% Perviou		
	1,560			% Impervio		
	1,560	100.00% Unconnected				
т.	1	0	V/.1	0	Description	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
4.3	50	0.3000	0.19		Sheet Flow,	
					Woods: Light underbrush n= 0.400 P2= 3.00"	
0.2	25	0.3000	2.74		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
4.1	245	0.0400	1.00		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
8.6	320	Total				

Summary for Subcatchment P7.10: Lot 27

Runoff	=	0.75 cfs @	12.14 hrs,	Volume=	2,638 cf,	Depth>	2.93"
Routed	I to Pond	RG27 : Rair	i Garden Lo	ot 27			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

	Area (sf)	CN	Description
*	3,845	90	Residential Lots, 65% imp, HSG C
	2,045	70	Woods, Good, HSG C
	940	89	Gravel roads, HSG C
	3,970	74	>75% Grass cover, Good, HSG C
	10,800	80	Weighted Average
	8,301		76.86% Pervious Area
	2,499		23.14% Impervious Area

			gineeering		Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023
HydroCA	D® 10.20-	2g s/n 03	362 © 202	2 HydroCAL	Software Solutions LLC Page 22
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.7	50	0.0400	0.09		Sheet Flow,
0.2	35	0.2000	3.13		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
9.9	85	Total			·
			Summa	ry for Su	bcatchment P7.11: Lot 28
Runoff Route	= ed to Pon		s @ 12.1 Rain Garc	6 hrs, Volu len Lot 28	Ime= 5,729 cf, Depth> 2.40"
			nod, UH=S Rainfall=		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
A	vrea (sf)	CN D	escription		
*	4,250			Lots, 65%	imp, HSG C
	18,600			od, HSG C	
	960 4,855		iravel road 75% Gras		ood, HSG C
	28,665		/eighted A		
	25,903 2,763			vious Area ervious Area	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.7	50	0.0400	0.09		Sheet Flow,
1.7	100	0.0400	1.00		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
0.2	30	0.2000	3.13		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
11.6	180	Total			
		S	Summary	/ for Sub	catchment P7.12: Lot 28-29
Runoff Route	= ed to Pon			7 hrs, Volu Garden Lots	
			nod, UH=S Rainfall=		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Δ	vrea (sf)	CN D	escription		

	Area (sf)	CN	Description
*	17,105	90	Residential Lots, 65% imp, HSG C
	5,987		35.00% Pervious Area
	11,118		65.00% Impervious Area

AP7Type III 24-hr10YearMass Rainfall=5.05"Prepared by Mcclure EngineeeringPrinted 5/1/2023HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLCPage 23
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
5.0 Direct Entry,
Summary for Subcatchment P7.13: Lot 30
Runoff = 0.71 cfs @ 12.07 hrs, Volume= 2,203 cf, Depth> 3.92" Routed to Pond RG30 : Rain Garden Lot 30
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"
Area (sf) CN Description
* 6,740 90 Residential Lots, 65% imp, HSG C
2,359 35.00% Pervious Area 4,381 65.00% Impervious Area
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/sec) (cfs)
5.0 Direct Entry,
Summary for Subcatchment P7.14: Clubhouse Parking Lot
Runoff = 0.93 cfs @ 12.07 hrs, Volume= 2,997 cf, Depth> 4.35" Routed to Pond CB1 : CB1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"
Area (sf) CN Description

A	rea (sf)	CN	Description							
	1,410	74 :	>75% Grass cover, Good, HSG C							
	750	98	Roofs, HSG C							
	6,100	98	Paved parking, HSG C							
	8,260	94	Weighted Average							
	1,410		17.07% Pervious Area							
	6,850	ł	82.93% Impervious Area							
Tc	Length	Slope		Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
					— • • — •					

5.0

Direct Entry,

Summary for Subcatchment P7.2: Lots 14-15

Runoff = 0.86 cfs @ 12.07 hrs, Volume= 2,676 cf, Depth> 3.92" Routed to Pond RG15L : Rain Garden 15L

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Postdevelopment *Type III 24-hr 10YearMass Rainfall=5.05"* Printed 5/1/2023 Solutions LLC Page 24

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

	A	rea (sf)	CN [Description							
*		8,190	90 F	Residential	esidential Lots, 65% imp, HSG C						
		2,867	35.00% Pervious Area								
		5,324	24 65.00% Impervious Area								
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	5.0	(ieet)	(10/11)	(11/360)	(013)	Direct Entry,					

Summary for Subcatchment P7.3: Lot 20/21

Runoff = 0.75 cfs @ 12.07 hrs, Volume= 2,250 cf, Depth> 3.41" Routed to Pond RG21 : Rain Garden 21

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN E	CN Description					
	7,915	85 1	85 1/8 acre lots, 65% imp, HSG B					
	2,770	3	35.00% Pervious Area					
	5,145	6	65.00% Impervious Area					
_		~		.	— • • • •			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.0					Direct Entry,			

Summary for Subcatchment P7.4: Lots 21-25

Runoff = 6.10 cfs @ 12.18 hrs, Volume= 23,625 cf, Depth> 2.84" Routed to Pond RG22-25 : Rain Gardens 22-25

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Area (sf)	CN	Description
38,735	90	1/8 acre lots, 65% imp, HSG C
11,360	85	1/8 acre lots, 65% imp, HSG B
49,505	70	Woods, Good, HSG C
320	74	Grass Paver, Good, HSG C
99,920	79	Weighted Average
67,358		67.41% Pervious Area
32,562		32.59% Impervious Area
	38,735 11,360 49,505 320 99,920 67,358	38,735 90 11,360 85 49,505 70 320 74 99,920 79 67,358 70

			gineeering 362 © 202		Postdevelopment <i>Type III 24-hr 10YearMass Rainfall=5.05"</i> Printed 5/1/2023 Software Solutions LLC Page 25
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.7	50	0.0400	0.09		Sheet Flow,
0.8	50	0.0400	1.00		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
2.6	230	0.0900	1.50		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
13.1	330	Total			

Summary for Subcatchment P7.5: Basin 1

Runoff	=	3.47 cfs @	12.10 hrs,	Volume=	11,141 cf,	Depth>	2.07"
Routed	to Pond	I IB 7.1 : Infilti	ration Basin			-	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

13,590 98 Water Surface, HSG B 1,860 98 Paved parking, HSG C 1,425 98 Paved parking, HSG B 28,270 55 Woods, Good, HSG B 8,160 74 >75% Grass cover, Good, HSG C 750 98 Roofs, HSG C 10,520 61 >75% Grass cover, Good, HSG B 64,575 70 Weighted Average
1,42598Paved parking, HSG B28,27055Woods, Good, HSG B8,16074>75% Grass cover, Good, HSG C75098Roofs, HSG C10,52061>75% Grass cover, Good, HSG B
28,270 55 Woods, Good, HSG B 8,160 74 >75% Grass cover, Good, HSG C 750 98 Roofs, HSG C 10,520 61 >75% Grass cover, Good, HSG B
8,160 74 >75% Grass cover, Good, HSG C 750 98 Roofs, HSG C 10,520 61 >75% Grass cover, Good, HSG B
750 98 Roofs, HSG C 10,520 61 >75% Grass cover, Good, HSG B
10,520 61 >75% Grass cover, Good, HSG B
64.575 70 Weighted Average
46,950 72.71% Pervious Area
17,625 27.29% Impervious Area
Tc Length Slope Velocity Capacity Description
(min) (feet) (ft/ft) (ft/sec) (cfs)
5.7 50 0.1500 0.15 Sheet Flow,
Woods: Light underbrush n= 0.400 P2= 3.00"
0.9 110 0.1500 1.94 Shallow Concentrated Flow,
Woodland Kv= 5.0 fps
6.6 160 Total

Summary for Subcatchment P7.6: Lots 17-20

Runoff	=	2.40 cfs @	12.07 hrs,	Volume=	7,307 cf,	Depth> 3	3.61"
Routed	I to Pond	RG-16/20 : I	Rain Garder	n Lots 17-20			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Postdevelopment "Type III 24-hr 10YearMass Rainfall=5.05 Printed 5/1/2023

Page 26

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Area (sf)	CN	Description						
11,200	90	0 1/8 acre lots, 65% imp, HSG C						
13,081	85	1/8 acre lots, 65% imp, HSG B						
24,281	87	37 Weighted Average						
8,498	35.00% Pervious Area							
15,783		65.00% Impervious Area						
Tc Length (min) (feet)	Slop (ft/l		Capacity (cfs)	Description				
5.0				Direct Entry,				
	Summary for Subcatchment P7.7: Lot 15							

Runoff = 1.11 cfs @ 12.07 hrs, Volume= 3,431 cf, Depth> 3.92" Routed to Pond RG15H : Rain Garden 15H

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

	A	rea (sf)	CN I	Description							
*		10,500	90 I	Residential Lots, 65% imp, HSG C							
		3,675	35.00% Pervious Area								
		6,825	5 65.00% Impervious Area								
	-										
	Tc	Length	Slope	,	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0		Direct Entry,								

Summary for Subcatchment P7.8: Lot 16

Runoff	=	0.71 cfs @	12.07 hrs,	Volume=	2,204 cf,	Depth>	3.92"
Routed	to Pond	RG16 : Rain	Garden Lo	t 16			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

A	rea (sf)	CN [escription						
	6,745	90 F	Residential Lots, 65% imp, HSG C						
	2,361	3	5.00% Per	vious Area					
	4,384	6	65.00% Impervious Area						
Тс	l enath	Slope	Velocity	Capacity	Description				
min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry,				
	Tc min)	2,361 4,384 Tc Length min) (feet)	6,745 90 F 2,361 3 4,384 6 Tc Length Slope min) (feet) (ft/ft)	6,745 90 Residential 2,361 35.00% Per 4,384 65.00% Imp Tc Length Slope Velocity min) (feet) (ft/ft) (ft/sec)	6,745 90 Residential Lots, 65% 2,361 35.00% Pervious Area 4,384 65.00% Impervious Ar Tc Length Slope Velocity Capacity min) (feet) (ft/ft) (ft/sec) (cfs)				

Summary for Subcatchment P7.9: Lots 26-27

Runoff = 1.44 cfs @ 12.07 hrs, Volume= 4,446 cf, Depth> 3.92" Routed to Pond RG-26/27 : Rain Garden Lots 26-27

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Area (sf)	CN	CN Description						
* 13,605	90	90 Residential Lots, 65% imp, HSG C						
4,762		35.00% Pervious Area						
8,843		65.00% Impervious Area						
Tc Length (min) (feet)			Capacity (cfs)	Description				
5.0				Direct Entry,				
		-						

Summary for Pond CB1: CB1

 Inflow Area =
 8,260 sf, 82.93% Impervious, Inflow Depth > 4.35" for 10YearMass event

 Inflow =
 0.93 cfs @
 12.07 hrs, Volume=
 2,997 cf

 Outflow =
 0.93 cfs @
 12.07 hrs, Volume=
 2,997 cf, Atten= 0%, Lag= 0.0 min

 Primary =
 0.93 cfs @
 12.07 hrs, Volume=
 2,997 cf

 Routed to Pond IB 7.1 : Infiltration Basin
 2,997 cf
 2,997 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 727.52' @ 12.07 hrs Flood Elev= 730.00'

#1 Primary 727.00' 12.0" Round Culvert L= 40.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 727.00' / 726.60' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=0.93 cfs @ 12.07 hrs HW=727.52' TW=705.61' (Dynamic Tailwater) -1=Culvert (Barrel Controls 0.93 cfs @ 3.32 fps)

Summary for Pond IB 7.1: Infiltration Basin

Inflow Area =	299,386 sf	, 39.73% Impervious	, Inflow Depth > 2.60" for 10YearMass event
Inflow =	15.52 cfs @	12.14 hrs, Volume=	64,853 cf
Outflow =	4.43 cfs @	12.70 hrs, Volume=	57,978 cf, Atten= 71%, Lag= 33.6 min
Discarded =	0.57 cfs @	12.70 hrs, Volume=	22,575 cf
Primary =	3.86 cfs @	12.70 hrs, Volume=	35,403 cf
Routed to Link	(P7 : AP7		
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0 cf
Routed to Link	(P7 : AP7		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 707.24' @ 12.70 hrs Surf.Area= 10,259 sf Storage= 24,154 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 73.1 min (893.5 - 820.4)

Volume	Invert	Avail.Sto	orage	Storage Description		
#1	704.00'	704.00' 58,84		cf Custom Stage Data (Irregular)Listed below (Recalc)		
Elevatio (fee		f.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
704.0 706.0 708.0 710.0)0)0)0 1	4,675 8,235 1,620	575.0 615.0 650.0 680.0	0 12,743 19,758 26,342	0 12,743 32,501 58,843	4,675 8,645 12,389 15,831
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	704.00'	2.41	0 in/hr Exfiltration o	ver Surface area	
#2	Secondary	709.00'		long x 10.0' bread		
#3	Primary	704.00'	Coef 18.0 L= 3 Inlet	d (feet) 0.20 0.40 0. f. (English) 2.49 2.56 " Round Culvert 5.0' CPP, square ec / Outlet Invert= 704.0	6 2.70 2.69 2.68 Ige headwall, Ke= 00' / 704.00' S= 0.	2.69 2.67 2.64 0.500 0000 '/' Cc= 0.900
#4	Device 3	709.00'	24.0	.013 Corrugated PE, " Horiz. Orifice/Grat ied to weir flow at low	e C= 0.600	low Area= 1.77 st
#5	Device 3	704.00'	Cus t Head	tom Weir/Orifice, Cv d (feet) 0.00 1.40 1.	= 2.62 (C= 3.28) 40 2.00 3.00 3.2	5 3.25 4.00 4.00 5.00
#6	Device 3	705.40'				25 0.50 0.50 0.66 0.66 I to weir flow at low heads

Discarded OutFlow Max=0.57 cfs @ 12.70 hrs HW=707.24' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.57 cfs)

Primary OutFlow Max=3.86 cfs @ 12.70 hrs HW=707.24' TW=0.00' (Dynamic Tailwater)

-3=Culvert (Passes 3.86 cfs of 12.77 cfs potential flow)

4=Orifice/Grate (Controls 0.00 cfs)

-5=Custom Weir/Orifice (Weir Controls 2.04 cfs @ 4.44 fps)

-6=Orifice/Grate (Orifice Controls 1.82 cfs @ 5.94 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=704.00' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG-16/20: Rain Garden Lots 17-20

Inflow Area = 24,281 sf, 65.00% Impervious, Inflow Depth > 3.61" for 10YearMass event 2.40 cfs @ 12.07 hrs, Volume= Inflow 7,307 cf = Outflow = 1.80 cfs @ 12.14 hrs, Volume= 7,274 cf, Atten= 25%, Lag= 3.9 min = 1.80 cfs @ 12.14 hrs, Volume= Primary 7,274 cf Routed to Pond IB 7.1 : Infiltration Basin

		Postdevelopment
AP7	Type III 24-hr	10YearMass Rainfall=5.05"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC	Page 29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 727.09' @ 12.14 hrs Surf.Area= 960 sf Storage= 1,205 cf

Plug-Flow detention time= 16.7 min calculated for 7,274 cf (100% of inflow) Center-of-Mass det. time= 13.8 min (813.2 - 799.4)

Volume	Invert	Avail.Storage	Storage Description
#1	724.25'	188 cf	12.0" Round Pipe Storage x 4 Inside #2
			L= 60.0'
#2	724.25'	1,077 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 4
			2,880 cf Overall - 188 cf Embedded = 2,692 cf x 40.0% Voids
#3	727.25'	120 cf	4.00'W x 60.00'L x 0.25'H Mulch × 4
			240 cf Overall x 50.0% Voids
#4	727.50'	1,493 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 4
		2,878 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	724.25'	6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 724.25' / 724.25' S= 0.0000 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	728.00'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	724.25'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	726.50'	4.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=1.80 cfs @ 12.14 hrs HW=727.09' TW=706.01' (Dynamic Tailwater)

-**1=Culvert** (Passes 1.80 cfs of 6.09 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.70 cfs @ 8.00 fps)

-4=Orifice/Grate (Orifice Controls 1.10 cfs @ 3.14 fps)

Summary for Pond RG-26/27: Rain Garden Lots 26-27

Inflow Are	a =	13,605 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event
Inflow	=	1.44 cfs @ 12.07 hrs, Volume=	4,446 cf
Outflow	=	1.10 cfs @ 12.13 hrs, Volume=	4,432 cf, Atten= 23%, Lag= 3.7 min
Primary	=	1.10 cfs @ 12.13 hrs, Volume=	4,432 cf
Routed	l to Pond	IB 7.1 : Infiltration Basin	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 747.42' @ 12.13 hrs Surf.Area= 960 sf Storage= 674 cf

Plug-Flow detention time= 15.2 min calculated for 4,430 cf (100% of inflow) Center-of-Mass det. time= 13.2 min (802.2 - 789.0)

Type III 24-hr 10YearMass Rainfall=5.05"

Printed 5/1/2023 Page 30

				-
Volume	Invert	Avail.Storage		Storage Description
#1	744.25'	ę	94 cf	12.0" Round Pipe Storage x 2 Inside #2
				L= 60.0'
#2	744.25'	53	38 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 2
				1,440 cf Overall - 94 cf Embedded = 1,346 cf x 40.0% Voids
#3	747.25'	60 cf		4.00'W x 60.00'L x 0.25'H Mulch x 2
				120 cf Overall x 50.0% Voids
#4	747.50'	747 cf		4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 2
		1,43	39 cf	Total Available Storage
		,		5
Device	Routing	Invert	Outl	et Devices
#1	Primary	744.25'	6.0"	Round Culvert X 2.00
			L= 1	0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 744.25' / 744.15' S= 0.0100 '/' Cc= 0.900
			n= 0	.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	748.00'		Horiz. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads
#3	Device 1	744.25'	2.0"	Vert. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads
#4	Device 1	746.50'	4.0"	Vert. Orifice/Grate X 2.00 C= 0.600
			-	ted to weir flow at low heads

Primary OutFlow Max=1.10 cfs @ 12.13 hrs HW=747.42' TW=705.98' (Dynamic Tailwater)

-1=Culvert (Passes 1.10 cfs of 3.23 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.37 cfs @ 8.46 fps)

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

-4=Orifice/Grate (Orifice Controls 0.73 cfs @ 4.19 fps)

Summary for Pond RG-28/29: Rain Garden Lots 28-29

 Inflow Area =
 17,105 sf, 65.00% Impervious, Inflow Depth > 3.92" for 10YearMass event

 Inflow =
 1.80 cfs @
 12.07 hrs, Volume=
 5,590 cf

 Outflow =
 1.25 cfs @
 12.15 hrs, Volume=
 5,574 cf, Atten= 31%, Lag= 4.5 min

 Primary =
 1.25 cfs @
 12.15 hrs, Volume=
 5,574 cf

 Routed to Pond IB 7.1 : Infiltration Basin
 5,574 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 743.22' @ 12.15 hrs Surf.Area= 1,557 sf Storage= 813 cf

Plug-Flow detention time= 14.7 min calculated for 5,574 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (801.8 - 789.0)

AP7

Type III 24-hr 10YearMass Rainfall=5.05"

Printed 5/1/2023

	D® 10.20-2g s			HydroCAD Software Solutions LLC Page 3
Volume	Invert	Avail.Sto	rage	Storage Description
#1	739.75'	ç	94 cf	12.0" Round Pipe Storage x 2 Inside #2 L= 60.0'
#2	739.75'	538 cf		
#3	742.75'	60 cf		•
#4	743.00'	74	l7 cf	
		1,43	89 cf	Total Available Storage
Device	Routing	Invert	Outl	let Devices
#1	Primary	739.75'	6.0" Round Culvert X 2.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 739.75' / 739.65' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf	
#2	Device 1	743.50'	\mathbf{c}	
#3	Device 1	739.75'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600	

Limited to weir flow at low heads
742.00' 4.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.25 cfs @ 12.15 hrs HW=743.22' TW=706.06' (Dynamic Tailwater)

-1=Culvert (Passes 1.25 cfs of 3.40 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.39 cfs @ 8.87 fps)

-4=Orifice/Grate (Orifice Controls 0.86 cfs @ 4.95 fps)

Summary for Pond RG15H: Rain Garden 15H

10,500 sf, 65.00% Impervious, Inflow Depth > 3.92" for 10YearMass event Inflow Area = Inflow = 1.11 cfs @ 12.07 hrs, Volume= 3.431 cf Outflow 1.00 cfs @ 12.11 hrs, Volume= 3,224 cf, Atten= 10%, Lag= 2.2 min = Discarded = 0.04 cfs @ 11.98 hrs, Volume= 897 cf 0.96 cfs @ 12.11 hrs, Volume= Primary = 2,327 cf Routed to Pond IB 7.1 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 719.15' @ 12.11 hrs Surf.Area= 720 sf Storage= 503 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 19.2 min (808.2 - 789.0)

AP7

#4

Device 1

Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 32

Volume	Invert	Avail.Storage	Storage Description
#1	715.25'	47 cf	
#2	715.25'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	718.25'	30 cf	
#4	718.50'	240 cf	
		586 cf	Total Available Storage
Device	Routing	Invert Out	let Devices
#1	Primary		' Round Culvert 10.0' CPP, square edge headwall, Ke= 0.500
		Inle	t / Outlet Invert= 717.25' / 717.15' S= 0.0100 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	719.00' 8.0'	'Horiz. Orifice/Grate C= 0.600
#3	Device 1		ited to weir flow at low heads ' Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	715.25' 2.4 ′	10 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.04 cfs @ 11.98 hrs HW=718.52' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.96 cfs @ 12.11 hrs HW=719.15' TW=705.83' (Dynamic Tailwater)

-1=Culvert (Passes 0.96 cfs of 2.10 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.40 cfs @ 1.27 fps)

-3=Orifice/Grate (Orifice Controls 0.32 cfs @ 6.42 fps)

-4=Orifice/Grate (Orifice Controls 0.24 cfs @ 4.88 fps)

Summary for Pond RG15L: Rain Garden 15L

Inflow Area =	8,190 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event		
Inflow =	0.86 cfs @ 12.07 hrs, Volume=	2,676 cf		
Outflow =	0.59 cfs @ 12.15 hrs, Volume=	2,506 cf, Atten= 31%, Lag= 4.6 min		
Discarded =	0.04 cfs @ 12.03 hrs, Volume=	859 cf		
Primary =	0.55 cfs @ 12.15 hrs, Volume=	1,647 cf		
Routed to Pond IB 7.1 : Infiltration Basin				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 710.42' @ 12.15 hrs Surf.Area= 720 sf Storage= 447 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 29.1 min (818.1 - 789.0)

Type III 24-hr 10YearMass Rainfall=5.05"

Printed 5/1/2023 Page 33

Volume	Invert	Avail.Storage	Storage Description		
#1	706.75'	47 cf	12.0" Round Pipe Storage Inside #2		
			L= 60.0'		
#2	706.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel		
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids		
#3	709.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch		
			60 cf Overall x 50.0% Voids		
#4	710.00'	240 cf	4.00'W x 60.00'L x 1.00'H Ponding		
		586 cf	Total Available Storage		
			^c		
Device	Routing	Invert Out	let Devices		
#1	Primary	708.75' 6.0 '	.0" Round Culvert		
	2	L= ^	L= 10.0' CPP, square edge headwall, Ke= 0.500		
		Inle	Inlet / Outlet Invert= 708.75' / 708.65' S= 0.0100 '/' Cc= 0.900		
		n= (n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	710.50' 6.0 '	6.0" Horiz. Orifice/Grate C= 0.600		
		Lim	ited to weir flow at low heads		
#3	Device 1	708.75' 2.0 '	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#4	Device 1	709.25' 4.0 '	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#5	Discarded	706.75' 2.4 1	0 in/hr Exfiltration over Surface area		

Discarded OutFlow Max=0.04 cfs @ 12.03 hrs HW=710.02' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Primary OutFlow Max=0.55 cfs @ 12.15 hrs HW=710.42' TW=706.07' (Dynamic Tailwater)

-1=Culvert (Passes 0.55 cfs of 1.13 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.13 cfs @ 6.06 fps)

-4=Orifice/Grate (Orifice Controls 0.42 cfs @ 4.82 fps)

Summary for Pond RG16: Rain Garden Lot 16

Inflow Area =	6,745 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event		
Inflow =	0.71 cfs @ 12.07 hrs, Volume=	2,204 cf		
Outflow =	0.53 cfs @ 12.14 hrs, Volume=	2,074 cf, Atten= 25%, Lag= 3.9 min		
Discarded =	0.04 cfs @ 12.06 hrs, Volume=	833 cf		
Primary =	0.49 cfs @ 12.14 hrs, Volume=	1,241 cf		
Routed to Pond IB 7.1 : Infiltration Basin				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 733.69' @ 12.14 hrs Surf.Area= 720 sf Storage= 391 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 40.4 min (829.3 - 789.0)

AP7

Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 34

Volume	Invert	Avail.Storage	Storage Description		
#1	730.25'	47 cf			
			L= 60.0'		
#2	730.25'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel		
#3	733.25'	30 cf	720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids 4.00'W x 60.00'L x 0.25'H Mulch		
110	100.20	00 01	60 cf Overall x 50.0% Voids		
#4	733.50'	240 cf	4.00'W x 60.00'L x 1.00'H Ponding		
		586 cf	Total Available Storage		
Device	Routing	Invert Outl	et Devices		
#1	Primary	732.25' 6.0"	' Round Culvert		
		L= 6	6.0' CPP, square edge headwall, Ke= 0.500		
		Inlet	let / Outlet Invert= 732.25' / 732.00' S= 0.0417 '/' Cc= 0.900		
		n= 0	n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1		6		
		Limi	ted to weir flow at low heads		
#3	Device 1	732.25' 2.0"	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#4	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#5	Discarded	-	0 in/hr Exfiltration over Surface area		

Discarded OutFlow Max=0.04 cfs @ 12.06 hrs HW=733.53' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.49 cfs @ 12.14 hrs HW=733.69' TW=706.00' (Dynamic Tailwater)

—1=Culvert (Passes 0.49 cfs of 1.03 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.12 cfs @ 5.60 fps)

-4=Orifice/Grate (Orifice Controls 0.37 cfs @ 4.23 fps)

Summary for Pond RG21: Rain Garden 21

Inflow Area = 7,915 sf, 65.00% Impervious, Inflow Depth > 3.41" for 10YearMass event Inflow 0.75 cfs @ 12.07 hrs, Volume= 2.250 cf = 0.54 cfs @ 12.14 hrs, Volume= Outflow = 2,105 cf, Atten= 27%, Lag= 4.2 min Discarded = 0.04 cfs @ 12.14 hrs, Volume= 791 cf 0.50 cfs @ 12.14 hrs, Volume= 1,314 cf Primary = Routed to Link P7 : AP7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 728.22' @ 12.14 hrs Surf.Area= 761 sf Storage= 393 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 34.0 min (839.6 - 805.7)

Type III 24-hr 10YearMass Rainfall=5.05"

Printed 5/1/2023 Page 35

HydroCAD® 10.20-20	s/n 03362	© 2022 HydroC	AD Software	e Solutions LLC	

Volume	Invert	Avail.Storage	Storage Description		
#1	724.75'	31 cf			
#2	724.75'	275 cf	L= 40.0' 6.00'W x 40.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 31 cf Embedded = 689 cf x 40.0% Voids		
#3	727.75'	30 cf	6.00'W x 40.00'L x 0.25'H Mulch		
#4	728.00'	337 cf	60 cf Overall x 50.0% Voids 6.00'W x 40.00'L x 1.00'H Ponding Z=2.0		
		674 cf	674 cf Total Available Storage		
Device	Routing	Invert Out	tlet Devices		
#1	Primary)" Round Culvert		
		L=	: 10.0' CPP, square edge headwall, Ke= 0.500		
		Inle	let / Outlet Invert= 726.75' / 726.65' S= 0.0100 '/' Cc= 0.900		
		n=	= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1		6.0" Horiz. Orifice/Grate C= 0.600		
		Lim	nited to weir flow at low heads		
#3	Device 1	726.75' 2.0	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#4	Device 1	-	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#5	Discarded	-	10 in/hr Exfiltration over Surface area		

Discarded OutFlow Max=0.04 cfs @ 12.14 hrs HW=728.22' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.50 cfs @ 12.14 hrs HW=728.22' TW=0.00' (Dynamic Tailwater)

1=Culvert (Passes 0.50 cfs of 1.04 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.12 cfs @ 5.66 fps)

-4=Orifice/Grate (Orifice Controls 0.38 cfs @ 4.31 fps)

Summary for Pond RG22-25: Rain Gardens 22-25

Inflow Area =	99,920 sf, 32.59% Impervious,	Inflow Depth > 2.84" for 10YearMass event
Inflow =	6.10 cfs @ 12.18 hrs, Volume=	23,625 cf
Outflow =	5.22 cfs @ 12.26 hrs, Volume=	23,569 cf, Atten= 14%, Lag= 4.9 min
Primary =	3.92 cfs @ 12.26 hrs, Volume=	17,677 cf
Routed to Pond	d IB 7.1 : Infiltration Basin	
Secondary =	1.31 cfs @ 12.26 hrs, Volume=	5,892 cf
Routed to Link	P7 : AP7	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 728.24' @ 12.26 hrs Surf.Area= 3,125 sf Storage= 1,640 cf

Plug-Flow detention time= 5.5 min calculated for 23,559 cf (100% of inflow) Center-of-Mass det. time= 4.1 min (833.1 - 829.0)

AP7

	ed by Mcclure D® 10.20-2g s/	0	0	Postdevelopment <i>Type III 24-hr 10YearMass Rainfall=5.05</i> " Printed 5/1/2023 IydroCAD Software Solutions LLC Page 36				
Volume	Invert	Avail Sto	rana	Storage Description				
#1	724.75'		38 cf					
	704 751	4.0-		L= 60.0'				
#2	724.75'	1,07	// CT	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 4 2,880 cf Overall - 188 cf Embedded = 2,692 cf x 40.0% Voids				
#3	727.75'	12	20 cf	4.00'W x 60.00'L x 0.25'H Mulch x 4				
#4	728.00'	1 /0	D3 cf	240 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 4				
<u></u>	720.00			Total Available Storage				
. .		-		C C C C C C C C C C C C C C C C C C C				
Device	Routing			et Devices				
#1 #2	Primary Secondary	724.75' 724.75'	L= 1 Inlet n= 0 8.0" L= 1 Inlet	Round Culvert X 3.00 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 724.75' / 724.75' S= 0.0000 '/' Cc= 0.900 .013 Corrugated PE, smooth interior, Flow Area= 0.35 sf Round Culvert 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 724.75' / 724.75' S= 0.0000 '/' Cc= 0.900				
#3	Device 1	728.50'	6.0"	.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf Horiz. Orifice/Grate X 6.00 C= 0.600 ted to weir flow at low heads				
#4	Device 2	728.50'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads				
#5	Device 1	724.75'	3.0"	Vert. Orifice/Grate X 6.00 C= 0.600 ted to weir flow at low heads				
#6	Device 2	724.75'	3.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads				
#7	Device 1	727.25'	3.0"	Vert. Orifice/Grate X 6.00 C= 0.600				
#8	Device 2	727.25'	3.0"	ted to weir flow at low heads Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads				
1=Cu 1−3= 1−5=	Primary OutFlow Max=3.91 cfs @ 12.26 hrs HW=728.24' TW=706.63' (Dynamic Tailwater) 1=Culvert (Passes 3.91 cfs of 8.95 cfs potential flow) -3=Orifice/Grate (Controls 0.00 cfs) -5=Orifice/Grate (Orifice Controls 2.60 cfs @ 8.83 fps) -7=Orifice/Grate (Orifice Controls 1.32 cfs @ 4.47 fps)							

Secondary OutFlow Max=1.30 cfs @ 12.26 hrs HW=728.24' TW=0.00' (Dynamic Tailwater) -2=Culvert (Passes 1.30 cfs of 2.98 cfs potential flow) -4=Orifice/Grate (Controls 0.00 cfs) -6=Orifice/Grate (Orifice Controls 0.87 cfs @ 8.83 fps)

8=Orifice/Grate (Orifice Controls 0.44 cfs @ 4.47 fps)

Summary for Pond RG27: Rain Garden Lot 27

 Inflow Area =
 10,800 sf, 23.14% Impervious, Inflow Depth > 2.93" for 10YearMass event

 Inflow =
 0.75 cfs @
 12.14 hrs, Volume=
 2,638 cf

 Outflow =
 0.59 cfs @
 12.22 hrs, Volume=
 2,629 cf, Atten= 21%, Lag= 5.2 min

 Primary =
 0.59 cfs @
 12.22 hrs, Volume=
 2,629 cf

 Routed to Pond IB 7.1 : Infiltration Basin
 2,629 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 758.07' @ 12.22 hrs Surf.Area= 738 sf Storage= 363 cf

Plug-Flow detention time= 14.8 min calculated for 2,629 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (836.6 - 823.8)

Volume	Invert	Avail.Storage	Storage Description
#1	754.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	754.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	757.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	758.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	754.75'	6.0" Round Culvert
			L= 36.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 754.75' / 753.50' S= 0.0347 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	758.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	754.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	757.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.59 cfs @ 12.22 hrs HW=758.07' TW=706.47' (Dynamic Tailwater)

-1=Culvert (Passes 0.59 cfs of 1.52 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.19 cfs @ 8.66 fps)

-4=Orifice/Grate (Orifice Controls 0.40 cfs @ 4.57 fps)

Summary for Pond RG28: Rain Garden Lot 28

Inflow Area	a =	28,665 sf,	9.64% Impervious,	Inflow Depth > 2.40"	for 10YearMass event
Inflow	=	1.53 cfs @	12.16 hrs, Volume=	5,729 cf	
Outflow	=	1.45 cfs @	12.20 hrs, Volume=	5,719 cf, Atte	n= 5%, Lag= 2.5 min
Primary	=	1.45 cfs @	12.20 hrs, Volume=	5,719 cf	
Routed	to Ponc	l IB 7.1 : Infiltr	ation Basin		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 756.97' @ 12.20 hrs Surf.Area= 480 sf Storage= 342 cf

Plug-Flow detention time= 4.5 min calculated for 5,719 cf (100% of inflow) Center-of-Mass det. time= 3.5 min (844.1 - 840.6)

Type III 24-hr 10YearMass Rainfall=5.05" Printed 5/1/2023

Prepared by Mcclure Engineeering	Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 38

Volume	Invert	Avail.Sto	rage	Storage Description
#1	753.75'	2	17 cf	
#2	753.75'	26	69 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	756.75'	3	30 cf	
#4	757.00'	37	73 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		72	20 cf	Total Available Storage
Device	Routing	Invert	Outl	et Devices
#1	Primary	753.75'		Round Culvert
				0.0' CPP, square edge headwall, Ke= 0.500
				: / Outlet Invert= 753.75' / 753.50' S= 0.0083 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1	757.50'		Horiz. Orifice/Grate X 2.00 C= 0.600
				ted to weir flow at low heads
#3	Device 1	753.75'	•••	Vert. Orifice/Grate X 2.00 C= 0.600
#4	Device 1	756.25'	4.0"	ted to weir flow at low heads Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads

Primary OutFlow Max=1.45 cfs @ 12.20 hrs HW=756.96' TW=706.37' (Dynamic Tailwater) **1=Culvert** (Passes 1.45 cfs of 2.65 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.83 cfs @ 8.46 fps)

-4=Orifice/Grate (Orifice Controls 0.62 cfs @ 3.56 fps)

Summary for Pond RG30: Rain Garden Lot 30

Inflow Are	a =	6,740 sf, 65.00% Impervious,	Inflow Depth > 3.92" for 10YearMass event				
Inflow	=	0.71 cfs @ 12.07 hrs, Volume=	2,203 cf				
Outflow	=	0.55 cfs @ 12.13 hrs, Volume=	2,196 cf, Atten= 23%, Lag= 3.7 min				
Primary	=	0.55 cfs @ 12.13 hrs, Volume=	2,196 cf				
Routed to Pond IB 7.1 : Infiltration Basin							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 730.91' @ 12.13 hrs Surf.Area= 480 sf Storage= 335 cf

Plug-Flow detention time= 15.3 min calculated for 2,196 cf (100% of inflow) Center-of-Mass det. time= 13.3 min (802.2 - 789.0)

AP7

Postdevelopment Type III 24-hr 10YearMass Rainfall=5.05"

Printed	5/1/2023

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 39

Volume	Invert	Avail.Storage	Storage Description				
#1	727.75'	47 cf	12.0" Round Pipe Storage Inside #2				
#2	727.75'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids				
#3	730.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch				
#4	731.00'	373 cf	60 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0				
		720 cf	Total Available Storage				
Device	Routing	Invert Out	let Devices				
#1	Primary	727.75' 6.0 '	" Round Culvert				
		L=	10.0' CPP, square edge headwall, Ke= 0.500				
		Inle	t / Outlet Invert= 727.75' / 727.65' S= 0.0100 '/' Cc= 0.900				
		n=	0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf				
#2	Device 1	731.50' 6.0 '	"Horiz. Orifice/Grate C= 0.600				
		Lim	ited to weir flow at low heads				
#3	Device 1	727.75' 2.0 '	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads				
#4	Device 1	730.00' 4.0 '	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads				
Primary	Primary OutFlow Max=0.55 cfs @ 12.13 hrs HW=730.91' TW=705.98' (Dynamic Tailwater)						

1=**Culvert** (Passes 0.55 cfs of 1.61 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs) **3=Orifice/Grate** (Orifice Controls 0.18 cfs @ 8.44 fps)

-4=Orifice/Grate (Orifice Controls 0.36 cfs @ 4.14 fps)

Summary for Link P7: AP7

Inflow Area =	400,631 sf, 31.37% Impervious, Ir	nflow Depth > 1.52"	for 10YearMass event
Inflow =	5.98 cfs @ 12.37 hrs, Volume=	50,889 cf	
Primary =	5.98 cfs @ 12.37 hrs, Volume=	50,889 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

AP7

Summary for Subcatchment P7.1: AP7 - To Wetland E

Runoff = 3.55 cfs @ 12.13 hrs, Volume= 13,219 cf, Depth> 1.70" Routed to Link P7 : AP7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

 A	rea (sf)	CN	Adj Desc	cription	
	78,255	55	Woo	ds, Good, I	HSG B
	13,515	61	>75%	6 Grass co	ver, Good, HSG B
	1,560	98	Unco	onnected ro	oofs, HSG B
	93,330	57	56 Weig	hted Avera	age, UI Adjusted
	91,770		98.3	3% Perviou	us Area
	1,560		1.67	% Impervic	ous Area
	1,560		100.	00% Uncor	nnected
Тс	Length	Slope	Velocity	Capacity	Description
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
4.3	50	0.3000	0.19		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
0.2	25	0.3000	2.74		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
4.1	245	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
8.6	320	Total			

Summary for Subcatchment P7.10: Lot 27

Runoff	=	1.00 cfs @	12.14 hrs,	Volume=	3,543 cf,	Depth>	3.94"
Routed	I to Pond	RG27 : Rain	i Garden Lo	ot 27			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

	Area (sf)	CN	Description					
*	3,845	90	Residential Lots, 65% imp, HSG C					
	2,045	70	Woods, Good, HSG C					
	940	89	Gravel roads, HSG C					
	3,970	74	>75% Grass cover, Good, HSG C					
	10,800	80	Weighted Average					
	8,301		76.86% Pervious Area					
	2,499		23.14% Impervious Area					

			gineeering 362 © 202		Postdevelopment <i>Type III 24-hr 25YearMass Rainfall=6.18"</i> Printed 5/1/2023 Software Solutions LLC Page 41				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
9.7 0.2	50 35	0.0400 0.2000	0.09 3.13		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
9.9	85	Total							
Summary for Subcatchment P7.11: Lot 28									
Runoff Route	Runoff = 2.14 cfs @ 12.16 hrs, Volume= 7,952 cf, Depth> 3.33" Routed to Pond RG28 : Rain Garden Lot 28								
			nod, UH=S s Rainfall=(ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs				
<u>A</u>	rea (sf)		escription	Lata 65%					
	4,250 18,600			od, HSG C	imp, HSG C				
	960	89 G	ravel road	ls, HSG C					
	4,855 28,665		/eighted A		ood, HSG C				
	25,903 2,763	9	0.36% Per	vious Area ervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
9.7	50	0.0400	0.09	X //	Sheet Flow,				
1.7	100	0.0400	1.00		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps				
0.2	30	0.2000	3.13		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps				
11.6	180	Total							
		S	Summary	/ for Sub	catchment P7.12: Lot 28-29				
Runoff Route	= ed to Pon			7 hrs, Volu Garden Lots					
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"									

 Area (sf)
 CN
 Description

 *
 17,105
 90
 Residential Lots, 65% imp, HSG C

 5,987
 35.00% Pervious Area

 11,118
 65.00% Impervious Area

AP7	Type III 24-hr 25YearMass Rainfall=6.18"								
Prepared by Mcclure Engineeering	Printed 5/1/2023								
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	olutions LLC Page 42								
Tc Length Slope Velocity Capacity Descriptic (min) (feet) (ft/ft) (ft/sec) (cfs)	n								
5.0 Direct En	try,								
Summary for Subcatchment P7.13: Lot 30									
Runoff = 0.90 cfs @ 12.07 hrs, Volume= Routed to Pond RG30 : Rain Garden Lot 30	2,818 cf, Depth> 5.02"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Tir Type III 24-hr 25YearMass Rainfall=6.18"	ne Span= 0.00-24.00 hrs, dt= 0.01 hrs								
Area (sf) CN Description									
* 6,740 90 Residential Lots, 65% imp, HSG 0	2								
2,359 35.00% Pervious Area									
4,381 65.00% Impervious Area									
Tc Length Slope Velocity Capacity Descriptic (min) (feet) (ft/ft) (ft/sec) (cfs)	'n								
5.0 Direct En	try,								
Summary for Subcatchment P7.14	4: Clubhouse Parking Lot								
Runoff = 1.16 cfs @ 12.07 hrs, Volume= Routed to Pond CB1 : CB1	3,766 cf, Depth> 5.47"								
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"									
Area (sf) CN Description									

A	rea (sf)	CN	Description						
	1,410	74	>75% Gras	s cover, Go	iood, HSG C				
	750	98	Roofs, HSC	ЭC					
	6,100	98	Paved park	ing, HSG C	С				
	8,260	94	Weighted Average						
	1,410		17.07% Pei	rvious Area	а				
	6,850		82.93% Imp	pervious Ar	rea				
-		01		A 11					
Tc	Length	Slope		Capacity					
(min)	(feet)	(ft/ft)	t) (ft/sec) (cfs)						
5.0					Direct Entry,				

Summary for Subcatchment P7.2: Lots 14-15

Runoff = 1.09 cfs @ 12.07 hrs, Volume= 3,425 cf, Depth> 5.02" Routed to Pond RG15L : Rain Garden 15L

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Postdevelopment *Type III 24-hr 25YearMass Rainfall=6.18"* Printed 5/1/2023 Solutions LLC Page 43

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

	A	rea (sf)	CN [Description							
*		8,190	90 F	Residential Lots, 65% imp, HSG C							
		2,867	3	35.00% Pervious Area							
		5,324	e	65.00% Impervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
	5.0	(ieet)	(10/11)	(11/360)	(013)	Direct Entry,					

Summary for Subcatchment P7.3: Lot 20/21

Runoff = 0.97 cfs @ 12.07 hrs, Volume= 2,948 cf, Depth> 4.47" Routed to Pond RG21 : Rain Garden 21

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN E	N Description							
	7,915	85 1	5 1/8 acre lots, 65% imp, HSG B							
	2,770	3	35.00% Pervious Area							
	5,145	6	5.00% Imp	pervious Are	ea					
Та	Longth	Slope	Voloaity	Consoity	Description					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0	(1001)	(1010)	(10000)	(010)	Direct Entry,					
0.0					Diroct Linky,					

Summary for Subcatchment P7.4: Lots 21-25

Runoff = 8.22 cfs @ 12.18 hrs, Volume= 31,902 cf, Depth> 3.83" Routed to Pond RG22-25 : Rain Gardens 22-25

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

	Area (sf)	CN	Description
	38,735	90	1/8 acre lots, 65% imp, HSG C
	11,360	85	1/8 acre lots, 65% imp, HSG B
	49,505	70	Woods, Good, HSG C
*	320	74	Grass Paver, Good, HSG C
	99,920	79	Weighted Average
	67,358		67.41% Pervious Area
	32,562		32.59% Impervious Area

AP7

AP7PostdevelopmenAP7Type III 24-hr 25YearMass Rainfall=6.18Prepared by Mcclure EngineeeringPrinted 5/1/2023										
HydroCA	D® 10.20-	2g_s/n 03	362 © 202	2 HydroCAD	Software Solutions LLC Page 44					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
9.7	50	0.0400	0.09		Sheet Flow,					
0.8	50	0.0400	1.00		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps					
2.6	230	0.0900	1.50		Shallow Concentrated Flow, Woodland Kv= 5.0 fps					
13.1	330	Total								

Summary for Subcatchment P7.5: Basin 1

Runoff	=	5.00 cfs @	12.10 hrs,	Volume=	15,846 cf,	Depth>	2.94"
Routed	to Pond	l IB 7.1 : Infilti	ration Basin				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

A	rea (sf)	CN E	Description					
	13,590	98 V	Water Surface, HSG B					
	1,860	98 F	aved park	ing, HSG C				
	1,425	98 F	aved park	ing, HSG B				
	28,270	55 V	Voods, Go	od, HSG B				
	8,160	74 >	75% Gras	s cover, Go	ood, HSG C			
	750	98 F	Roofs, HSG	G C				
	10,520	61 >	75% Gras	s cover, Go	bod, HSG B			
	64,575	70 V	Veighted A	verage				
	46,950	7	2.71% Per	vious Area				
	17,625	2	7.29% Imp	pervious Are	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	50	0.1500	0.15		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
0.9	110	0.1500	1.94		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
6.6	160	Total						

Summary for Subcatchment P7.6: Lots 17-20

Runoff	=	3.08 cfs @	12.07 hrs,	Volume=	9,482 cf,	Depth> 4.69"	
Routed	to Pond	d RG-16/20 : I	Rain Garder	n Lots 17-20			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Postdevelopment Type III 24-hr 25YearMass Rainfall=6.18" Printed 5/1/2023

Page 45

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

A	rea (sf)	CN	Description						
	11,200	90	1/8 acre lots, 65% imp, HSG C						
	13,081	85	1/8 acre lot	s, 65% imp	o, HSG B				
	24,281	87	Weighted A	verage					
	8,498	;	35.00% Pei	vious Area	а				
	15,783		65.00% Imp	pervious Ar	rea				
т.	1			0	Description				
Tc (min)	Length	Slope		Capacity	•				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry,				
	Summary for Subcatchment P7.7: Lot 15								

Runoff = 1.40 cfs @ 12.07 hrs, Volume= 4,390 cf, Depth> 5.02" Routed to Pond RG15H : Rain Garden 15H

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

	A	rea (sf)	CN I	Description							
*		10,500	90	Residential Lots, 65% imp, HSG C							
		3,675	:	35.00% Pervious Area							
		6,825	(65.00% Impervious Area							
	_		~		• •						
	Tc	Length	Slope		Capacity	Description					
_	(min)	(feet) (ft/ft) (ft/sec) (cfs)									
	5.0		Direct Entry,								
				•							

Summary for Subcatchment P7.8: Lot 16

Runoff	=	0.90 cfs @	12.07 hrs,	Volume=	2,820 cf,	Depth>	5.02"
Routed	to Pond	RG16 : Rain	Garden Lo	t 16			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Description					
Residential Lots, 65% imp, HSG C					
5.00% Pervious Area					
65.00% Impervious Area					
e Velocity Capacity Description t) (ft/sec) (cfs)					

AP7

Summary for Subcatchment P7.9: Lots 26-27

Runoff = 1.81 cfs @ 12.07 hrs, Volume= 5,689 cf, Depth> 5.02" Routed to Pond RG-26/27 : Rain Garden Lots 26-27

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Area (s	f) CN	CN Description						
* 13,60	5 90	0 Residential Lots, 65% imp, HSG C						
4,76	2	35.00% Pervious Area						
8,84	3	65.00% Impervious Area						
Tc Leng (min) (fee			Capacity (cfs)	Description				
5.0	Direct Entry,							
		_						

Summary for Pond CB1: CB1

 Inflow Area =
 8,260 sf, 82.93% Impervious, Inflow Depth > 5.47"
 for 25YearMass event

 Inflow =
 1.16 cfs @
 12.07 hrs, Volume=
 3,766 cf

 Outflow =
 1.16 cfs @
 12.07 hrs, Volume=
 3,766 cf

 Primary =
 1.16 cfs @
 12.07 hrs, Volume=
 3,766 cf

 Routed to Pond IB 7.1 : Infiltration Basin
 3,766 cf
 3,766 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 727.59' @ 12.07 hrs Flood Elev= 730.00'

#1 Primary 727.00' 12.0" Round Culvert L= 40.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 727.00' / 726.60' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf	

Primary OutFlow Max=1.16 cfs @ 12.07 hrs HW=727.59' TW=706.28' (Dynamic Tailwater) ☐ 1=Culvert (Barrel Controls 1.16 cfs @ 3.48 fps)

Summary for Pond IB 7.1: Infiltration Basin

Inflow Area =	299,386 sf	, 39.73% Impervious	, Inflow Depth > 3.50" for 25YearMass event
Inflow =	20.00 cfs @	12.11 hrs, Volume=	87,309 cf
Outflow =	6.88 cfs @	12.58 hrs, Volume=	79,374 cf, Atten= 66%, Lag= 27.9 min
Discarded =	0.65 cfs @	12.58 hrs, Volume=	24,482 cf
Primary =	6.23 cfs @	12.58 hrs, Volume=	54,892 cf
Routed to Link	(P7 : AP7		
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0 cf
Routed to Link	(P7 : AP7		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 708.01' @ 12.58 hrs Surf.Area= 11,634 sf Storage= 32,612 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 65.2 min (878.6 - 813.5)

Volume	Invert	Avail.Storage		Storage Description		
#1	704.00'	58,8	43 cf	Custom Stage Data	a (Irregular)Listed	below (Recalc)
Elevatio (fee		f.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
704.0 706.0 708.0 710.0)0)0)0 1	4,675 8,235 1,620	575.0 615.0 650.0 680.0	0 12,743 19,758 26,342	0 12,743 32,501 58,843	4,675 8,645 12,389 15,831
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	704.00'	2.41	0 in/hr Exfiltration o	ver Surface area	
#2	Secondary	709.00'		long x 10.0' bread		
#3	Primary	704.00'	Coel 18.0 L= 3 Inlet	d (feet) 0.20 0.40 0 f. (English) 2.49 2.5 " Round Culvert 5.0' CPP, square ed / Outlet Invert= 704.0	6 2.70 2.69 2.68 dge headwall, Ke= 00' / 704.00' S= 0.	2.69 2.67 2.64 0.500 0000 '/' Cc= 0.900
#4	Device 3	709.00'	24.0	.013 Corrugated PE "Horiz. Orifice/Grat ted to weir flow at low	e C= 0.600	low Area= 1.77 st
#5	Device 3	704.00'	Cus t Head	tom Weir/Orifice, Cv d (feet) 0.00 1.40 1	*= 2.62 (C= 3.28) .40 2.00 3.00 3.2	5 3.25 4.00 4.00 5.00
#6	Device 3	705.40'				25 0.50 0.50 0.66 0.66 I to weir flow at low heads

Discarded OutFlow Max=0.65 cfs @ 12.58 hrs HW=708.01' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.65 cfs)

Primary OutFlow Max=6.23 cfs @ 12.58 hrs HW=708.01' TW=0.00' (Dynamic Tailwater)

-3=Culvert (Passes 6.23 cfs of 15.36 cfs potential flow)

4=Orifice/Grate (Controls 0.00 cfs)

-5=Custom Weir/Orifice (Weir Controls 3.99 cfs @ 4.73 fps)

-6=Orifice/Grate (Orifice Controls 2.24 cfs @ 7.30 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=704.00' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG-16/20: Rain Garden Lots 17-20

Inflow Area = 24,281 sf, 65.00% Impervious, Inflow Depth > 4.69" for 25YearMass event 3.08 cfs @ 12.07 hrs, Volume= Inflow 9,482 cf = Outflow 2.31 cfs @ 12.14 hrs, Volume= 9,445 cf, Atten= 25%, Lag= 3.9 min = = 2.31 cfs @ 12.14 hrs, Volume= Primary 9,445 cf Routed to Pond IB 7.1 : Infiltration Basin

	Postdevelopme	ent
AP7	Type III 24-hr 25YearMass Rainfall=6.1	8″
Prepared by Mcclure Engineeering	Printed 5/1/202	23
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software S	Solutions LLC Page	<u>48</u>

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 727.53' @ 12.14 hrs Surf.Area= 2,912 sf Storage= 1,415 cf

Plug-Flow detention time= 15.5 min calculated for 9,445 cf (100% of inflow) Center-of-Mass det. time= 13.0 min (805.2 - 792.2)

Volume	Invert	Avail.Storage	Storage Description
#1	724.25'	188 cf	12.0" Round Pipe Storage x 4 Inside #2
			L= 60.0'
#2	724.25'	1,077 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 4
			2,880 cf Overall - 188 cf Embedded = 2,692 cf x 40.0% Voids
#3	727.25'	120 cf	4.00'W x 60.00'L x 0.25'H Mulch × 4
			240 cf Overall x 50.0% Voids
#4	727.50'	1,493 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 4
		2,878 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	724.25'	6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 724.25' / 724.25' S= 0.0000 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	728.00'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	724.25'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	726.50'	4.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=2.31 cfs @ 12.14 hrs HW=727.53' TW=706.72' (Dynamic Tailwater)

1=Culvert (Passes 2.31 cfs of 6.58 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.75 cfs @ 8.61 fps)

-4=Orifice/Grate (Orifice Controls 1.56 cfs @ 4.47 fps)

Summary for Pond RG-26/27: Rain Garden Lots 26-27

Inflow Are	a =	13,605 sf, 65.00% Impervious	, Inflow Depth > 5.02" for 25YearMass event
Inflow	=	1.81 cfs @ 12.07 hrs, Volume=	5,689 cf
Outflow	=	1.26 cfs @ 12.15 hrs, Volume=	5,673 cf, Atten= 31%, Lag= 4.5 min
Primary	=	1.26 cfs @ 12.15 hrs, Volume=	5,673 cf
Routed	l to Pond	d IB 7.1 : Infiltration Basin	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 747.73' @ 12.15 hrs Surf.Area= 1,561 sf Storage= 818 cf

Plug-Flow detention time= 14.6 min calculated for 5,671 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (795.2 - 782.3)

Type III 24-hr 25YearMass Rainfall=6.18"

AF I				$I y p \in II Z + II Z + II Z = I P =$
Prepare	ed by Mcclure	Engineeer	ing	Printed 5/1/202
HydroCA	<u>.D® 10.20-2g_s</u>	/n 03362 © 2	<u>2022 F</u>	HydroCAD Software Solutions LLC Page 4
Volume	Invert	Avail.Sto	rade	Storage Description
#1	744.25'			12.0" Round Pipe Storage x 2 Inside #2 L= 60.0'
#2	744.25'	53	38 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,440 cf Overall - 94 cf Embedded = 1,346 cf x 40.0% Voids
#3	747.25'	(60 cf	4.00'W x 60.00'L x 0.25'H Mulch x 2 120 cf Overall x 50.0% Voids
#4	747.50'	74	17 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,43	39 cf	Total Available Storage
Device	Routing	Invert	Outl	let Devices
#1	Primary	744.25'	L= 1 Inlet	' Round Culvert X 2.00 10.0' CPP, square edge headwall, Ke= 0.500 t / Outlet Invert= 744.25' / 744.15' S= 0.0100 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	748.00'	6.0"	' Horiz. Orifice/Grate X 2.00 C= 0.600 ited to weir flow at low heads
#3	Device 1	744 25'	2 0"	Vert Orifice/Grate $X = 200$ C= 0.600

#3	Device 1	744.25'	2.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	746.50'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=1.25 cfs @ 12.15 hrs HW=747.73' TW=706.78' (Dynamic Tailwater)

-1=Culvert (Passes 1.25 cfs of 3.40 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.39 cfs @ 8.88 fps)

-4=Orifice/Grate (Orifice Controls 0.87 cfs @ 4.97 fps)

Summary for Pond RG-28/29: Rain Garden Lots 28-29

17,105 sf, 65.00% Impervious, Inflow Depth > 5.02" for 25YearMass event Inflow Area = Inflow = 2.28 cfs @ 12.07 hrs, Volume= 7,152 cf Outflow = 1.45 cfs @ 12.16 hrs, Volume= 7,134 cf, Atten= 36%, Lag= 5.2 min 1.45 cfs @ 12.16 hrs, Volume= Primary = 7,134 cf Routed to Pond IB 7.1 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 743.54' @ 12.16 hrs Surf.Area= 1,723 sf Storage= 1,024 cf

Plug-Flow detention time= 14.5 min calculated for 7,134 cf (100% of inflow) Center-of-Mass det. time= 12.9 min (795.2 - 782.3)

AP7

Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023 Page 50

	lascont	Aveil Cte		Stevens Description
Volume	Invert	Avaii.Storage		Storage Description
#1	739.75'	94 cf		12.0" Round Pipe Storage x 2 Inside #2 L= 60.0'
#2	739.75'	538 cf		4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,440 cf Overall - 94 cf Embedded = 1,346 cf x 40.0% Voids
#3	742.75'	60 cf		4.00'W x 60.00'L x 0.25'H Mulch x 2
	740.001			120 cf Overall x 50.0% Voids
#4	743.00'	/2	1/ cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 2
		1,43	39 cf	Total Available Storage
				-
Device	Routing	Invert	Outl	et Devices
#1	Primary	739.75'	6.0"	Round Culvert X 2.00
	,		L= 1	0.0' CPP, square edge headwall, Ke= 0.500
				: / Outlet Invert= 739.75' / 739.65' S= 0.0100 '/' Cc= 0.900
				0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	743.50'		Horiz. Orifice/Grate X 2.00 C= 0.600
π∠	DEVICE	740.00		ted to weir flow at low heads
#3	Device 1	739.75'		
#3	Device I	139.13		Vert. Orifice/Grate X 2.00 C= 0.600
	During 4	740.00		ted to weir flow at low heads
#4	Device 1	742.00'	-	Vert. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads

Primary OutFlow Max=1.45 cfs @ 12.16 hrs HW=743.53' TW=706.85' (Dynamic Tailwater)

-**1=Culvert** (Passes 1.45 cfs of 3.55 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.07 cfs @ 0.61 fps)

-3=Orifice/Grate (Orifice Controls 0.40 cfs @ 9.26 fps)

-4=Orifice/Grate (Orifice Controls 0.98 cfs @ 5.63 fps)

Summary for Pond RG15H: Rain Garden 15H

10,500 sf, 65.00% Impervious, Inflow Depth > 5.02" for 25YearMass event Inflow Area = Inflow = 1.40 cfs @ 12.07 hrs, Volume= 4.390 cf Outflow 1.37 cfs @ 12.09 hrs, Volume= 4,170 cf, Atten= 2%, Lag= 1.0 min = Discarded = 0.04 cfs @ 11.90 hrs, Volume= 954 cf 1.33 cfs @ 12.09 hrs, Volume= Primarv = 3,216 cf Routed to Pond IB 7.1 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 719.23' @ 12.09 hrs Surf.Area= 720 sf Storage= 521 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 15.9 min (798.2 - 782.3)

AP7

Prepared by Mcclure Engineeering

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Postdevelopment Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023

		•	•		
<u>HydroCAD</u>	® 10.20-2g	s/n 03362	<u>© 2022 Hyd</u>	roCAD Software Solutions LLC	<u>Page 51</u>

Volume	Invert	Avail.Storage	Storage Description
#1	715.25'	47 cf	
			L= 60.0'
#2	715.25'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
#2	710 051	30 cf	720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	718.25'	50 CI	4.00'W x 60.00'L x 0.25'H Mulch 60 cf Overall x 50.0% Voids
#4	718.50'	240 cf	
-		586 cf	
			·
Device	Routing	Invert Out	let Devices
#1	Primary	717.25' 8.0 '	' Round Culvert
		L= 1	10.0' CPP, square edge headwall, Ke= 0.500
		Inle	t / Outlet Invert= 717.25' / 717.15' S= 0.0100 '/' Cc= 0.900
		n= (0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1		'Horiz. Orifice/Grate C= 0.600
			ited to weir flow at low heads
#3	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	715.25' 2.4 ′	10 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.04 cfs @ 11.90 hrs HW=718.51' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=1.32 cfs @ 12.09 hrs HW=719.23' TW=706.40' (Dynamic Tailwater)

-1=Culvert (Passes 1.32 cfs of 2.16 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.75 cfs @ 1.57 fps)

-3=Orifice/Grate (Orifice Controls 0.32 cfs @ 6.56 fps)

-4=Orifice/Grate (Orifice Controls 0.25 cfs @ 5.06 fps)

Summary for Pond RG15L: Rain Garden 15L

Inflow Area =	8,190 sf, 65.00% Impervious,	Inflow Depth > 5.02" for 25YearMass event			
Inflow =	1.09 cfs @ 12.07 hrs, Volume=	3,425 cf			
Outflow =	0.92 cfs @ 12.12 hrs, Volume=	3,221 cf, Atten= 16%, Lag= 2.9 min			
Discarded =	0.04 cfs @ 11.99 hrs, Volume=	915 cf			
Primary =	0.88 cfs @ 12.12 hrs, Volume=	2,306 cf			
Routed to Pond IB 7.1 : Infiltration Basin					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 710.64' @ 12.12 hrs Surf.Area= 720 sf Storage= 500 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 21.2 min (803.5 - 782.3)

AP7

Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023 Page 52

Volume	Invert	Avail.Storage	e Storage Description
#1	706.75'	47 c	f 12.0" Round Pipe Storage Inside #2 L= 60.0'
#2	706.75'	269 c	
#3	709.75'	30 c	f 4.00'W x 60.00'L x 0.25'H Mulch
#4	710.00'	240 c	60 cf Overall x 50.0% Voids f 4.00'W x 60.00'L x 1.00'H Ponding
		586 c	
Device	Routing	Invert Ou	Itlet Devices
#1	Primary	708.75' 6.)" Round Culvert
	,	L=	10.0' CPP, square edge headwall, Ke= 0.500
			et / Outlet Invert= 708.75' / 708.65' S= 0.0100 '/' Cc= 0.900
		n=	0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	710.50' 6.)" Horiz. Orifice/Grate C= 0.600
		Lir	nited to weir flow at low heads
#3	Device 1	708.75' 2.)" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	709.25' 4.	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	706.75' 2. 4	10 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.04 cfs @ 11.99 hrs HW=710.00' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Primary OutFlow Max=0.88 cfs @ 12.12 hrs HW=710.64' TW=706.61' (Dynamic Tailwater)

—1=Culvert (Passes 0.88 cfs of 1.21 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.27 cfs @ 1.23 fps)

-3=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.47 fps)

-4=Orifice/Grate (Orifice Controls 0.46 cfs @ 5.33 fps)

Summary for Pond RG16: Rain Garden Lot 16

Inflow Area =	6,745 sf, 65.00% Impervious,	Inflow Depth > 5.02" for 25YearMass event			
Inflow =	0.90 cfs @ 12.07 hrs, Volume=	2,820 cf			
Outflow =	0.61 cfs @ 12.15 hrs, Volume=	2,647 cf, Atten= 32%, Lag= 4.7 min			
Discarded =	0.04 cfs @ 12.02 hrs, Volume=	887 cf			
Primary =	0.57 cfs @ 12.15 hrs, Volume=	1,759 cf			
Routed to Pond IB 7.1 : Infiltration Basin					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 733.98' @ 12.15 hrs Surf.Area= 720 sf Storage= 461 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 28.1 min (810.5 - 782.3)

AP7

Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023 Page 53

Volume	Invert	Avail.Storage	Storage Description
#1	730.25'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	730.25'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	733.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	733.50'	240 cf	4.00'W x 60.00'L x 1.00'H Ponding
		586 cf	Total Available Storage
			Ũ
Device	Routing	Invert Out	et Devices
#1	Primary	732.25' 6.0 "	Round Culvert
	,	L= 6	6.0' CPP, square edge headwall, Ke= 0.500
			/ Outlet Invert= 732.25' / 732.00' S= 0.0417 '/' Cc= 0.900
		n= 0	0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	734.00' 6.0 "	Horiz. Orifice/Grate C= 0.600
		Limi	ted to weir flow at low heads
#3	Device 1	732.25' 2.0 "	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	732.75' 4.0 "	Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	730.25' 2.41	0 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.04 cfs @ 12.02 hrs HW=733.50' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Primary OutFlow Max=0.57 cfs @ 12.15 hrs HW=733.98' TW=706.80' (Dynamic Tailwater)

-1=Culvert (Passes 0.57 cfs of 1.15 cfs potential flow)

2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.13 cfs @ 6.17 fps)

-4=Orifice/Grate (Orifice Controls 0.43 cfs @ 4.96 fps)

Summary for Pond RG21: Rain Garden 21

Inflow Area =	7,915 sf, 65.00% Impervious,	Inflow Depth > 4.47" for 25YearMass event
Inflow =	0.97 cfs @ 12.07 hrs, Volume=	2,948 cf
Outflow =	0.62 cfs @ 12.16 hrs, Volume=	2,762 cf, Atten= 36%, Lag= 5.2 min
Discarded =	0.05 cfs @ 12.16 hrs, Volume=	849 cf
Primary =	0.58 cfs @ 12.16 hrs, Volume=	1,913 cf
Routed to Link	P7 : AP7	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 728.51' @ 12.16 hrs Surf.Area= 817 sf Storage= 482 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 22.8 min (820.9 - 798.1)

AP7

Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023

	HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 54				
<u>i iyaroo</u> ,	10.20 2g 0				
Volume	Invert	Avail.Storage	Storage Description		
#1	724.75'	31 cf	12.0" Round Pipe Storage Inside #2 L= 40.0'		
#2	724.75'	275 cf	6.00'W x 40.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 31 cf Embedded = 689 cf x 40.0% Voids		
#3	727.75'	30 cf	6.00'W x 40.00'L x 0.25'H Mulch 60 cf Overall x 50.0% Voids		
#4	728.00'	337 cf	6.00'W x 40.00'L x 1.00'H Ponding Z=2.0		
		674 cf	Total Available Storage		
Device	Routing	Invert Ou	tlet Devices		
#1	Primary	L= Inle	6.75' 6.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 726.75' / 726.65' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf		
#2	Device 1	728.50' 6.0	"Horiz. Orifice/Grate C= 0.600 nited to weir flow at low heads		
#3	Device 1	726.75' 2.0	" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#4	Device 1	727.25' 4.0	" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		

Discarded OutFlow Max=0.05 cfs @ 12.16 hrs HW=728.51' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=0.58 cfs @ 12.16 hrs HW=728.51' TW=0.00' (Dynamic Tailwater)

1=Culvert (Passes 0.58 cfs of 1.16 cfs potential flow)

724.75'

2=Orifice/Grate (Weir Controls 0.00 cfs @ 0.24 fps)

-3=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.23 fps)

-4=Orifice/Grate (Orifice Controls 0.44 cfs @ 5.02 fps)

Summary for Pond RG22-25: Rain Gardens 22-25

2.410 in/hr Exfiltration over Surface area

Inflow Area =	99,920 sf, 32.59% Impervious,	Inflow Depth > 3.83" for 25YearMass event		
Inflow =	8.22 cfs @ 12.18 hrs, Volume=	31,902 cf		
Outflow =	7.75 cfs @ 12.23 hrs, Volume=	31,839 cf, Atten= 6%, Lag= 2.8 min		
	5.81 cfs @ 12.23 hrs, Volume=	23,880 cf		
Routed to Pond IB 7.1 : Infiltration Basin				
Secondary =	1.94 cfs @ 12.23 hrs, Volume=	7,960 cf		
Routed to Link	P7 : AP7			

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 728.63' @ 12.23 hrs Surf.Area= 3,552 sf Storage= 2,201 cf

Plug-Flow detention time= 5.3 min calculated for 31,826 cf (100% of inflow) Center-of-Mass det. time= 4.1 min (824.6 - 820.5)

AP7

#5

Discarded

AP7Type III 24-hr25YearMass Rainfall=6.18"Prepared by Mcclure EngineeeringPrinted 5/1/2023HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLCPage 55					
HydroCA	D® 10.20-2g s/	<u>n 03362 © 2</u>	<u>2022 H</u>	lydroCAD Software Solutions LLC Page 55	
Volume	Invert	Avail.Stor	rage	Storage Description	
#1	724.75'	18	38 cf	12.0" Round Pipe Storage x 4 Inside #2 L= 60.0'	
#2	724.75'	1,07	77 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 4 2,880 cf Overall - 188 cf Embedded = 2,692 cf x 40.0% Voids	
#3	727.75'	12	20 cf	4.00'W x 60.00'L x 0.25'H Mulch x 4 240 cf Overall x 50.0% Voids	
#4	728.00'	1,49	93 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 4	
				Total Available Storage	
Device	Routing	Invert	Outl	et Devices	
<u>Device</u> #1	Primary	724.75'		Round Culvert X 3.00	
#2	Secondary	724.75'	L= 1 Inlet n= 0 8.0" L= 1 Inlet	0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 724.75' / 724.75' S= 0.0000 '/' Cc= 0.900 .013 Corrugated PE, smooth interior, Flow Area= 0.35 sf Round Culvert 0.0' CPP, square edge headwall, Ke= 0.500 / Outlet Invert= 724.75' / 724.75' S= 0.0000 '/' Cc= 0.900 .013 Corrugated PE, smooth interior, Flow Area= 0.35 sf	
#3	Device 1	728.50'	6.0"	Horiz. Orifice/Grate X 6.00 C= 0.600 ted to weir flow at low heads	
#4	Device 2	728.50'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads	
#5	Device 1	724.75'	3.0"	Vert. Orifice/Grate X 6.00 C= 0.600 ted to weir flow at low heads	
#6	Device 2	724.75'	3.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads	
#7	Device 1	727.25'	3.0"	Vert. Orifice/Grate X 6.00 C= 0.600 ted to weir flow at low heads	
#8	Device 2	727.25'	3.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads	
Primary OutFlow Max=5.80 cfs @ 12.23 hrs HW=728.63' TW=707.22' (Dynamic Tailwater) 1=Culvert (Passes 5.80 cfs of 9.50 cfs potential flow) -3=Orifice/Grate (Weir Controls 1.46 cfs @ 1.18 fps)					

-5=Orifice/Grate (Orifice Controls 2.75 cfs @ 9.33 fps)

-7=Orifice/Grate (Orifice Controls 1.59 cfs @ 5.40 fps)

Secondary OutFlow Max=1.93 cfs @ 12.23 hrs HW=728.63' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 1.93 cfs of 3.17 cfs potential flow) -4=Orifice/Grate (Weir Controls 0.49 cfs @ 1.18 fps) -6=Orifice/Grate (Orifice Controls 0.92 cfs @ 9.33 fps)

-8=Orifice/Grate (Orifice Controls 0.53 cfs @ 5.40 fps)

Summary for Pond RG27: Rain Garden Lot 27

Inflow Area = 10,800 sf, 23.14% Impervious, Inflow Depth > 3.94" for 25YearMass event Inflow 1.00 cfs @ 12.14 hrs, Volume= 3,543 cf = 3,533 cf, Atten= 32%, Lag= 7.3 min Outflow 0.68 cfs @ 12.26 hrs, Volume= = 0.68 cfs @ 12.26 hrs, Volume= Primary = 3.533 cf Routed to Pond IB 7.1 : Infiltration Basin

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 758.49' @ 12.26 hrs Surf.Area= 848 sf Storage= 493 cf

Plug-Flow detention time= 14.6 min calculated for 3,532 cf (100% of inflow) Center-of-Mass det. time= 12.9 min (828.3 - 815.4)

Invert	Avail.Storage	Storage Description
754.75'	47 cf	12.0" Round Pipe Storage Inside #2
		L= 60.0'
754.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
		720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
757.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
		60 cf Overall x 50.0% Voids
758.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
	720 cf	Total Available Storage
	754.75' 754.75' 757.75'	754.75' 47 cf 754.75' 269 cf 757.75' 30 cf 758.00' 373 cf

Device	Routing	Invert	Outlet Devices
#1	Primary	754.75'	6.0" Round Culvert
			L= 36.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 754.75' / 753.50' S= 0.0347 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	758.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	754.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	757.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.68 cfs @ 12.26 hrs HW=758.48' TW=707.37' (Dynamic Tailwater)

-1=Culvert (Passes 0.68 cfs of 1.60 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.20 cfs @ 9.20 fps)

-4=Orifice/Grate (Orifice Controls 0.48 cfs @ 5.53 fps)

Summary for Pond RG28: Rain Garden Lot 28

Inflow Area	a =	28,665 sf	, 9.64% Impervious,	Inflow Depth > 3.33"	for 25YearMass event
Inflow	=	2.14 cfs @	12.16 hrs, Volume=	7,952 cf	
Outflow	=	1.85 cfs @	12.23 hrs, Volume=	7,941 cf, Atte	en= 14%, Lag= 4.4 min
Primary	=	1.85 cfs @	12.23 hrs, Volume=	7,941 cf	
Routed	to Pond	d IB 7.1 : Infilti	ration Basin		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 757.53' @ 12.23 hrs Surf.Area= 861 sf Storage= 511 cf

Plug-Flow detention time= 4.3 min calculated for 7,941 cf (100% of inflow) Center-of-Mass det. time= 3.5 min (834.7 - 831.2)

Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023 Page 57

Volume	Invert	Avail.Storage		Storage Description
#1	753.75'	۷	17 cf	12.0" Round Pipe Storage Inside #2
				L= 60.0'
#2	753.75'	26	69 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
				720 cf Overall - 47 cf Embedded = 673 cf \times 40.0% Voids
#3	756.75'	3	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
	757 001	0-	70.5	60 cf Overall x 50.0% Voids
#4	757.00'	31	/3 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		72	20 cf	Total Available Storage
Device	Routing	Invert	Outl	et Devices
#1	Primary	753.75'	8.0"	Round Culvert
	·		L= 3	30.0' CPP, square edge headwall, Ke= 0.500
			Inlet	/ Outlet Invert= 753.75' / 753.50' S= 0.0083 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 s	
#2	Device 1	757.50'	6.0"	Horiz. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads
#3	Device 1	753.75'	3.0"	Vert. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads
#4	Device 1	756.25'	4.0"	Vert. Orifice/Grate X 2.00 C= 0.600
			Limi	ted to weir flow at low heads

Limited to weir flow at low heads

Primary OutFlow Max=1.85 cfs @ 12.23 hrs HW=757.53' TW=707.26' (Dynamic Tailwater)

-1=Culvert (Passes 1.85 cfs of 2.91 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.06 cfs @ 0.58 fps)

-3=Orifice/Grate (Orifice Controls 0.90 cfs @ 9.21 fps)

-4=Orifice/Grate (Orifice Controls 0.89 cfs @ 5.08 fps)

Summary for Pond RG30: Rain Garden Lot 30

Inflow Are	a =	6,740 sf, 65.00% Impervious	, Inflow Depth > 5.02" for 25YearMass event					
Inflow	=	0.90 cfs @ 12.07 hrs, Volume=	2,818 cf					
Outflow	=	0.62 cfs @ 12.15 hrs, Volume=	2,810 cf, Atten= 30%, Lag= 4.5 min					
Primary	=	0.62 cfs @ 12.15 hrs, Volume=	2,810 cf					
Routed to Pond IB 7.1 : Infiltration Basin								

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 731.22' @ 12.15 hrs Surf.Area= 777 sf Storage= 406 cf

Plug-Flow detention time= 14.6 min calculated for 2,809 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (795.2 - 782.3)

AP7

Prepared by Mcclure Engineeering

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

Postdevelopment Type III 24-hr 25YearMass Rainfall=6.18"

Printed 5/1/2023

HydroCAD® 10.20-2g	s/n 03362	© 2022 HydroCAE) Software Solutions LLC	Page 58

Volume	Invert	Avail Stor	rage Storage Description			
-	IIIVEIL	Avail.Store				
#1	727.75'	4	17 cf 12.0" Round Pipe Storage Inside #2			
	707 75	0.0	L= 60.0'			
#2	727.75'	26	69 cf 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel			
40		0	720 cf Overall - 47 cf Embedded = 673 cf \times 40.0% Voids			
#3	730.75'	30	30 cf 4.00'W x 60.00'L x 0.25'H Mulch			
			60 cf Overall x 50.0% Voids			
#4	731.00'	37:	73 cf 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0			
		720	20 cf Total Available Storage			
			0			
Device	Routing	Invert	Outlet Devices			
#1	Primary	727.75'	6.0" Round Culvert			
			L= 10.0' CPP, square edge headwall, Ke= 0.500			
			Inlet / Outlet Invert= 727.75' / 727.65' S= 0.0100 '/' Cc= 0.900			
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf			
#2	Device 1		6.0" Horiz. Orifice/Grate C= 0.600			
π ∠	Device		Limited to weir flow at low heads			
#2	Davias 1					
#3	Device 1		2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads			
#4	Device 1	730.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads			
· · ·		U U				
1=Cu	Ivert (Passes	0.62 cfs of 1	1.70 cfs potential flow)			
T	1 - 2 - Orifica / Crota / Controla 0.00 ofa)					

-2=Orifice/Grate (Controls 0.00 cfs) -3=Orifice/Grate (Orifice Controls 0.19 cfs @ 8.86 fps)

-4=Orifice/Grate (Orifice Controls 0.43 cfs @ 4.94 fps)

Summary for Link P7: AP7

Inflow Area =	400,631 sf, 31.37% Impervious,	Inflow Depth > 2.34"	for 25YearMass event
Inflow =	9.23 cfs @ 12.41 hrs, Volume=	77,984 cf	
Primary =	9.23 cfs @ 12.41 hrs, Volume=	77,984 cf, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Summary for Subcatchment P7.1: AP7 - To Wetland E

Runoff = 6.29 cfs @ 12.13 hrs, Volume= 22,073 cf, Depth> 2.84" Routed to Link P7 : AP7

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

 А	rea (sf)	CN	Adj Desc	cription				
	78,255	55	Woods, Good, HSG B					
	13,515	61	>75%	>75% Grass cover, Good, HSG B				
	1,560	98	Unco	Unconnected roofs, HSG B				
	93,330	57	7 56 Weighted Average, UI Adjusted					
	91,770		98.33% Pervious Área					
	1,560		1.67	% Impervic	ous Area			
	1,560		100.	00% Uncor	nnected			
Тс	Length	Slope	Velocity	Capacity	Description			
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
4.3	50	0.3000	0.19		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.00"			
0.2	25	0.3000	2.74		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
4.1	245	0.0400	1.00		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
8.6	320	Total						

Summary for Subcatchment P7.10: Lot 27

Runoff	=	1.40 cfs @	12.13 hrs,	Volume=	4,995 cf,	Depth>	5.55"
Routed	to Pond	d RG27 : Rair	i Garden Lo	ot 27			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	Area (sf)	CN	Description		
*	3,845	90	Residential Lots, 65% imp, HSG C		
	2,045	70	Woods, Good, HSG C		
	940	89	Gravel roads, HSG C		
	3,970	74	>75% Grass cover, Good, HSG C		
	10,800	80	Weighted Average		
	8,301		6.86% Pervious Area		
	2,499		23.14% Impervious Area		

					Postdevelopment
AP7					Type III 24-hr 100YearMass Rainfall=7.93"
			gineeering 362 © 202		Printed 5/1/2023 Software Solutions LLC Page 60
<u> </u>		<u>g 0,1100</u>	002 0 202	21194100/12	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.7	50	0.0400	0.09		Sheet Flow,
0.2	35	0.2000	3.13		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
9.9	85	Total			
			Summa	ry for Su	bcatchment P7.11: Lot 28
Runoff Route	= ed to Pon		s @ 12.1 Rain Garc	6 hrs, Volu Jen Lot 28	me= 11,591 cf, Depth> 4.85"
Runoff b	y SCS TF	R-20 meth	nod, UH=S	SCS, Weigh	ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
			s Rainfall		
A	rea (sf)		escription		
*	4,250				imp, HSG C
	18,600 960		ravel road	od, HSG C	
	4,855				ood, HSG C
	28,665		Veighted A		· · · · · · · · · · · · · · · · · · ·
	25,903			rvious Area	
	2,763	9	.64% Impe	ervious Area	a
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.7	50	0.0400	0.09		Sheet Flow,
1.7	100	0.0400	1.00		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow,
0.2	30	0.2000	3.13		Woodland Kv= 5.0 fps Shallow Concentrated Flow,
11.6	180	Total			Short Grass Pasture Kv= 7.0 fps
11.0	100				
		S	Summary	y for Sub	catchment P7.12: Lot 28-29
Runoff Route	= ed to Pon			7 hrs, Volu Garden Lots	
			nod, UH=S ss Rainfall [:]		ted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Δ	rea (sf)	CN D	escription		
*	17,105				imp, HSG C

	Area (st)	CN	Description
*	17,105	90	Residential Lots, 65% imp, HSG C
	5,987		35.00% Pervious Area
	11,118		65.00% Impervious Area

AP7Type III 24-hr100YearMass Rainfall=7.93Prepared by Mcclure EngineeeringPrinted 5/1/202HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLCPage 6	3″ 23								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
5.0 Direct Entry,									
Summary for Subcatchment P7.13: Lot 30									
Runoff = 1.18 cfs @ 12.07 hrs, Volume= 3,781 cf, Depth> 6.73" Routed to Pond RG30 : Rain Garden Lot 30									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"									
Area (sf) CN Description									
* 6,740 90 Residential Lots, 65% imp, HSG C	_								
2,359 35.00% Pervious Area									
4,381 65.00% Impervious Area									
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)									
5.0 Direct Entry,	_								
Summary for Subcatchment P7.14: Clubhouse Parking Lot									
Runoff = 1.50 cfs @ 12.07 hrs, Volume= 4,961 cf, Depth> 7.21" Routed to Pond CB1 : CB1									
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"									
Area (sf) CN Description									

A	rea (st)	CN	Description							
	1,410	74	>75% Grass cover, Good, HSG C							
	750	98	Roofs, HSG	СС						
	6,100	98	Paved park	Paved parking, HSG C						
	8,260	94	Weighted Average							
	1,410		17.07% Per	vious Area						
	6,850		82.93% Imp	pervious Are	ea					
Тс	Length	Slope	,	Capacity	Description					
(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)						

5.0

Direct Entry,

Summary for Subcatchment P7.2: Lots 14-15

Runoff = 1.44 cfs @ 12.07 hrs, Volume= 4,594 cf, Depth> 6.73" Routed to Pond RG15L : Rain Garden 15L

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	А	rea (sf)	CN	Description							
*		8,190	90	Residential Lots, 65% imp, HSG C							
		2,867		35.00% Pervious Area							
		5,324		65.00% Imp	pervious Are	ea					
	Тс	Length	Slope	Velocity	Capacity	Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	5.0					Direct Entry,					

Summary for Subcatchment P7.3: Lot 20/21

1.31 cfs @ 12.07 hrs, Volume= 4,050 cf, Depth> 6.14" Runoff = Routed to Pond RG21 : Rain Garden 21

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Area (sf)	CN I	Description						
7,915	85	5 1/8 acre lots, 65% imp, HSG B						
2,770		35.00% Pervious Area						
5,145	(65.00% Impervious Area						
Tc Length (min) (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
5.0				Direct Entry,				

Summary for Subcatchment P7.4: Lots 21-25

Runoff 11.55 cfs @ 12.18 hrs, Volume= 45,215 cf, Depth> 5.43" = Routed to Pond RG22-25 : Rain Gardens 22-25

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	Area (sf)	CN	Description
	38,735	90	1/8 acre lots, 65% imp, HSG C
	11,360	85	1/8 acre lots, 65% imp, HSG B
	49,505	70	Woods, Good, HSG C
*	320	74	Grass Paver, Good, HSG C
	99,920	79	Weighted Average
	67,358		67.41% Pervious Area
	32,562		32.59% Impervious Area
	32,562		32.59% Impervious Area

AP7

			gineeering 362 © 202		Postdevelopment <i>Type III 24-hr 100YearMass Rainfall=7.93"</i> Printed 5/1/2023 Software Solutions LLC Page 63
<u></u>					<u> </u>
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.7	50	0.0400	0.09		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.00"
0.8	50	0.0400	1.00		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
2.6	230	0.0900	1.50		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
13.1	330	Total			

Summary for Subcatchment P7.5: Basin 1

Runoff	=	7.50 cfs @	12.10 hrs,	Volume=	23,670 cf,	Depth>	4.40"
Routed	l to Pond	d IB 7.1 : Infilti	ration Basin				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

A	rea (sf)	CN E	escription						
	13,590	98 V	Nater Surface, HSG B						
	1,860	98 F	aved park	ing, HSG C					
	1,425	98 F	aved park	ing, HSG B					
	28,270	55 V	Voods, Go	od, HSG B					
	8,160	74 >	75% Gras	s cover, Go	ood, HSG C				
	750	98 F	Roofs, HSG	СС					
	10,520	61 >	75% Gras	s cover, Go	ood, HSG B				
	64,575	70 V	Weighted Average						
	46,950	7	2.71% Per	vious Area					
	17,625	2	7.29% Imp	pervious Are	ea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.7	50	0.1500	0.15		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.00"				
0.9	110	0.1500	1.94		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
6.6	160	Total							

Summary for Subcatchment P7.6: Lots 17-20

Runoff	=	4.12 cfs @	12.07 hrs,	Volume=	12,903 cf,	Depth>	6.38"
Routed	l to Pond	RG-16/20 : I	Rain Garder	n Lots 17-20		-	

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93" Printed 5/1/2023

Page 64

Prepared by Mcclure Engineeering HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC

A	rea (sf)	CN Description							
	11,200	90 [·]	1/8 acre lot	s, 65% imp	, HSG C				
	13,081	85 ´	1/8 acre lot	s, 65% imp	, HSG B				
	24,281	87 \	Neighted A	verage					
	8,498	(35.00% Pei	vious Area					
	15,783	6	65.00% Imp	pervious Ar	ea				
Tc	Length	Slope		Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
5.0					Direct Entry	/,			
	Summary for Subcatchment P7.7: Lot 15								
Runoff									
Route	ed to Pone	d RG15H	∃ : Rain Ga	Routed to Pond RG15H : Rain Garden 15H					

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

_	A	rea (sf)	CN [Description								
*		10,500	90 F	Residential Lots, 65% imp, HSG C								
		3,675	3	35.00% Pervious Area								
		6,825	6	65.00% Impervious Area								
	-		<u></u>		A							
	Tc	Length	Slope	,	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	5.0					Direct Entry,						
				_								

Summary for Subcatchment P7.8: Lot 16

Runoff = 1.19 cfs @ 12.07 hrs, Volume= 3,784 cf, Depth> 6.73" Routed to Pond RG16 : Rain Garden Lot 16

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	Ar	ea (sf)	CN [Description				
*		6,745	90 F	Residential Lots, 65% imp, HSG C				
		2,361	3	35.00% Pervious Area				
		4,384	6	65.00% Impervious Area				
	Тс	Length	Slope	Velocity	Capacity	Description		
(m	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•		
	5.0					Direct Entry,		

AP7

Summary for Subcatchment P7.9: Lots 26-27

Runoff = 2.39 cfs @ 12.07 hrs, Volume= 7,632 cf, Depth> 6.73" Routed to Pond RG-26/27 : Rain Garden Lots 26-27

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

	Area (sf)	CN	Description						
*	13,605	90							
	4,762	4,762 35.00% Pervious Area							
	8,843	8,843 65.00% Impervious Area							
- (mi	rc Length n) (feet)	Slop (ft/f		Capacity (cfs)	Description				
5	.0				Direct Entry,				

Summary for Pond CB1: CB1

 Inflow Area =
 8,260 sf, 82.93% Impervious, Inflow Depth > 7.21" for 100YearMass event

 Inflow =
 1.50 cfs @
 12.07 hrs, Volume=
 4,961 cf

 Outflow =
 1.50 cfs @
 12.07 hrs, Volume=
 4,961 cf

 Primary =
 1.50 cfs @
 12.07 hrs, Volume=
 4,961 cf

 Routed to Pond IB 7.1 : Infiltration Basin
 4,961 cf
 4,961 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 727.69' @ 12.07 hrs Flood Elev= 730.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	727.00'	12.0" Round Culvert L= 40.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 727.00' / 726.60' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.50 cfs @ 12.07 hrs HW=727.69' TW=707.16' (Dynamic Tailwater) ☐ 1=Culvert (Barrel Controls 1.50 cfs @ 3.68 fps)

Summary for Pond IB 7.1: Infiltration Basin

Inflow Area =	299,386 sf	, 39.73% Impervious,	Inflow Depth > 4.95" for 100YearMass event
Inflow =	29.70 cfs @	12.14 hrs, Volume=	123,525 cf
Outflow =	11.32 cfs @	12.55 hrs, Volume=	114,761 cf, Atten= 62%, Lag= 24.6 min
Discarded =	0.73 cfs @	12.55 hrs, Volume=	27,045 cf
Primary =	10.59 cfs @	12.55 hrs, Volume=	87,716 cf
Routed to Link	(P7 : AP7		
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0 cf
Routed to Link	(P7 : AP7		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Peak Elev= 708.98' @ 12.55 hrs Surf.Area= 13,123 sf Storage= 44,618 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 60.6 min (866.0 - 805.4)

Volume	Invert	Avail.Sto	orage	Storage Description		
#1	704.00'	58,8	43 cf	Custom Stage Data	a (Irregular)Listed	below (Recalc)
Elevatio (fee		f.Area F (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
704.0 706.0 708.0 710.0)0)0)0 1	4,675 8,235 1,620	575.0 615.0 650.0 680.0	0 12,743 19,758 26,342	0 12,743 32,501 58,843	4,675 8,645 12,389 15,831
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	704.00'	2.41	0 in/hr Exfiltration o	ver Surface area	
#2	Secondary	709.00'		long x 10.0' bread		
#3	Primary	704.00'	Coef 18.0 L= 3 Inlet	d (feet) 0.20 0.40 0 f. (English) 2.49 2.50 " Round Culvert 5.0' CPP, square ec / Outlet Invert= 704.0	6 2.70 2.69 2.68 lge headwall, Ke= 00' / 704.00' S= 0.	2.69 2.67 2.64 0.500 0000 '/' Cc= 0.900
#4	Device 3	709.00'	24.0	.013 Corrugated PE Horiz. Orifice/Grat ied to weir flow at low	e C= 0.600	low Area= 1.77 st
#5	Device 3	704.00'	Cus t Head	tom Weir/Orifice, Cw d (feet) 0.00 1.40 1	2.62 (C= 3.28) .40 2.00 3.00 3.2	5 3.25 4.00 4.00 5.00
#6	Device 3	705.40'				25 0.50 0.50 0.66 0.66 I to weir flow at low heads

Discarded OutFlow Max=0.73 cfs @ 12.55 hrs HW=708.98' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.73 cfs)

Primary OutFlow Max=10.59 cfs @ 12.55 hrs HW=708.98' TW=0.00' (Dynamic Tailwater)

-3=Culvert (Passes 10.59 cfs of 17.50 cfs potential flow)

4=Orifice/Grate (Controls 0.00 cfs)

-5=Custom Weir/Orifice (Weir Controls 7.92 cfs @ 5.33 fps)

-6=Orifice/Grate (Orifice Controls 2.67 cfs @ 8.70 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=704.00' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond RG-16/20: Rain Garden Lots 17-20

Inflow Area = 24,281 sf, 65.00% Impervious, Inflow Depth > 6.38" for 100YearMass event 4.12 cfs @ 12.07 hrs, Volume= Inflow 12,903 cf = Outflow 2.66 cfs @ 12.16 hrs, Volume= 12,862 cf, Atten= 35%, Lag= 5.1 min = = 2.66 cfs @ 12.16 hrs, Volume= Primary 12,862 cf Routed to Pond IB 7.1 : Infiltration Basin

		Postdevelopment
AP7	Type III 24-hr	100YearMass Rainfall=7.93"
Prepared by Mcclure Engineeering		Printed 5/1/2023
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software	Solutions LLC	Page 67

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 727.90' @ 12.16 hrs Surf.Area= 3,301 sf Storage= 1,854 cf

Plug-Flow detention time= 14.8 min calculated for 12,862 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (796.6 - 783.8)

Volume	Invert	Avail.Storage	Storage Description
#1	724.25'	188 cf	12.0" Round Pipe Storage x 4 Inside #2
			L= 60.0'
#2	724.25'	1,077 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 4
			2,880 cf Overall - 188 cf Embedded = 2,692 cf x 40.0% Voids
#3	727.25'	120 cf	4.00'W x 60.00'L x 0.25'H Mulch × 4
			240 cf Overall x 50.0% Voids
#4	727.50'	1,493 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 × 4
		2,878 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	724.25'	6.0" Round Culvert X 4.00
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 724.25' / 724.25' S= 0.0000 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	728.00'	6.0" Horiz. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#3	Device 1	724.25'	2.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads
#4	Device 1	726.50'	4.0" Vert. Orifice/Grate X 4.00 C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=2.66 cfs @ 12.16 hrs HW=727.90' TW=707.81' (Dynamic Tailwater)

-1=Culvert (Passes 2.66 cfs of 6.97 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.79 cfs @ 9.09 fps)

-4=Orifice/Grate (Orifice Controls 1.87 cfs @ 5.35 fps)

Summary for Pond RG-26/27: Rain Garden Lots 26-27

Inflow Are	a =	13,605 sf, 65.00% Impervious, Inflow Depth > 6.73" for 100YearMass	event			
Inflow	=	2.39 cfs @ 12.07 hrs, Volume= 7,632 cf				
Outflow	=	I.63 cfs @ 12.15 hrs, Volume= 7,614 cf, Atten= 32%, Lag= 4.6 r	min			
Primary	=	I.63 cfs @ 12.15 hrs, Volume= 7,614 cf				
Routed to Pond IB 7.1 : Infiltration Basin						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 748.08' @ 12.15 hrs Surf.Area= 1,747 sf Storage= 1,059 cf

Plug-Flow detention time= 14.4 min calculated for 7,611 cf (100% of inflow) Center-of-Mass det. time= 12.9 min (787.6 - 774.8)

Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

HydroCAD	<u>® 10.20-2g_s</u>	<u>/n 03362 © 2022 H</u>	HydroCAD Software Solutions LLC Page 68
Volume	Invert	Avail.Storage	Storage Description
#1	744.25'	94 cf	12.0" Round Pipe Storage x 2 Inside #2
#2	744.25'	538 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravelx 2
#3	747.25'	60 cf	1,440 cf Overall - 94 cf Embedded = 1,346 cf x 40.0% Voids 4.00'W x 60.00'L x 0.25'H Mulch x 2
#4	747.50'	747 cf	120 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
	141.00		Total Available Storage

Device Routing Invert Outlet Devices #1 Primary 744.25' 6.0" Round Culvert X 2.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 744.25' / 744.15' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf 6.0" Horiz. Orifice/Grate X 2.00 C= 0.600 #2 Device 1 748.00' Limited to weir flow at low heads #3 Device 1 744.25' 2.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads 4.0" Vert. Orifice/Grate X 2.00 C= 0.600 #4 Device 1 746.50' Limited to weir flow at low heads

Primary OutFlow Max=1.63 cfs @ 12.15 hrs HW=748.08' TW=707.75' (Dynamic Tailwater)

-1=Culvert (Passes 1.63 cfs of 3.58 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.23 cfs @ 0.92 fps)

-3=Orifice/Grate (Orifice Controls 0.41 cfs @ 9.32 fps)

4=Orifice/Grate (Orifice Controls 1.00 cfs @ 5.72 fps)

Summary for Pond RG-28/29: Rain Garden Lots 28-29

Inflow Are	a =	17,105 sf, 65.00% Impervious,	Inflow Depth > 6.73" for 100YearMass event		
Inflow	=	3.01 cfs @ 12.07 hrs, Volume=	9,596 cf		
Outflow	=	2.38 cfs @ 12.13 hrs, Volume=	9,575 cf, Atten= 21%, Lag= 3.4 min		
Primary	=	2.38 cfs @ 12.13 hrs, Volume=	9,575 cf		
Routed to Pond IB 7.1 : Infiltration Basin					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 743.73' @ 12.13 hrs Surf.Area= 1,833 sf Storage= 1,187 cf

Plug-Flow detention time= 14.2 min calculated for 9,575 cf (100% of inflow) Center-of-Mass det. time= 12.8 min (787.5 - 774.8)

AP7

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

r roparea by meetare Engineeening	
HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 69

Volume	Invert	Avail.Sto	rage	Storage Description
#1	739.75'	ę	94 cf	
#2	739.75'	53	38 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 2 1,440 cf Overall - 94 cf Embedded = 1,346 cf x 40.0% Voids
#3	742.75'	6	60 cf	4.00'W x 60.00'L x 0.25'H Mulch x 2
#4	743.00'	74	17 cf	120 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 2
		1,439 cf Total Available Storage		
Device	Routing	Invert	Outl	et Devices
#1	Primary	739.75'	L= 1 Inlet	Round Culvert X 2.00 0.0' CPP, square edge headwall, Ke= 0.500 : / Outlet Invert= 739.75' / 739.65' S= 0.0100 '/' Cc= 0.900 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	743.50'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600	
#3	Device 1	739.75'	Limited to weir flow at low heads 2.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads	
#4	Device 1	742.00'	4.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads	

Primary OutFlow Max=2.38 cfs @ 12.13 hrs HW=743.73' TW=707.59' (Dynamic Tailwater)

-1=Culvert (Passes 2.38 cfs of 3.65 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.91 cfs @ 2.33 fps)

-3=Orifice/Grate (Orifice Controls 0.41 cfs @ 9.51 fps)

-4=Orifice/Grate (Orifice Controls 1.05 cfs @ 6.03 fps)

Summary for Pond RG15H: Rain Garden 15H

Inflow Area =	10,500 sf, 65.00% Impervious,	Inflow Depth > 6.73" for 100YearMass event			
Inflow =	1.85 cfs @ 12.07 hrs, Volume=	5,890 cf			
Outflow =	1.72 cfs @ 12.10 hrs, Volume=	5,666 cf, Atten= 7%, Lag= 1.7 min			
Discarded =	0.04 cfs @ 11.80 hrs, Volume=	1,022 cf			
Primary =	1.68 cfs @ 12.10 hrs, Volume=	4,645 cf			
Routed to Pond IB 7.1 : Infiltration Basin					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 719.41' @ 12.10 hrs Surf.Area= 720 sf Storage= 565 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 14.4 min (789.2 - 774.8)

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

	0	0		
HydroCAD® 10.20-2g	s/n 03362	© 2022 Hy	droCAD Software Solutions LLC	Page 70

Volume	Invert	Avail.Storage	Storage Description		
#1	715.25'	47 cf			
			L= 60.0'		
#2	715.25'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel		
#3	718.25'	30 cf	720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids 4.00'W x 60.00'L x 0.25'H Mulch		
#5	110.25	50 61	60 cf Overall x 50.0% Voids		
#4	718.50'	240 cf	4.00'W x 60.00'L x 1.00'H Ponding		
		586 cf	Total Available Storage		
Device	Routing	Invert Out	let Devices		
#1	Primary	717.25' 8.0 '	' Round Culvert		
	•	L= 1	10.0' CPP, square edge headwall, Ke= 0.500		
			Inlet / Outlet Invert= 717.25' / 717.15' S= 0.0100 '/' Cc= 0.900		
			0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf		
#2	Device 1		'Horiz. Orifice/Grate C= 0.600		
112	Device 1		ited to weir flow at low heads		
#3	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#4	Device 1		Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads		
#5	Discarded	715.25' 2.4 '	10 in/hr Exfiltration over Surface area		

Discarded OutFlow Max=0.04 cfs @ 11.80 hrs HW=718.51' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=1.68 cfs @ 12.10 hrs HW=719.41' TW=707.38' (Dynamic Tailwater)

-1=Culvert (Passes 1.68 cfs of 2.27 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.08 cfs @ 3.09 fps)

-3=Orifice/Grate (Orifice Controls 0.34 cfs @ 6.87 fps)

-4=Orifice/Grate (Orifice Controls 0.27 cfs @ 5.46 fps)

Summary for Pond RG15L: Rain Garden 15L

Inflow Area =	8,190 sf, 65.00% Impervious,	Inflow Depth > 6.73" for 100YearMass event			
Inflow =	1.44 cfs @ 12.07 hrs, Volume=	4,594 cf			
Outflow =	1.26 cfs @ 12.11 hrs, Volume=	4,373 cf, Atten= 13%, Lag= 2.5 min			
Discarded =	0.04 cfs @ 11.91 hrs, Volume=	983 cf			
Primary =	1.22 cfs @ 12.11 hrs, Volume=	3,390 cf			
Routed to Pond IB 7.1 : Infiltration Basin					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 710.86' @ 12.11 hrs Surf.Area= 720 sf Storage= 552 cf

Plug-Flow detention time= 44.6 min calculated for 4,372 cf (95% of inflow) Center-of-Mass det. time= 17.3 min (792.0 - 774.8)

AP7

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC	Page 71

Volume	Invert	Avail.Storage	Storage Description
#1	706.75'	47 cf	
#2	706.75'	269 cf	L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	709.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
#4	710.00'	240 cf	60 cf Overall x 50.0% Voids 4.00'W x 60.00'L x 1.00'H Ponding
		586 cf	Total Available Storage
Device	Routing	Invert Ou	tlet Devices
#1	Primary	••••	" Round Culvert
			10.0' CPP, square edge headwall, Ke= 0.500 et / Outlet Invert= 708.75' / 708.65' S= 0.0100 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1		"Horiz. Orifice/Grate C= 0.600
#3 #4 #5	Device 1 Device 1 Discarded	708.75' 2.0 709.25' 4.0	 nited to weir flow at low heads "Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads "Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads 10 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.04 cfs @ 11.91 hrs HW=710.01' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=1.22 cfs @ 12.11 hrs HW=710.86' TW=707.47' (Dynamic Tailwater)

-1=Culvert (Passes 1.22 cfs of 1.29 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.56 cfs @ 2.87 fps)

-3=Orifice/Grate (Orifice Controls 0.15 cfs @ 6.85 fps)

-4=Orifice/Grate (Orifice Controls 0.50 cfs @ 5.78 fps)

Summary for Pond RG16: Rain Garden Lot 16

Inflow Area =	6,745 sf, 65.00% Impervious,	Inflow Depth > 6.73" for 100YearMass event			
Inflow =	1.19 cfs @ 12.07 hrs, Volume=	3,784 cf			
Outflow =	1.06 cfs @ 12.11 hrs, Volume=	3,574 cf, Atten= 11%, Lag= 2.3 min			
Discarded =	0.04 cfs @ 11.97 hrs, Volume=	956 cf			
Primary =	1.02 cfs @ 12.11 hrs, Volume=	2,618 cf			
Routed to Pond IB 7.1 : Infiltration Basin					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 734.18' @ 12.11 hrs Surf.Area= 720 sf Storage= 510 cf

Plug-Flow detention time= 50.7 min calculated for 3,574 cf (94% of inflow) Center-of-Mass det. time= 19.9 min (794.6 - 774.8)

AP7

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 72			•	•		
	Hydr	oCAD® 10.20-2g	s/n 03362	© 2022 Hyd	droCAD Software Solutions LLC	Page 72

Volume	Invert	Avail Storago	Storage Description
	IIIVEIL	Avail.Storage	Storage Description
#1	730.25'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
що	700 051	000 of	
#2	730.25'	269 cf	
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	733.25'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	733.50'	240 cf	
	755.50	240 0	
		586 cf	Total Available Storage
			•
Device	Routing	Invert Out	tlet Devices
#1	Primary	732.25' 6.0 '	" Round Culvert
		L=	6.0' CPP, square edge headwall, Ke= 0.500
			t / Outlet Invert= 732.25' / 732.00' S= 0.0417 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	734.00' 6.0 '	"Horiz. Orifice/Grate C= 0.600
		Lim	ited to weir flow at low heads
#3	Device 1	732.25' 2.0	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1		"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	730.25' 2.4 '	10 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.04 cfs @ 11.97 hrs HW=733.50' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=1.02 cfs @ 12.11 hrs HW=734.18' TW=707.45' (Dynamic Tailwater)

—1=Culvert (Passes 1.02 cfs of 1.23 cfs potential flow)

2=Orifice/Grate (Weir Controls 0.40 cfs @ 1.40 fps)

-3=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.55 fps)

-4=Orifice/Grate (Orifice Controls 0.47 cfs @ 5.42 fps)

Summary for Pond RG21: Rain Garden 21

Inflow Area = 7,915 sf, 65.00% Impervious, Inflow Depth > 6.14" for 100YearMass event Inflow 1.31 cfs @ 12.07 hrs, Volume= 4.050 cf = 1.09 cfs @ 12.12 hrs, Volume= 3,841 cf, Atten= 17%, Lag= 3.0 min Outflow = Discarded = 0.05 cfs @ 12.12 hrs, Volume= 922 cf 1.04 cfs @ 12.12 hrs, Volume= 2,918 cf Primary = Routed to Link P7 : AP7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 728.70' @ 12.12 hrs Surf.Area= 857 sf Storage= 552 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 17.0 min (806.4 - 789.3)

AP7

Prepared by Mcclure Engineeering

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

	0	0		
HydroCAD® 10.20-2g	s/n 03362	© 2022 Hy	vdroCAD Software Solutions LLC	Page 73

Volume	Invert	Avail Storage	Storage Description
-		U	
#1	724.75'	31 cf	12.0" Round Pipe Storage Inside #2
			L= 40.0'
#2	724.75'	275 cf	
#2	724.75	275 0	
			720 cf Overall - 31 cf Embedded = 689 cf x 40.0% Voids
#3	727.75'	30 cf	6.00'W x 40.00'L x 0.25'H Mulch
			60 cf Overall_x 50.0% Voids
#4	728.00'	337 of	6.00'W x 40.00'L x 1.00'H Ponding Z=2.0
#4	120.00	557 61	
		674 cf	Total Available Storage
			-
Device	Routing	Invert Ou	tlet Devices
#1	Primary	726.75' 6.0	" Round Culvert
		L=	10.0' CPP, square edge headwall, Ke= 0.500
			et / Outlet Invert= 726.75' / 726.65' S= 0.0100 '/' Cc= 0.900
			0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	728.50' 6.0	"Horiz. Orifice/Grate C= 0.600
		Lin	nited to weir flow at low heads
#3	Device 1	726.75' 2.0	"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1		"Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	724.75' 2.4	10 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.05 cfs @ 12.12 hrs HW=728.70' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.05 cfs)

Primary OutFlow Max=1.04 cfs @ 12.12 hrs HW=728.70' TW=0.00' (Dynamic Tailwater)

-1=Culvert (Passes 1.04 cfs of 1.23 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.42 cfs @ 2.16 fps)

-3=Orifice/Grate (Orifice Controls 0.14 cfs @ 6.58 fps)

-4=Orifice/Grate (Orifice Controls 0.48 cfs @ 5.46 fps)

Summary for Pond RG22-25: Rain Gardens 22-25

Inflow Area = 99,920 sf, 32.59% Impervious, Inflow Depth > 5.43" for 100YearMass event Inflow 11.55 cfs @ 12.18 hrs, Volume= 45.215 cf = 10.81 cfs @ 12.23 hrs, Volume= 45,143 cf, Atten= 6%, Lag= 2.9 min Outflow = 8.11 cfs @ 12.23 hrs, Volume= 33,858 cf Primary = Routed to Pond IB 7.1 : Infiltration Basin Secondary = 2.70 cfs @ 12.23 hrs, Volume= 11,286 cf Routed to Link P7 : AP7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 728.89' @ 12.23 hrs Surf.Area= 3,838 sf Storage= 2,653 cf

Plug-Flow detention time= 4.8 min calculated for 45,125 cf (100% of inflow) Center-of-Mass det. time= 3.9 min (814.5 - 810.7)

AP7

Prepared by Mcclure Engineeering

Postdevelopment AP7 Type III 24-hr 100YearMass Rainfall=7.93" Prepared by Mcclure Engineeering Printed 5/1/2023 HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC Page 74 Avail.Storage Storage Description Volume Invert #1 724.75' 12.0" Round Pipe Storage x 4 Inside #2 188 cf L = 60.0'#2 724.75' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel x 4 1,077 cf 2,880 cf Overall - 188 cf Embedded = 2,692 cf \times 40.0% Voids #3 727.75 120 cf 4.00'W x 60.00'L x 0.25'H Mulch x 4 240 cf Overall x 50.0% Voids #4 728.00' 1,493 cf 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0 x 4 2,878 cf Total Available Storage Device Routing Invert Outlet Devices #1 Primary 724.75' 8.0" Round Culvert X 3.00 L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 724.75' / 724.75' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf #2 Secondary 724.75' 8.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 724.75' / 724.75' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf 6.0" Horiz. Orifice/Grate X 6.00 C= 0.600 #3 Device 1 728.50' Limited to weir flow at low heads #4 Device 2 728.50' 6.0" Horiz. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads #5 Device 1 724.75 3.0" Vert. Orifice/Grate X 6.00 C= 0.600 Limited to weir flow at low heads #6 Device 2 724.75' 3.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads #7 Device 1 727.25' 3.0" Vert. Orifice/Grate X 6.00 C= 0.600 Limited to weir flow at low heads #8 Device 2 727.25' 3.0" Vert. Orifice/Grate X 2.00 C= 0.600 Limited to weir flow at low heads Primary OutFlow Max=8.11 cfs @ 12.23 hrs HW=728.89' TW=708.25' (Dynamic Tailwater) **1=Culvert** (Passes 8.11 cfs of 9.83 cfs potential flow)

- -1=Culvert (Passes 8.11 cts of 9.83 cts potential flow)
 - **3=Orifice/Grate** (Orifice Controls 3.52 cfs @ 2.99 fps)
 - -5=Orifice/Grate (Orifice Controls 2.84 cfs @ 9.64 fps)
 - **7=Orifice/Grate** (Orifice Controls 1.74 cfs @ 5.92 fps)

Secondary OutFlow Max=2.70 cfs @ 12.23 hrs HW=728.89' TW=0.00' (Dynamic Tailwater) 2=Culvert (Passes 2.70 cfs of 3.28 cfs potential flow) 4=Orifice/Grate (Orifice Controls 1.17 cfs @ 2.99 fps) 6=Orifice/Grate (Orifice Controls 0.95 cfs @ 9.64 fps)

-8=Orifice/Grate (Orifice Controls 0.58 cfs @ 5.92 fps)

Summary for Pond RG27: Rain Garden Lot 27

 Inflow Area =
 10,800 sf, 23.14% Impervious, Inflow Depth > 5.55" for 100YearMass event

 Inflow =
 1.40 cfs @
 12.13 hrs, Volume=
 4,995 cf

 Outflow =
 1.19 cfs @
 12.20 hrs, Volume=
 4,983 cf, Atten= 15%, Lag= 4.0 min

 Primary =
 1.19 cfs @
 12.20 hrs, Volume=
 4,983 cf

 Routed to Pond IB 7.1 : Infiltration Basin
 4,983 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 758.73' @ 12.20 hrs Surf.Area= 916 sf Storage= 592 cf

Plug-Flow detention time= 14.2 min calculated for 4,981 cf (100% of inflow) Center-of-Mass det. time= 12.7 min (818.5 - 805.7)

Volume	Invert	Avail.Storage	Storage Description
#1	754.75'	47 cf	12.0" Round Pipe Storage Inside #2
			L= 60.0'
#2	754.75'	269 cf	4.00'W x 60.00'L x 3.00'H Soil Media and Gravel
			720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	757.75'	30 cf	4.00'W x 60.00'L x 0.25'H Mulch
			60 cf Overall x 50.0% Voids
#4	758.00'	373 cf	4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		720 cf	Total Available Storage

Device	Routing	Invert	Outlet Devices
#1	Primary	754.75'	6.0" Round Culvert
			L= 36.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 754.75' / 753.50' S= 0.0347 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#2	Device 1	758.50'	6.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads
#3	Device 1	754.75'	2.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	757.00'	4.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.19 cfs @ 12.20 hrs HW=758.73' TW=708.11' (Dynamic Tailwater)

-1=Culvert (Passes 1.19 cfs of 1.64 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 0.45 cfs @ 2.31 fps)

-3=Orifice/Grate (Orifice Controls 0.21 cfs @ 9.51 fps)

-4=Orifice/Grate (Orifice Controls 0.53 cfs @ 6.02 fps)

Summary for Pond RG28: Rain Garden Lot 28

Inflow Are	a =	28,665 sf	, 9.64% Impervious,	Inflow Depth > 4.85" for 100YearMass event
Inflow	=	3.12 cfs @	12.16 hrs, Volume=	11,591 cf
Outflow	=	2.95 cfs @	12.20 hrs, Volume=	11,578 cf,Atten= 5%,Lag= 2.4 min
Primary	=	2.95 cfs @	12.20 hrs, Volume=	11,578 cf
Routed	l to Ponc	I IB 7.1 : Infilt	ration Basin	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 757.80' @ 12.20 hrs Surf.Area= 934 sf Storage= 622 cf

Plug-Flow detention time= 4.1 min calculated for 11,578 cf (100% of inflow) Center-of-Mass det. time= 3.4 min (823.9 - 820.5)

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

HydroCAD® 10.20-2g s/n 03362 © 2022 HydroCAD Software Solutions LLC P	<u>age 76</u>

Volume	Invert	Avail.Sto	rage	Storage Description
#1	753.75'	2	17 cf	
#2	753.75'	269 cf		L= 60.0' 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids
#3	756.75'	30 cf		
#4	757.00'	373 cf		4.00'W x 60.00'L x 1.00'H Ponding Z=2.0
		72	20 cf	Total Available Storage
Device	Routing	Invert	Outl	et Devices
#1	Primary	753.75'		Round Culvert
				0.0' CPP, square edge headwall, Ke= 0.500
				/ Outlet Invert= 753.75' / 753.50' S= 0.0083 '/' Cc= 0.900
	D · · · ·			0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#2	Device 1			Horiz. Orifice/Grate X 2.00 C= 0.600
#2	Davias 1	750 751		ted to weir flow at low heads
#3	Device 1	753.75'		Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads
#4	Device 1	756.25'	4.0"	Vert. Orifice/Grate X 2.00 C= 0.600 ted to weir flow at low heads

Primary OutFlow Max=2.95 cfs @ 12.20 hrs HW=757.80' TW=708.10' (Dynamic Tailwater)

-1=Culvert (Passes 2.95 cfs of 3.02 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.03 cfs @ 2.62 fps)

-3=Orifice/Grate (Orifice Controls 0.94 cfs @ 9.54 fps)

-4=Orifice/Grate (Orifice Controls 0.99 cfs @ 5.66 fps)

Summary for Pond RG30: Rain Garden Lot 30

Inflow Are	a =	6,740 sf, 65.00% Impervious,	Inflow Depth > 6.73" for 100YearMass event		
Inflow	=	1.18 cfs @ 12.07 hrs, Volume=	3,781 cf		
Outflow	=	0.80 cfs @ 12.15 hrs, Volume=	3,772 cf, Atten= 32%, Lag= 4.7 min		
Primary	=	0.80 cfs @ 12.15 hrs, Volume=	3,772 cf		
Routed to Pond IB 7.1 : Infiltration Basin					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 731.57' @ 12.15 hrs Surf.Area= 872 sf Storage= 526 cf

Plug-Flow detention time= 14.5 min calculated for 3,770 cf (100% of inflow) Center-of-Mass det. time= 12.9 min (787.6 - 774.8)

AP7

Prepared by Mcclure Engineeering

Postdevelopment Type III 24-hr 100YearMass Rainfall=7.93"

Printed 5/1/2023

H	/droCAD®	0 10.20-2g	s/n 03362	© 2022 HydroCAI) Software Solutions LLC	Page 77

Volume	Invert	Avail.Storage	e Storage Description	
#1	727.75'	<u>v</u>	of 12.0" Round Pipe Storage Inside #2	
#2	727.75'	269 c	L= 60.0' cf 4.00'W x 60.00'L x 3.00'H Soil Media and Gravel 720 cf Overall - 47 cf Embedded = 673 cf x 40.0% Voids	
#3	730.75'	30 c		
#4	731.00'	373 c	of 4.00'W x 60.00'L x 1.00'H Ponding Z=2.0	
		720 c		
Device	Routing	Invert Ou	utlet Devices	
#1	Primary		0" Round Culvert	
		Inl	= 10.0' CPP, square edge headwall, Ke= 0.500 let / Outlet Invert= 727.75' / 727.65' S= 0.0100 '/' Cc= 0.900	
#2	Device 1	731.50' 6.0	 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf 0" Horiz. Orifice/Grate C= 0.600 mited to weir flow at low heads 	
#3	Device 1		0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads	
#4	Device 1	730.00' 4.0	0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads	
Primary OutFlow Max=0.80 cfs @ 12.15 hrs HW=731.57' TW=707.76' (Dynamic Tailwater)				

1=**Culvert** (Passes 0.80 cfs of 1.79 cfs potential flow)

-2=Orifice/Grate (Weir Controls 0.10 cfs @ 0.88 fps) -3=Orifice/Grate (Orifice Controls 0.20 cfs @ 9.31 fps)

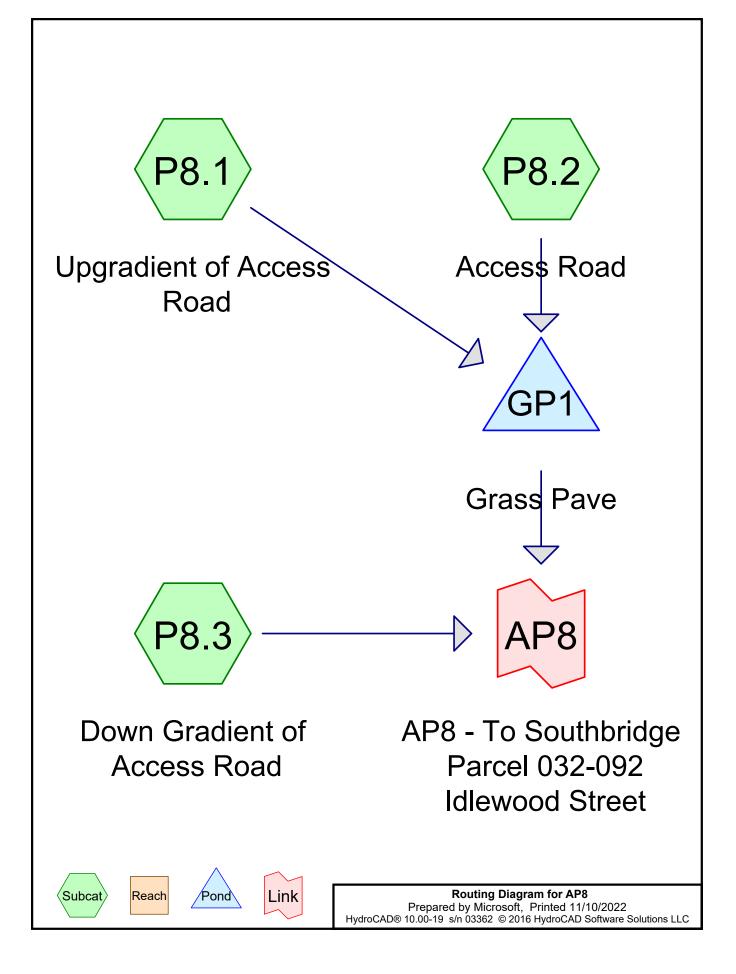
-4=Orifice/Grate (Orifice Controls 0.50 cfs @ 5.71 fps)

Summary for Link P7: AP7

Inflow Are	a =	400,631 sf, 31.37% Impervious, Inflow Depth > 3.71" for 100YearMass event
Inflow	=	15.48 cfs @ 12.32 hrs, Volume= 123,993 cf
Primary	=	15.48 cfs @ 12.32 hrs, Volume= 123,993 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

AP7 Prepared by Mcclure Engineeering



Runoff = 0.24 cfs @ 12.16 hrs, Volume= 958 cf, Depth> 0.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

_	A	rea (sf)	CN [Description		
		10,315	70 V	Voods, Go	od, HSG C	
_		2,460	74 >	75% Gras	s cover, Go	bod, HSG C
		12,775	71 V	Veighted A	verage	
		12,775	1	00.00% Pe	ervious Are	а
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.8	50	0.0500	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	1.6	110	0.0500	1.12		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	10.4	160	Total			

10.4 160 Total

Summary for Subcatchment P8.2: Access Road

Runoff = 0.13 cfs @ 12.08 hrs, Volume= 395 cf, Depth> 1.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Α	rea (sf)	CN E	Description					
	4,460	74 >	74 >75% Grass cover, Good, HSG C					
	4,460	1	00.00% Pe	ervious Are	a			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P8.3: Down Gradient of Access Road

Runoff = 0.08 cfs @ 12.15 hrs, Volume= 310 cf, Depth> 0.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2YearMass Rainfall=3.24"

Area (sf)	CN	Description
4,095	70	Woods, Good, HSG C
290	74	>75% Grass cover, Good, HSG C
4,385 4,385		Weighted Average 100.00% Pervious Area

•	d by Mic D® 10.00-		362 © 201	6 HydroCAE	Postdevelopment <i>Type III 24-hr 2YearMass Rainfall=3.24"</i> Printed 11/10/2022 Software Solutions LLC Page 3		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
8.8	50	0.0500	0.09		Sheet Flow,		
0.7	50	0.0500	1.12		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps		
9.5	100	Total					
			Sum	mary for	Pond GP1: Grass Pave		
Inflow Outflow Discarde Primary	Outflow = 0.10 cfs @ 12.02 hrs, Volume= 1,353 cf, Atten= 70%, Lag= 0.0 min Discarded = 0.10 cfs @ 12.02 hrs, Volume= 1,353 cf Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf						
					.00-24.00 hrs, dt= 0.01 hrs 800 sf Storage= 267 cf		
				ated: outflo 884.1 - 869	w precedes inflow) 9.6)		
Volume	Inv	ert A	vail.Storag	e Storage	e Description		
#1	764.0	00'	958		x 225.00'L x 1.33'H Prismatoid of Overall x 40.0% Voids		
Device	Routing		Invert O	utlet Devic	es		
#1 #2	#1 Discarded 764.00' 2.410 in/hr Exfiltration over Surface area						
	Discarded OutFlow Max=0.10 cfs @ 12.02 hrs HW=764.01' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.10 cfs)						
	Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=764.00' TW=0.00' (Dynamic Tailwater) 1 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)						
Sı	ummary	/ for Lin	k AP8: /	AP8 - To \$	Southbridge Parcel 032-092 Idlewood Street		
Inflow Ar Inflow Primary	=	0.08 cfs	s @ 12.1	00% Imperv 5 hrs, Volu 5 hrs, Volu			
Primary	outflow =	Inflow, T	ime Span=	0.00-24.00	0 hrs, dt= 0.01 hrs		

Runoff = 0.63 cfs @ 12.15 hrs, Volume= 2,288 cf, Depth> 2.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

_	A	rea (sf)	CN [Description		
		10,315	70 V	Voods, Go	od, HSG C	
_		2,460	74 >	-75% Gras	s cover, Go	ood, HSG C
		12,775	71 V	Veighted A	verage	
12,775 100.00% Pervious Area					ervious Are	а
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.8	50	0.0500	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	1.6	110	0.0500	1.12		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	10.4	160	Total			

10.4 160 Total

Summary for Subcatchment P8.2: Access Road

Runoff = 0.30 cfs @ 12.08 hrs, Volume= 893 cf, Depth> 2.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Area	(sf) CN	N D	escription					
4,4	60 74	74 >75% Grass cover, Good, HSG C						
4,4	60	10	00.00% Pe	ervious Are	a			
Tc Leı (min) (f	0	lope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P8.3: Down Gradient of Access Road

Runoff = 0.21 cfs @ 12.14 hrs, Volume= 756 cf, Depth> 2.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10YearMass Rainfall=5.05"

Area (sf)	CN	Description
4,095	70	Woods, Good, HSG C
290	74	>75% Grass cover, Good, HSG C
4,385 4,385		Weighted Average 100.00% Pervious Area

	d by Mic D® 10.00-		362 © 201	6 HydroCAE	Postdevelopment <i>Type III 24-hr 10YearMass Rainfall=5.05"</i> Printed 11/10/2022 Software Solutions LLC Page 5		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
8.8	50	0.0500	0.09		Sheet Flow,		
0.7	50	0.0500	1.12		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps		
9.5	100	Total					
			Sum	mary for	Pond GP1: Grass Pave		
Inflow Outflow Discarde Primary	Outflow = 0.94 cfs @ 12.18 hrs, Volume= 3,181 cf, Atten= 0%, Lag= 3.6 min Discarded = 0.10 cfs @ 11.77 hrs, Volume= 2,560 cf						
					.00-24.00 hrs, dt= 0.01 hrs 800 sf Storage= 549 cf		
				lated: outflo 878.8 - 843	w precedes inflow) 3.9)		
Volume	Inv	ert Av	vail.Storag	e Storage	e Description		
#1	764.0	00'	958		x 225.00'L x 1.33'H Prismatoid of Overall x 40.0% Voids		
Device	Routing		Invert C	utlet Devic	es		
#1 #2	Discarde Primary		64.75' 2 H 2 C	25.0' long lead (feet) .50 3.00 3 coef. (Englis	Exfiltration over Surface area x 5.0' breadth Broad-Crested Rectangular Weir 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 .50 4.00 4.50 5.00 5.50 sh) 2.34 2.50 2.70 2.68 2.66 2.65 2.65 2.65 .66 2.68 2.70 2.74 2.79 2.88		
	Discarded OutFlow Max=0.10 cfs @ 11.77 hrs HW=764.01' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.10 cfs)						
	Primary OutFlow Max=0.73 cfs @ 12.18 hrs HW=764.76' TW=0.00' (Dynamic Tailwater) 1 2=Broad-Crested Rectangular Weir (Weir Controls 0.73 cfs @ 0.26 fps)						
Sı	ummary	for Lin	k AP8: /	\P8 - To \$	Southbridge Parcel 032-092 Idlewood Street		
Inflow Ar Inflow Primary Primary	= =	1.04 cfs 1.04 cfs	s@ 12.1 s@ 12.1	8 hrs, Volu 8 hrs, Volu			

Runoff = 0.90 cfs @ 12.15 hrs, Volume= 3,234 cf, Depth> 3.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

_	A	rea (sf)	CN [Description		
		10,315	70 V	Voods, Go	od, HSG C	
_		2,460	74 >	-75% Gras	s cover, Go	ood, HSG C
		12,775	71 V	Veighted A	verage	
12,775 100.00% Pervious Area					ervious Are	а
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.8	50	0.0500	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	1.6	110	0.0500	1.12		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	10.4	160	Total			

10.4 160 Total

Summary for Subcatchment P8.2: Access Road

Runoff = 0.42 cfs @ 12.08 hrs, Volume= 1,239 cf, Depth> 3.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Α	rea (sf)	CN E	Description					
	4,460	74 >	74 >75% Grass cover, Good, HSG C					
	4,460	1	00.00% Pe	ervious Are	a			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry,			

Summary for Subcatchment P8.3: Down Gradient of Access Road

Runoff = 0.31 cfs @ 12.14 hrs, Volume= 1,075 cf, Depth> 2.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 25YearMass Rainfall=6.18"

Area (sf)	CN	Description
4,095	70	Woods, Good, HSG C
290	74	>75% Grass cover, Good, HSG C
4,385 4,385	70	Weighted Average 100.00% Pervious Area

AP8 Preparec <u>HydroCAD</u>			362 © 201	6 HydroCAE	Postdevelopment <i>Type III 24-hr 25YearMass Rainfall=6.18"</i> Printed 11/10/2022 Software Solutions LLC Page 7
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.8	50	0.0500	0.09		Sheet Flow,
0.7	50	0.0500	1.12		Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
9.5	100	Total			· · ·
			Sum	mary for	Pond GP1: Grass Pave
Inflow Are Inflow Outflow Discarded Primary Routing b	= = = t =	1.23 cfs 1.27 cfs 0.10 cfs 1.17 cfs	s @ 12.1 s @ 12.1 s @ 11.6 s @ 12.1	2 hrs, Volu 0 hrs, Volu 7 hrs, Volu 0 hrs, Volu	Ime= 4,474 cf, Atten= 0%, Lag= 0.0 min Ime= 3,093 cf
Peak Elev	v= 764.7	7' @ 12.1	I0 hrs Su	rf.Area= 1,8	800 sf Storage= 552 cf
				ated: outflo 866.2 - 834	w precedes inflow) 4.1)
Volume	Inv	ert Av	vail.Storag	e Storage	e Description
#1	764.0	00'	958		x 225.00'L x 1.33'H Prismatoid of Overall x 40.0% Voids
	Routing			utlet Devic	
	Discarde Primary		64.75' 2 : H 2 C	25.0' long ead (feet) .50 3.00 3 oef. (Englis	Exfiltration over Surface area x 5.0' breadth Broad-Crested Rectangular Weir 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 .50 4.00 4.50 5.00 5.50 sh) 2.34 2.50 2.70 2.68 2.66 2.65 2.65 2.65 .66 2.68 2.70 2.74 2.79 2.88
) 11.67 hrs ls 0.10 cfs)	HW=764.01' (Free Discharge)
					IW=764.77' TW=0.00' (Dynamic Tailwater) Controls 1.17 cfs @ 0.31 fps)
Su	mmary	for Lin	k AP8: A	AP8 - To \$	Southbridge Parcel 032-092 Idlewood Street
Inflow Are Inflow Primary	=	1.46 cfs	s@ 12.1	0 hrs. Volu	vious, Inflow Depth > 1.36" for 25YearMass event me= 2,456 cf me= 2,456 cf, Atten= 0%, Lag= 0.0 min
Primary o	outflow =	Inflow, Ti	ime Span=	• 0.00-24.00	0 hrs, dt= 0.01 hrs

Runoff = 1.34 cfs @ 12.14 hrs, Volume= 4,800 cf, Depth> 4.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

_	A	rea (sf)	CN [Description		
		10,315	70 V	Voods, Go	od, HSG C	
_		2,460	74 >	-75% Gras	s cover, Go	ood, HSG C
		12,775	71 V	Veighted A	verage	
		12,775	1	00.00% Pe	ervious Are	а
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.8	50	0.0500	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.00"
	1.6	110	0.0500	1.12		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	10.4	160	Total			

10.4 160 Total

Summary for Subcatchment P8.2: Access Road

Runoff = 0.60 cfs @ 12.07 hrs, Volume= 1,806 cf, Depth> 4.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Α	rea (sf)	CN E	Description							
	4,460	74 >	>75% Grass cover, Good, HSG C							
	4,460	1	100.00% Pervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					

Summary for Subcatchment P8.3: Down Gradient of Access Road

Runoff = 0.46 cfs @ 12.13 hrs, Volume= 1,606 cf, Depth> 4.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100YearMass Rainfall=7.93"

Are	ea (sf)	CN	Description
	4,095	70	Woods, Good, HSG C
	290	74	>75% Grass cover, Good, HSG C
	4,385 4,385	70	Weighted Average 100.00% Pervious Area

AP8 Type III 24-hr 100YearMass Rainfall=7.93' Prepared by Microsoft Printed 11/10/2022 HydroCAD® 10.00-19 s/n 03362 © 2016 HydroCAD Software Solutions LLC Page 9
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)
8.8 50 0.0500 0.09 Sheet Flow,
0.7 50 0.0500 1.12 Woods: Light underbrush n= 0.400 P2= 3.00" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
9.5 100 Total
Summary for Pond GP1: Grass Pave
Inflow Area = 17,235 sf, 0.00% Impervious, Inflow Depth > 4.60" for 100YearMass event Inflow = 1.82 cfs @ 12.12 hrs, Volume= 6,606 cf Outflow = 1.82 cfs @ 12.12 hrs, Volume= 6,606 cf, Atten= 0%, Lag= 0.1 min Discarded = 0.10 cfs @ 11.35 hrs, Volume= 3,805 cf Primary = 1.72 cfs @ 12.12 hrs, Volume= 2,801 cf
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 764.77' @ 12.12 hrs Surf.Area= 1,800 sf Storage= 556 cf
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 28.1 min (851.0 - 823.0)
Volume Invert Avail.Storage Storage Description
#1 764.00' 958 cf 8.00'W x 225.00'L x 1.33'H Prismatoid 2,394 cf Overall x 40.0% Voids
Device Routing Invert Outlet Devices
#1 Discarded 764.00' 2.410 in/hr Exfiltration over Surface area #2 Primary 764.75' 2.410 in/hr Exfiltration over Surface area #2 Primary 764.75' 225.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.66 2.65 2.65 2.65 2.65 2.68 2.70 2.74 2.79 2.88
Discarded OutFlow Max=0.10 cfs @ 11.35 hrs HW=764.01' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.10 cfs)
Primary OutFlow Max=1.72 cfs @ 12.12 hrs HW=764.77' TW=0.00' (Dynamic Tailwater) 2=Broad-Crested Rectangular Weir (Weir Controls 1.72 cfs @ 0.35 fps)
Summary for Link AP8: AP8 - To Southbridge Parcel 032-092 Idlewood Street
Inflow Area = 21,620 sf, 0.00% Impervious, Inflow Depth > 2.45" for 100YearMass event Inflow = 2.18 cfs @ 12.12 hrs, Volume= 4,408 cf Primary = 2.18 cfs @ 12.12 hrs, Volume= 4,408 cf, Atten= 0%, Lag= 0.0 min
Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

McClure Engineering, Inc. March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023

APPENDIX F

ADDITIONAL DRAINAGE CALCULATION WORKSHEETS

"55+ Manufactured Housing Community", Lot 3 Berry Farms Road, Sturbridge, MA – Stormwater Management Report

	Project: Lot 3 Be	rry Farms	Road	Project	t #: 287-21	18-K		
Perfor	Description: Infiltration Basin 7.1							
				Calculate	ed Mound	Heiaht:	1.2	feet
						0		
Input Parameters (in	put only shaded a	areas):						
Recharge Period	<i>t</i> =	<u>0.5</u>	days	Basin Drains in 9	Hours			
Width of Field	W =	<u>28</u>	feet					
Length of Field	L =	<u>260</u>	feet					
Hydraulic Conductivity	K=	44.33	ft/day	Loamy Sand per	chart = 5.13E	-4 ft/s = 44.	3 ft/d	
Specific Yield	V =	0.21	ft ³ /ft ³	Loamy Sand per	chart = 0.21 (70% mediu	m san	d, 30% s ⁱ
Saturated Thickness	D =	5	feet	, ,	,			
Daily Flow	 Q =	<u>-</u> 61,033	gpd	8,159 c.f.				
Daily Flow	Q –	01,000	gpu	·				
Coloulated Paramete					ription	k (ft/s		
Calculated Paramete	<u>15.</u>			Sand Loamy		(5.77E-04)		
1/2 width	0-	14	feet	Sandy Loa	am	(5.13E-04) [1.56E-04] (1.13E-04) [3.45E-05]		
				Silty Loan	n	(2.36E-05) [7.19E-06]		
1/2 length		130		Loamy	way Laam	(2.28E-05) [6.94E-06] (2.07E-05) [6.31E-06]		
Recharge Rate	<i>j</i> =	1.12	ft/day	Silty Clay	iyey Loam ey Loam	(5.57E-06)	_	
	KD			Clay Loan		(8.04E-06)	-	-
	$\gamma = \frac{KD}{V} =$	1055.5	ft²/day		iyey Loam	(7.11E-06)	-	-
	•			Silty Clay Clay		(3.34E-06) (4.21E-06)		
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\pi}} =$	0.3047		eidy	Refere		[1120	2 00)
	$\sqrt{4\gamma t}$			Material	Kelere	Specific Y	ield	
					Maximum	Minim	um	Averag
Dimensionless length	$\beta = \frac{b}{\sqrt{4\pi}} =$	2 8295		Clay Sandy clay	0.05	0.03	3	0.02
Bintonoloniloco longin	$\sqrt{4\gamma t}$	2.0200		Silt	0.19	0.03		0.07
				Fine Sand Medium Sand	0.28	0.1		0.21
• • •				Coarse Sand	0.35	0.2		0.27
Solution:				Gravelly Sand Fine gravel	0.35	0.2		0.25
				Medium gravel	0.35	0.13		0.23
From Table 1of Hantus	<u>sh (1967), attache</u>	<u>d:</u>		Coarse gravel	0.26	0.12		0.22
Function S*(a , b) =	0.5229			Applied	Hydrology 4th	Edition, C.	W. Fel	ter
Water Table + Mound	$h_m = 1$	$\sqrt{h_i^2 + \left[\frac{2}{K}\right]}$	$\frac{\dot{l}}{d}\lambda t\cdot S*(a$	(α, β)				
	$h_m =$	6.2	feet					
Mound Height =	h _m - D =	1.2	feet					
Reference: Hantush, M.S. 19 Water Resources Research, 3		y of Groundw	vater Mounds	in Response to Un	iform Percolat	ion."		

	Project: Blueberry	Project #: 287-2119-K							
Perfor	med By: PE	Description: RG Type C							
				Calculated Mound Height: 0.1 feet					
Input Parameters (inp	put only shaded a	ireas):							
Recharge Period	<i>t</i> =	<u>0.2</u>	days	Basin Drains in 4	.5 Hours				
Width of Field	W =	<u>4</u>	feet						
Length of Field	L =	<u>60</u>	feet						
Hydraulic Conductivity	K=	<u>44.3</u>	ft/day	Loamy Sand per	chart = 5.13E	-4 ft/s = 44.3 ft/o	ł		
Specific Yield	V =	0.21	ft ³ /ft ³	Loamy Sand per	chart = 0.21 (70% medium sa	nd, 30% silt)		
' Saturated Thickness	D =	5	feet		·				
Daily Flow	 Q =	<u> </u>	gpd	268 c.f.					
		<u>_,</u>	36.			1. (ft /_) [- (-1		
Calculated Paramete	ers:			Sand	ription	k (ft/s) [n (5.77E-04) [1.7			
				Loamy		(5.13E-04) [1.5			
1/2 width	a =	2	feet	Sandy Loa		(1.13E-04) [3.4			
1/2 length		30	feet	Silty Loan Loamy	n	(2.36E-05) [7.3 (2.28E-05) [6.9			
Recharge Rate		1.12	ft/day		yey Loam	(2.07E-05) [6.3			
	J —	1.12	liveay	Silty Clay	ey Loam	(5.57E-06) [1.7	70E-06]		
	$\gamma = \frac{KD}{V} =$	4054.0	c ₁ 211	Clay Loan		(8.04E-06) [2.4			
	V = V	1054.8	ft²/day	Silty Clay	yey Loam	(7.11E-06) [2.1 (3.34E-06) [1.0			
D	а			Clay		(4.21E-06) [1.2			
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$	0.0689			Refere				
	V ,			Material	Maximum	Specific Yield Minimum	Average		
	b			Clay	0.05	0	0.02		
Dimensionless length	$\beta = \frac{b}{\sqrt{4\gamma t}} =$	1.0328		Sandy clay Silt	0.12	0.03	0.07		
	$\sqrt{-\mu}$			Fine Sand	0.19	0.03	0.07		
				Medium Sand	0.32	0.15	0.26		
Solution:				Coarse Sand Gravelly Sand	0.35	0.2	0.27		
				Fine gravel	0.35	0.21	0.25		
From Table 1 of Hantus	<u>sh (1967), attache</u>	<u>d:</u>		Medium gravel Coarse gravel	0.26	0.13	0.23		
Function S*(a , b) =	0.1413			Applied	Hydrology 4th	Edition, C.W. F	etter		
Water Table + Mound	$h_m = 1$	$h_i^2 + \left\lfloor \frac{2j}{K} \right\rfloor$	$-\lambda t \cdot S * (c$	(α, β)					
	$h_m =$	5.1	feet						
		0.1	feet	_					

Water Resources Research, 3, pp. 227-234.

	Project: Blueberry	/ Hill Esta	tes	Project	#: 287-21	19-K		
Perfor	med By: PE	Description: RG Type D						
			Calculated Mound Height: 0.2 feet					
Input Parameters (inp	out only shaded a	<u>reas):</u>						
Recharge Period	<i>t</i> =	<u>0.2</u>	days	Basin Drains in 4	.5 Hours			
Width of Field	W =	<u>5</u>	feet					
Length of Field	L =	<u>60</u>	feet					
Hydraulic Conductivity	K=	<u>11.23</u>	ft/day	Sandy Loam per	chart = 1.13E-	4 ft/s = 8.86 ft/d		
Specific Yield	V=	0.19	ft ³ /ft ³	Sandy Loam per	chart = 0.15 (6	60% medium san	d, 40% silt)	
Saturated Thickness	D =	5	feet	, ,	· ·		. ,	
Daily Flow	 Q =	<u>-</u> 1,645	gpd	220 c.f.				
Daily Tiow	Q –	<u>1,040</u>	gpu					
Colouisted Devemente					ription	k (ft/s) [m		
Calculated Paramete	15.			Sand		(5.77E-04) [1.7 (5.13E-04) [1.5		
1/2 width		25	fact	Sandy Loa	am	(1.13E-04) [3.4		
	a =	2.5	feet	Silty Loan	1~ ·	(2.36E-05) [7.1		
1/2 length	<i>b</i> =		feet	Loamy	unul dam.	(2.28E-05) [6.94E-06] (2.07E-05) [6.31E-06]		
Recharge Rate	<i>j</i> =	0.73	ft/day	Sandy Cla Silty Claye		(5.57E-06) [1.7		
	KD			Clay Loan		(8.04E-06) [2.4		
	$\gamma = \frac{KD}{V} =$	295.5	ft²/day		yey Loam	(7.11E-06) [2.1		
				Silty Clay Clay		(3.34E-06) [1.0 (4.21E-06) [1.2		
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$	0.1626			Refere			
	$\sqrt{4\gamma t}$			Material	Referen	Specific Yield		
				Clay	Maximum 0.05	Minimum 0	Average 0.02	
Dimensionless length	$\beta = \frac{b}{\sqrt{4 vt}} =$	1.9511		Sandy clay	0.03	0.03	0.02	
Ũ	$\sqrt{4\gamma t}$			Silt Fine Sand	0.19	0.03	0.07	
				Medium Sand	0.20	0.15	0.21	
Solution:				Coarse Sand	0.35	0.2	0.27	
				Gravelly Sand Fine gravel	0.35	0.2	0.25	
From Table 1 of Hantus	sh (1967) attachou	4.		Medium gravel	0.26	0.13	0.23	
		<u>u.</u>		Coarse gravel Applied	0.26 Hydrology 4th	0.12 Edition, C.W. Fe	0.22 etter	
Function S*(a , b) =	0.3165				, ,,			
Water Table + Mound	$h_m = \sqrt{1}$	$h_i^2 + \left[\frac{2j}{K}\right]$	$\frac{j}{2}\lambda t\cdot S*(a$	(α,β)				
	$h_m =$	5.2	feet					
				_				

Water Resources Research, 3, pp. 227-234.

T:__2019 DOCUMENTS K\287-2118-K_JW Management_Fiske Hill East_Site Plan\Stormwater\Rev2_PE\Appendices\APP_F_RG Type D Mounding Analysis.xls Printed: 5/1/2023, 2:31 PM

	Project: Blueberr	ry Hill Esta	tes	Pro	oject	: #: 287-211	9-K	
Perfo	Desc	Description: RG2.1						
	Calculated Mound Height: 1.0 feet							
							-	
Input Parameters (in	put only shaded a	areas):						
Recharge Period	<i>t</i> =	<u>0.3</u>	days	Basin Drair	ns in 5	Hours		
Width of Field	W =	<u>50</u>	feet					
Length of Field	L =	<u>85</u>	feet					
Hydraulic Conductivity	K=	<u>44.3</u>	ft/day	Loamy Sar	nd per	chart = 5.13E-	4 ft/s = 44.3 ft/d	
Specific Yield	V =	0.21	ft ³ /ft ³	Loamy San	nd per	chart = 0.21 (7	70% medium sa	nd, 30% silt)
Saturated Thickness	D =	5	feet	,	•	,		,
Daily Flow	Q =		gpd	3,780 c.f.				
Daily 110W	Q =	20,210	gpu	·				
						ription	k (ft/s) [m	
Calculated Paramete	<u>ers:</u>			San			(5.77E-04) [1.7	
4/0		05	<i>.</i> .	Loa San	my dy Loa	am	(5.13E-04) [1.5 (1.13E-04) [3.4	-
1/2 width	a =	25	feet		y Loan		(2.36E-05) [7.1	-
1/2 length	<i>b</i> =	42.5	feet	Loa			(2.28E-05) [6.9	-
Recharge Rate	<i>j</i> =	0.89	ft/day				(2.07E-05) [6.3	
	КD				y Loan		(5.57E-06) [1.7 (8.04E-06) [2.4	
	$\gamma = \frac{KD}{V} =$	1054.8	ft²/day				(7.11E-06) [2.1	
	V		,		y Clay		(3.34E-06) [1.0	-
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$	0 7027		Clay	y		(4.21E-06) [1.2	8E-06]
	$\alpha = \frac{1}{\sqrt{4\gamma t}}$	0.7027				Referen		
	·			Materia	al	Maximum	Specific Yield Minimum	Average
	b			Clay		0.05	0	0.02
Dimensionless length	$\beta = \frac{b}{\sqrt{4\gamma t}} =$	1.1946		Sandy cl Silt	ay	0.12	0.03	0.07
	$\sqrt{-\mu}$			Fine Sar	nd	0.13	0.03	0.07
				Medium S		0.32	0.15	0.26
Solution:				Coarse Sa Gravelly S		0.35	0.2	0.27
				Fine grav		0.35	0.21	0.25
From Table 1 of Hantu	sh (1967) attach(d		Medium gr		0.26	0.13	0.23
Function S*(a , b) =		.		Coarse gr		0.26 Hydrology 4th	0.12 Edition, C.W. Fe	0.22 etter
	0.8265			· · · · · ·				
Water Table + Mound	$h_m = L$	$\sqrt{h_i^2 + \left[\frac{2j}{K}\right]}$	$-\lambda t \cdot S * (a)$	(α,β)				
	$h_m =$	6.0	feet					
Mound Height =	h _m - D =	1.0	feet	7				
				<u> </u>				
Reference: Hantush, M.S. 19		y of Groundw	ater Mounds	in Response	to Uni	form Percolation	on."	
Water Resources Research, 3	3, pp. 227-234.							

T:__2019 DOCUMENTS K\287-2118-K_JW Management_Fiske Hill East_Site Plan\Stormwater\Rev1\Appendices\APP_F_RG2.1 Mounding Analysis.xls Printed: 3/21/2023, 3:35 PM

	Project: Blueberry	y Hill Esta	tes	Projec	: t #: 287-211	19-K	
Perform	Description: RG3.1						
		Calculated Mound Height: 0.5 feet					
Input Parameters (inp	ut only shaded a	areas):					
Recharge Period	<i>t</i> =	<u>0.5</u>	days	Basin Drains in	13 Hours		
Width of Field	W =	<u>13</u>	feet				
Length of Field	L =	<u>110</u>	feet				
Hydraulic Conductivity	K =	<u>44.3</u>	ft/day	Loamy Sand pe	⁻ chart = 5.13E-	-4 ft/s = 44.3 ft/d	
Specific Yield	V =	0.21	ft ³ /ft ³	Loamy Sand pe	⁻ chart = 0.21 (7	70% medium san	id, 30% silt)
' Saturated Thickness	D =	5	feet	, , , , , , , , , , , , , , , , , , ,	,		. ,
Daily Flow	Q =	<u>s,648</u>	gpd	1,156 c.f.			
Daily 110W	Q -	0,040	gpu	1,100 с.1.			
				Des	cription	k (ft/s) [m	n/s]
Calculated Parameter	<u>s:</u>			Sand		(5.77E-04) [1.7	
			<i>.</i> .	Loamy Sandy Lo	am	(5.13E-04) [1.5 (1.13E-04) [3.4	
1/2 width	a =	6.5	feet	Silty Loa		(2.36E-05) [7.1	
1/2 length	b =	55	feet	Loamy		(2.28E-05) [6.9	4E-06]
Recharge Rate	<i>j</i> =	0.81	ft/day	<u>`</u>	ayey Loam	(2.07E-05) [6.3	
	KD			Clay Loa	/ey Loam m	(5.57E-06) [1.7 (8.04E-06) [2.4	
	$\gamma = \frac{KD}{V} =$	1054.8	ft²/day		ayey Loam	(7.11E-06) [2.1	
				Silty Clar	/	(3.34E-06) [1.0	
Dimensionless width	$\alpha = \frac{a}{a} =$	0.1415		Clay		(4.21E-06) [1.2	8E-06]
	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$				Refere	nce #1 Specific Yield	
				Material	Maximum	Minimum	Average
	_ b	4 4075		Clay	0.05	0	0.02
Dimensionless length	$\beta = \frac{b}{\sqrt{4\gamma t}} =$	1.1975		Sandy clay Silt	0.12	0.03	0.07
	V 172			Fine Sand	0.28	0.1	0.21
				Medium Sand Coarse Sand	0.32	0.15	0.26
Solution:				Gravelly Sand	0.35	0.2	0.27
				Fine gravel Medium gravel	0.35	0.21	0.25
From Table 1 of Hantus	<u>h (1967), attache</u>	<u>d:</u>		Coarse gravel	0.20	0.13	0.23
Function S*(a , b) =	0.2764			Applie	d Hydrology 4th	Edition, C.W. Fe	etter
Water Table + Mound	$h_m = 1$	$\sqrt{h_i^2 + \left[\frac{2}{K}\right]}$	$\frac{j}{2}\lambda t \cdot S^*($	(α, β)			
	<i>h</i> _{<i>m</i>} =	5.5	feet				
Mound Height =	h _m - D =	0.5	feet	7			
Reference: Hantush, M.S. 1967 Water Resources Research, 3,	7. "Growth and Decay		ater Mounds	in Response to Un	iform Percolatio	on."	

β	0.02	0.04	0.06	0.08	0.10	0.14	0.18	0.22	0.26
α 0.02	0.0041	0.0073	0.0101	0.0125	0.0146	0.0184	0.0216	0.0243	0.0267
0.04	0.0073	0.0135	0.0188	0.0236	0.0278	0.0353	0.0416	0.0470	0.0518
0.06	0.0101	0.0188	0.0266	0.0335	0.0398	0.0509	0.0602	0.0684	0.0754
0.08	0.0125	0.0236	0.0335	0.0425	0.0508	0.0652	0.0776	0.0884	0.0978
0.10	0.0146	0.0278	0.0398	0.0508	0.0608	0.0786	0.0939	0.1072	0.1188
0.14	0.0184	0.0353	0.0509	0.0652	0.0786	0.1025	0.1232	0.1414	0.1573
0.18	0.0216	0.0416	0.0602	0.0776	0.0939	0.1232	0.1490	0.1716	0.1916
0.22	0.0243	0.0470	0.0684	0.0884	0.1072	0.1414	0.1716	0.1984	0.2222
0.26	0.0267	0.0518	0.0754	0.0978	0.1188	0.1573	0.1916	0.2222	0.2494
0.30	0.0288	0.0559	0.0817	0.1060	0.1290	0.1714	0.2094	0.2433	0.2737
0.34	0.0306	0.0596	0.0871	0.1133	0.1381	0.1839	0.2251	0.2621	0.2954
0.38	0.0322	0.0628	0.0920	0.1197	0.1461	0.1949	0.2391	0.2789	0.3147
0.42	0.0337	0.0657	0.0963	0.1254	0.1532	0.2048	0.2515	0.2938	0.3320
0.46	0.0349	0.0683	0.1001	0.1305	0.1595	0.2135	0.2626	0.3071	0.3474
0.50	0.0361	0.0705	0.1035	0.1350	0.1650	0.2212	0.2724	0.3189	0.3612
0.54	0.0371	0.0725	0.1065	0.1389	0.1700	0.2281	0.2812	0.3295	0.3735
0.58	0.0380	0.0743	0.1091	0.1425	0.1744	0.2343	0.2890	0.3389	0.3844
0.62	0.0387	0.0759	0.1115	0.1456	0.1783	0.2397	0.2959	0.3472	0.3941
0.66	0.0394	0.0773	0.1136	0.1484	0.1818	0.2445	0.3020	0.3547	0.4027
0.70	0.0401	0.0785	0.1154	0.1509	0.1849	0.2488	0.3075	0.3612	0.4104
0.74	0.0406	0.0796	0.1171	0.1531	0.1876	0.2526	0.3123	0.3671	0.4172
0.78	0.0411	0.0806	0.1185	0.1550	0.1900	0.2559	0.3166	0.3722	0.4232
0.82	0.0415	0.0814	0.1198	0.1567	0.1921	0.2589	0.3203	0.3768	0.4286
0.86	0.0419	0.0822	0.1209	0.1582	0.1940	0.2615	0.3237	0.3808	0.4333
0.90	0.0422	0.0828	0.1219	0.1595	0.1957	0.2638	0.3266	0.3844	0.4374
0.94	0.0425	0.0834	0.1228	0.1607	0.1971	0.2658	0.3292	0.3875	0.4411
0.98	0.0428	0.0839	0.1236	0.1617	0.1984	0.2676	0.3314	0.3902	0.4442
1.00	0.0429	0.0842	0.1239	0.1622	0.1990	0.2684	0.3324	0.3914	0.4457
1.20	0.0437	0.0858	0.1263	0.1654	0.2030	0.2740	0.3396	0.4001	0.4558
1.40	0.0441	0.0866	0.1275	0.1669	0.2049	0.2767	0.3431	0.4043	0.4608
1.80	0.0444	0.0871	0.1283	0.1680	0.2062	0.2785	0.3454	0.4071	0.4641
2.00	0.0444	0.0871	0.1284	0.1681	0.2064	0.2787	0.3457	0.4075	0.4645
2.20	0.0444	0.0872	0.1284	0.1682	0.2065	0.2788	0.3458	0.4076	0.4646
2.50	0.0444	0.0872	0.1284	0.1682	0.2065	0.2788	0.3458	0.4077	0.4647
3.00	0.0444	0.0872	0.1284	0.1682	0.2065	0.2789	0.3458	0.4077	0.4647

0.30	0.34	0.38	0.42	0.46	0.50	0.54	0.58	0.62	0.66
0.0288	0.0306	0.0322	0.0337	0.0349	0.0361	0.0371	0.0380	0.0387	0.0394
0.0559	0.0596	0.0628	0.0657	0.0683	0.0705	0.0725	0.0743	0.0759	0.0773
0.0817	0.0871	0.0920	0.0963	0.1001	0.1035	0.1065	0.1091	0.1115	0.1136
0.1060	0.1133	0.1197	0.1254	0.1305	0.1350	0.1389	0.1425	0.1456	0.1484
0.1290	0.1381	0.1461	0.1532	0.1595	0.1650	0.1700	0.1744	0.1783	0.1818
0.1714	0.1839	0.1949	0.2048	0.2135	0.2212	0.2281	0.2343	0.2397	0.2445
0.2094	0.2251	0.2391	0.2515	0.2626	0.2724	0.2812	0.2890	0.2959	0.3020
0.2433	0.2621	0.2789	0.2938	0.3071	0.3189	0.3295	0.3389	0.3472	0.3547
0.2737	0.2954	0.3147	0.3320	0.3474	0.3612	0.3735	0.3844	0.3941	0.4027
0.3009	0.3252	0.3470	0.3665	0.3839	0.3995	0.4134	0.4257	0.4368	0.4466
0.3252	0.3520	0.3761	0.3976	0.4169	0.4341	0.4495	0.4633	0.4756	0.4865
0.3470	0.3761	0.4022	0.4256	0.4466	0.4654	0.4823	0.4973	0.5108	0.5227
0.3665	0.3976	0.4256	0.4508	0.4734	0.4937	0.5119	0.5281	0.5427	0.5556
0.3839	0.4169	0.4466	0.4734	0.4975	0.5191	0.5385	0.5559	0.5715	0.5854
0.3995	0.4341	0.4654	0.4937	0.5191	0.5420	0.5626	0.5810	0.5975	0.6122
0.4134	0.4495	0.4823	0.5119	0.5385	0.5626	0.5842	0.6036	0.6209	0.6364
0.4257	0.4633	0.4973	0.5281	0.5559	0.5810	0.6036	0.6238	0.6420	0.6582
0.4368	0.4756	0.5108	0.5427	0.5715	0.5975	0.6209	0.6420	0.6609	0.6778
0.4466	0.4865	0.5227	0.5556	0.5854	0.6122	0.6364	0.6582	0.6778	0.6953
0.4553	0.4962	0.5334	0.5672	0.5977	0.6254	0.6503	0.6728	0.6929	0.7110
0.4630	0.5048	0.5429	0.5774	0.6087	0.6371	0.6627	0.6857	0.7064	0.7250
0.4699	0.5125	0.5513	0.5865	0.6185	0.6475	0.6736	0.6972	0.7184	0.7375
0.4760	0.5192	0.5587	0.5946	0.6272	0.6567	0.6834	0.7074	0.7291	0.7486
0.4813	0.5252	0.5653	0.6017	0.6348	0.6648	0.6920	0.7165	0.7386	0.7584
0.4860	0.5305	0.5711	0.6080	0.6416	0.6721	0.6996	0.7245	0.7469	0.7671
0.4902	0.5351	0.5762	0.6136	0.6476	0.6784	0.7063	0.7316	0.7543	0.7748
0.4938	0.5392	0.5807	0.6184	0.6528	0.6840	0.7123	0.7378	0.7608	0.7816
0.4955	0.5410	0.5827	0.6206	0.6552	0.6865	0.7150	0.7406	0.7638	0.7846
0.5070	0.5540	0.5969	0.6362	0.6719	0.7044	0.7339	0.7605	0.7846	0.8064
0.5127	0.5603	0.6039	0.6438	0.6801	0.7132	0.7432	0.7704	0.7949	0.8171
0.5165	0.5645	0.6086	0.6489	0.6856	0.7190	0.7494	0.7769	0.8018	0.8243
0.5169	0.5651	0.6092	0.6495	0.6863	0.7198	0.7502	0.7778	0.8027	0.8252
0.5171	0.5653	0.6094	0.6497	0.6865	0.7200	0.7505	0.7781	0.8030	0.8255
0.5172	0.5653	0.6095	0.6498	0.6867	0.7202	0.7506	0.7782	0.8032	0.8257
0.5172	0.5654	0.6095	0.6499	0.6867	0.7202	0.7506	0.7782	0.8032	0.8257

0.70	0.74	0.78	0.82	0.86	0.90	0.94	0.98	1.00	1.20
0.0401	0.0406	0.0411	0.0415	0.0419	0.0422	0.0425	0.0428	0.0429	0.0437
0.0785	0.0796	0.0806	0.0814	0.0822	0.0828	0.0834	0.0839	0.0842	0.0858
0.1154	0.1171	0.1185	0.1198	0.1209	0.1219	0.1228	0.1236	0.1239	0.1263
0.1509	0.1531	0.1550	0.1567	0.1582	0.1595	0.1607	0.1617	0.1622	0.1654
0.1849	0.1876	0.1900	0.1921	0.1940	0.1957	0.1971	0.1984	0.1990	0.2030
0.2488	0.2526	0.2559	0.2589	0.2615	0.2638	0.2658	0.2676	0.2684	0.2740
0.3075	0.3123	0.3166	0.3203	0.3237	0.3266	0.3292	0.3314	0.3324	0.3396
0.3612	0.3671	0.3722	0.3768	0.3808	0.3844	0.3875	0.3902	0.3914	0.4001
0.4104	0.4172	0.4232	0.4286	0.4333	0.4374	0.4411	0.4442	0.4457	0.4558
0.4553	0.4630	0.4699	0.4760	0.4813	0.4860	0.4902	0.4938	0.4955	0.5070
0.4962	0.5048	0.5125	0.5192	0.5252	0.5305	0.5351	0.5392	0.5410	0.5540
0.5334	0.5429	0.5513	0.5587	0.5653	0.5711	0.5762	0.5807	0.5827	0.5969
0.5672	0.5774	0.5865	0.5946	0.6017	0.6080	0.6136	0.6184	0.6206	0.6362
0.5977	0.6087	0.6185	0.6272	0.6348	0.6416	0.6476	0.6528	0.6552	0.6719
0.6254	0.6371	0.6475	0.6567	0.6648	0.6721	0.6784	0.6840	0.6865	0.7044
0.6503	0.6627	0.6736	0.6834	0.6920	0.6996	0.7063	0.7123	0.7150	0.7339
0.6728	0.6857	0.6972	0.7074	0.7165	0.7245	0.7316	0.7378	0.7406	0.7605
0.6929	0.7064	0.7184	0.7291	0.7386	0.7469	0.7543	0.7608	0.7638	0.7846
0.7110	0.7250	0.7375	0.7486	0.7584	0.7671	0.7748	0.7816	0.7846	0.8064
0.7272	0.7417	0.7546	0.7660	0.7762	0.7852	0.7932	0.8002	0.8034	0.8259
0.7417	0.7566	0.7698	0.7816	0.7921	0.8014	0.8096	0.8168	0.8201	0.8434
0.7546	0.7698	0.7834	0.7956	0.8063	0.8159	0.8243	0.8317	0.8351	0.8591
0.7660	0.7816	0.7956	0.8080	0.8190	0.8288	0.8374	0.8450	0.8485	0.8731
0.7762	0.7921	0.8063	0.8190	6.8302	0.8402	0.8491	0.8569	0.8604	0.8855
0.7852	0.8014	0.8159	0.8288	0.8402	0.8504	0.8594	0.8674	0.8710	0.8966
0.7932	0.8096	0.8243	0.8374	0.8491	0.8594	0.8686	0.8767	0.8803	0.9064
0.8002	0.8168	0.8317	0.8450	0.8569	0.8674	0.8767	0.8849	0.8886	0.9151
0.8034	0.8201	0.8351	0.8485	0.8604	0.8710	0.8803	0.8886	0.8924	0.9191
0.8259	0.8434	0.8591	0.8731	0.8855	0.8966	0.9064	0.9151	0.9191	0.9472
0.9370	0.8549	0.8710	0.8853	0.8980	0.9094	0.9195	0.9284	0.9324	0.9614
0.8445	0.8627	0.8789	0.8935	0.9065	0.9180	0.9282	0.9373	0.9414	0.9709
0.8454	0.8636	0.8799	0.8945	0.9075	0.9191	0.9294	0.9384	0.9426	0.9722
0.8458	0.8640	0.8803	0.8949	0.9079	0.9195	0.9298	0.9389	0.9430	0.9726
0.8460	0.8642	0.8805	0.8951	0.9081	0.9197	0.9300	0.9391	0.9432	0.9728
0.8460	0.8642	0.8805	0.8951	0.9081	0.9197	0.9300	0.9391	0.9433	0.9729

1.40	1.80	2.00	2.20	2.50	3.00
0.0441	0.0444	0.0444	0.0444	0.0444	0.0444
0.0866	0.0871	0.0871	0.0872	0.0872	0.0872
0.1275	0.1283	0.1284	0.1284	0.1284	0.1284
0.1669	0.1680	0.1681	0.1682	0.1682	0.1682
0.2049	0.2062	0.2064	0.2065	0.2065	0.2065
0.2767	0.2785	0.2787	0.2788	0.2788	0.2789
0.3431	0.3454	0.3457	0.3458	0.3458	0.3458
0.4043	0.4071	0.4075	0.4076	0.4077	0.4077
0.4608	0.4641	0.4645	0.4646	0.4647	0.4647
0.5127	0.5165	0.5169	0.5171	0.5172	0.5172
0.5603	0.5645	0.5651	0.5653	0.5653	0.5654
0.6039	0.6086	0.6092	0.6094	0.6095	0.6095
0.6438	0.6489	0.6495	0.6497	0.6498	0.6499
0.6801	0.6856	0.6863	0.6865	0.6867	0.6867
0.7132	0.7190	0.7198	0.7200	0.7202	0.7202
0.7432	0.7494	0.7502	0.7505	0.7506	0.7506
0.7704	0.7769	0.7778	0.7781	0.7782	0.7782
0.7949	0.8018	0.8027	0.8030	0.8032	0.8032
0.8171	0.8243	0.8252	0.8255	0.8257	0.8257
0.8370	0.8445	0.8454	0.8458	0.8460	0.8460
0.8549	0.8627	0.8636	0.8640	0.8642	0.8642
0.8710	0.8789	0.8799	0.8803	0.8805	0.8805
0.8853	0.8935	0.8945	0.8949	0.8951	0.8951
0.8980	0.9065	0.9015	0.9079	0.9081	0.9081
0.9094	0.9180	0.9191	0.9195	0.9197	0.9197
0.9195	0.9282	0.9294	0.9298	0.9300	0.9300
0.9284	0.9373	0.9384	0.9389	0.9391	0.9391
0.9324	0.9414	0.9426	0.9430	0.9432	0.9433
0.9614	0.9709	0.9722	0.9726	0.9728	0.9729
0.9759	0.9858	0.9871	0.9875	0.9878	0.9878
0.9858	0.9959	0.9972	0.9977	0.9979	0.9980
0.9871	0.9972	0.9985	0.9990	0.9992	0.9993
0.9875	0.9977	0.9990	0.9995	0.9997	0.9998
0.9878	0.9979	0.9992	0.9997	1.0000	1.0000
0.9878	0.9980	0.9993	0.9998	1.0000	1.0000

Field measurements of hydraulic conductivity should be used for all but pr

What follows are four sets of qualitative conductivity estimates. An est on the most similar soil type from as many sou

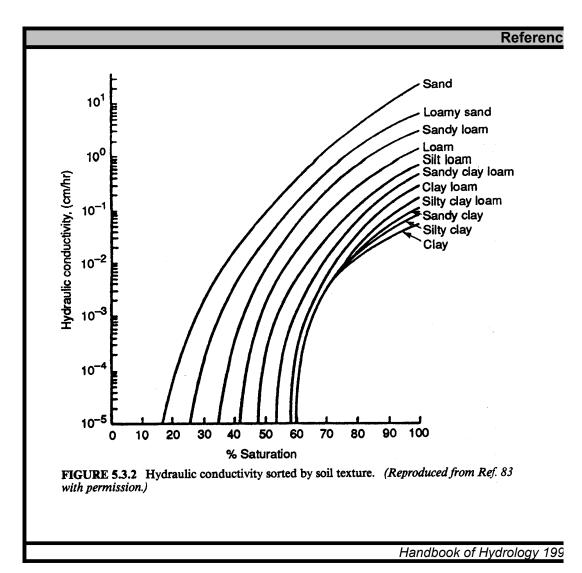
Reference #1					
Material	Intrinsic Permeability (darcys)	Hydraulic Conductivity (cm/s)			
Clay	10 ⁻⁶ -10 ⁻³	10 ⁻⁹ -10 ⁻⁶			
Silt, sandy silts, clayey sands, till	10 ⁻³ -10 ⁻¹	10 ⁻⁶ -10 ⁻⁴			
Silty sands, fine sands	10 ⁻² -10 ⁻¹	10⁻⁵-10⁻³			
Well-sorted sands, glacial outwash	1.0-10 ²	10 ⁻³ -10 ⁻¹			
Well-sorted gravel	10.0-10 ³	10 ⁻² -1.0			
Applied Hydrolog	gy 4th Edition, C.W. I	Fetter			

Sediment or rock type	Hydraulic conductivity, m/day
Clays	10 ⁻⁷ -10 ⁻³
Silts	10 ⁻⁴ -10 ⁻⁰
Fine to coarse sands	10 ⁻² -10 ⁺³
Gravels	10 ⁺² -10 ⁺⁵
Glacial till	<u>See Table 1</u>
Shales (matrix)	10 ⁻⁸ -10 ⁻⁴
Shales (fractured and weathered)	10 ⁻⁴ -10 ⁰
Sandstones (well- cemented)	10 ⁻⁵ -10 ⁻²
Sandstones (friable)	10 ⁻³ -10 ⁰
Carbonates	<u>See Table 3</u>
Salt	10 ⁻¹⁰ -10 ⁻⁸
Anhydrite	10 ⁻⁷ -10 ⁻⁶
Unfractured igneous and metamorphic rocks	10 ⁻⁹ -10 ⁻⁵
Fractured igneous and	10-5 10-1

Table
Rock Type
Cenozoic floo
Dense, unfractured
Vesicular
Interbeds
Quaternary
Vesicular
Tuffs
Densely welded (matrix)
Densely welded (fractured)
Nonwelded

metamorphic rocks	10 - 10 -
Basalts	See Table 2
	-

Hand



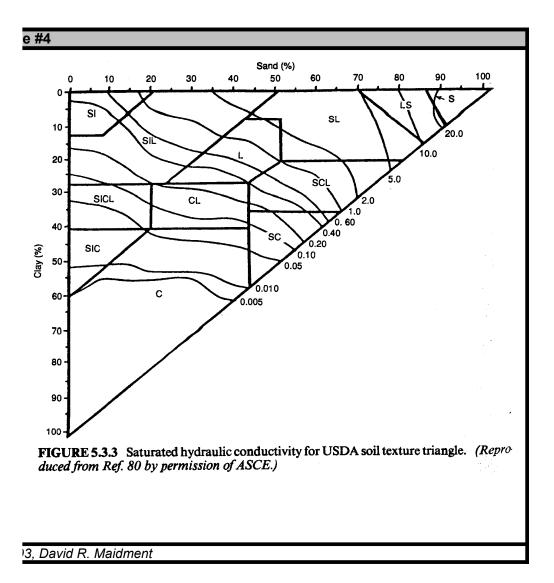
eliminary evaluation of mounding

imated conductivity should be selected based rces as are applicable.

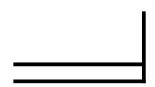
Reference #2						
Geologic Material	Hydraulic Conductivity, m/s					
Coarse gravels	10 ⁻¹ -10 ⁻²					
Sands and gravels	10 ⁻² -10 ⁻⁵					
Fine sands, silts, loess	10 ⁻⁵ -10 ⁻⁹					
Clay, shale, glacial till	10 ⁻⁵ -10 ⁻¹³					
Dolomitic limestones	10 ⁻³ -10 ⁻⁵					
Weathered chalk	10 ⁻³ -10 ⁻⁵					
Unweathered chalk	10 ⁻⁶ -10 ⁻⁹					
Limestone	10 ⁻³ -10 ⁻⁹					
Sandstone	10 ⁻⁴ -10 ⁻¹⁰					
Unweathered granite, gneiss, compact basalt	10 ⁻⁷ -10 ⁻¹³					
Practical Handbook of Ground-Water Monitoring 1991, David M. Nielsen						

10⁻³-10⁻²

Reference #3						
1		Tab				
Hydraulic Conductivity,	Нус	raulic Conduct	vity, m/day		Little a la an r	
m/day	Glacial Deposits	Unweathered	Weathered	Fractured	Lithology	
d basalts	Basal till	10 ⁻⁶ -10 ⁻²	10 ⁻⁴ -10 ⁻¹	10 ⁻⁴ -10 ⁰	Carbonate mud	
10 ⁻⁶ -10 ⁻³	Supraglacial till	10 ⁻⁴ -10 ⁰	10 ⁻⁴ -10 ⁰	10 ⁻⁴ -10 ⁰	Dolomite	
10 ⁻⁴ -10 ⁻³	Glaciolacustrine	10 ⁻⁸ -10 ⁻⁴		10 ⁻⁶ -10 ⁻³	Tertiary limestone	
10 ⁻³ -10 ⁺³	Loess	10 ⁻⁶ -10 ⁰	10 ⁻⁵ -10 ⁻²		Paleozoic limestone	
	Glaciofluvial	10 ⁻⁶ -10 ⁺²			Oolitic limestone	
basalts	Handbook	of Hydrology, D	David R. Maid	lment	Holocene coral	
10 ⁺¹ -10 ⁺³					limestone	
					Karstified limestone	
					Chalk	
<10-6						
10 ⁻⁶ -10 ⁺¹						



le 3
Hydraulic Conductivity, m/day
10 ⁻³ -10 ⁻¹
10 ⁻⁴ -10 ⁰
10 ⁻⁴ -10 ⁰
10 ⁻⁴ -10 ⁰
10 ⁻² -10 ⁻¹
10 ² -10 ⁴
10 ⁻¹ -10 ⁷
10 ⁻³ -10 ⁰



What follows are two sets of specific yield estimates. An estimated specific yield should be selected based on the most similar soil type from as many sources as are applicable.

Reference #1							
Material	Specific Yield						
Waterial	Maximum	Minimum	Average				
Clay	0.05	0	0.02				
Sandy clay	0.12	0.03	0.07				
Silt	0.19	0.03	0.07				
Fine Sand	0.28	0.1	0.21				
Medium Sand	0.32	0.15	0.26				
Coarse Sand	0.35	0.2	0.27				
Gravelly Sand	0.35	0.2	0.25				
Fine gravel	0.35	0.21	0.25				
Medium gravel	0.26	0.13	0.23				
Coarse gravel	0.26	0.12	0.22				
Applied	Applied Hydrology 4th Edition, C.W. Fetter						

Reference #2						
Rocks	Specific Yield					
Clay	0.01-0.10					
Sand	0.10-0.30					
Gravel	0.15-0.30					
Sand and gravel	0.15-0.25					
Sandstone	0.05-0.15					
Shale	0.005-0.05					
Limestone	0.005-0.05					
Practical Handbook of Ground- water Monitoring 1991, David M. Nielsen						

	Project #: 287-2119-K Description: RG4.1							
Performed By: PE								
				Calculated Mound Height: 1.0 fee				
				Calculate			1661	
Input Parameters (inp	out only shaded a	areas):						
Recharge Period	<i>t</i> =	<u>1.5</u>	days	Basin Drains in 4 Hours				
Width of Field	W =	<u>12</u>	feet					
Length of Field	L =	<u>360</u>	feet					
Hydraulic Conductivity	K=	33.3	ft/day	1/3 Sandy Loam,	2/3 Loamy Sa	and		
Specific Yield	V =	0.2	ft ³ /ft ³	1/3 Sandy Loam,				
Saturated Thickness	↓ = D =	<u>5</u>	feet	., e eanay <u>Leann</u> ,	2/0 200			
Daily Flow	D = Q =	<u>5</u> 25,852	gpd	3,456 c.f.				
Daily 110W	Q –	20,002	gpu					
Coloulated Paramete					ription	k (ft/s) [m/s]		
Calculated Parameters:				Sand		(5.77E-04) [1.76E-04] (5.13E-04) [1.56E-04]		
1/0 width		c	fact	Sandy Loa	am		.13E-04) [3.45E-05]	
1/2 width	a =	6	feet	Silty Loam		(2.36E-05) [7.19E-06]		
1/2 length	b =	180	feet	Loamy		(2.28E-05) [6.94E-06]		
Recharge Rate	j =	0.80	ft/day		Sandy Clayey Loam (2.07E-05)			
				Silty Claye		(5.57E-06) [1.7		
	$\gamma = \frac{KD}{V} =$	022 5	e ₁ 2/1	Clay Loan Sandy Cla	n yey Loam	(8.04E-06) [2.4 (7.11E-06) [2.1		
	' - V - V	032.3	ft²/day	Silty Clay	yey Loan	(3.34E-06) [1.0		
		0.0849		Clay		(4.21E-06) [1.2		
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$			Reference #1				
	$\sqrt{4\gamma l}$			Material		Specific Yield		
					Maximum	Minimum	Average	
Dimonoionlogo longth	$\beta = b$	2 5/60		Clay Sandy clay	0.05	0.03	0.02	
Dimensionless length	$\beta = \frac{b}{\sqrt{4\gamma t}} =$	2.5409		Silt	0.12	0.03	0.07	
	V			Fine Sand	0.28	0.1	0.21	
				Medium Sand Coarse Sand	0.32	0.15	0.26	
Solution:				Gravelly Sand	0.35	0.2	0.27	
				Fine gravel	0.35	0.21	0.25	
From Table 1 of Hantus	Medium gravel	0.26	0.13	0.23				
<u>From Table 1 of Hantush (1967), attached:</u> Function S*(a , b) = 0.1776				Coarse gravel 0.26 0.12 0.22 Applied Hydrology 4th Edition, C.W. Fetter				
Function S (a, b) –	0.1776				, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			
Water Table + Mound	$h_m = \gamma$	$h_i^2 + \left[\frac{2j}{K}\right]$	$-\lambda t \cdot S * (a)$	(α, β)				
	$h_m =$	6.0	feet					
				_				

T:__2019 DOCUMENTS K\287-2118-K_JW Management_Fiske Hill East_Site Plan\Stormwater\Rev2_PE\Appendices\APP_F_RG4.1 Mounding Analysis.xls Printed: 5/1/2023, 2:35 PM

Recharge Period Width of Field Length of Field Hydraulic Conductivity Specific Yield Saturated Thickness Daily Flow Calculated Parameters: 1/2 width 1/2 length	only shaded a	<u>1</u> <u>10</u> <u>240</u> <u>27.8</u> <u>0.2</u> <u>5</u>	days feet feet ft/day ft ³ /ft ³ feet	Descriptio Calculate Basin Drains in 4 1/2 Sandy Loam,	ed Mound	Height: 0.7	feet
Recharge Period Width of Field Length of Field Hydraulic Conductivity Specific Yield Saturated Thickness Daily Flow Calculated Parameters: 1/2 width 1/2 length	t = $W =$ $L =$ $K =$ $V =$ $D =$ $Q =$	<u>1</u> <u>10</u> <u>240</u> <u>27.8</u> <u>0.2</u> <u>5</u>	feet feet ft/day ft ³ /ft ³	Basin Drains in 4	Hours	Height: 0.7	feet
Length of Field Hydraulic Conductivity Specific Yield Saturated Thickness Daily Flow Calculated Parameters: 1/2 width 1/2 length	t = $W =$ $L =$ $K =$ $V =$ $D =$ $Q =$	<u>1</u> <u>10</u> <u>240</u> <u>27.8</u> <u>0.2</u> <u>5</u>	feet feet ft/day ft ³ /ft ³	Basin Drains in 4	Hours		1001
Recharge Period Width of Field Length of Field Hydraulic Conductivity Specific Yield Saturated Thickness Daily Flow Calculated Parameters: 1/2 width 1/2 length	t = $W =$ $L =$ $K =$ $V =$ $D =$ $Q =$	<u>1</u> <u>10</u> <u>240</u> <u>27.8</u> <u>0.2</u> <u>5</u>	feet feet ft/day ft ³ /ft ³				
Recharge Period Width of Field Length of Field Hydraulic Conductivity Specific Yield Saturated Thickness Daily Flow Calculated Parameters: 1/2 width 1/2 length	t = $W =$ $L =$ $K =$ $V =$ $D =$ $Q =$	<u>1</u> <u>10</u> <u>240</u> <u>27.8</u> <u>0.2</u> <u>5</u>	feet feet ft/day ft ³ /ft ³				
Width of Field Length of Field Hydraulic Conductivity Specific Yield Saturated Thickness Daily Flow Calculated Parameters: 1/2 width 1/2 length	W = L = K = V = D = Q =	<u>10</u> 240 27.8 0.2 5	feet feet ft/day ft ³ /ft ³				
Length of Field Hydraulic Conductivity Specific Yield Saturated Thickness Daily Flow Calculated Parameters: 1/2 width 1/2 length	L = K = V = D = Q =	<u>240</u> <u>27.8</u> <u>0.2</u> <u>5</u>	feet ft/day ft ³ /ft ³	1/2 Sandy Loam,			
Daily Flow <u>Calculated Parameters:</u> 1/2 width 1/2 length	K = V = D = Q =	<u>27.8</u> <u>0.2</u> <u>5</u>	ft/day ft ³ /ft ³	1/2 Sandy Loam,	4/01 - 0		
Hydraulic Conductivity Specific Yield Saturated Thickness Daily Flow <u>Calculated Parameters:</u> 1/2 width 1/2 length	V = D = Q =	<u>27.8</u> <u>0.2</u> <u>5</u>	ft ³ /ft ³	1/2 Sandy Loam,	4/01 - 0		
Specific Yield Saturated Thickness Daily Flow Calculated Parameters: 1/2 width 1/2 length	V = D = Q =	<u>0.2</u> <u>5</u>	ft ³ /ft ³		1/2 Loamy Sa	and	
Saturated Thickness Daily Flow <u>Calculated Parameters:</u> 1/2 width 1/2 length	D = Q =	<u>5</u>		1/2 Sandy Loam,			
Daily Flow <u>Calculated Parameters:</u> 1/2 width 1/2 length	Q =		ICCI	., <u> </u>			
Calculated Parameters: 1/2 width 1/2 length		14,362		1 000 6			
<u>Calculated Parameters:</u> 1/2 width 1/2 length Recharge Rate			gpd	1,920 c.f.			
1/2 width 1/2 length				Desc	ription	k (ft/s) [m	/s]
1/2 length				Sand		(5.77E-04) [1.76E-04]	
1/2 length				Loamy		(5.13E-04) [1.50	
0	a =	5	feet	Sandy Loam		(1.13E-04) [3.45E-05] (2.36E-05) [7.19E-06]	
0	b =	120	feet	Silty Loam Loamy		(2.28E-05) [7.19E-06] (2.28E-05) [6.94E-06]	
Recharge Rate	<i>j</i> =		ft/day	Sandy Clayey Loam		(2.07E-05) [6.31E-06]	
	<i>J</i> —	0.00	n/uay	Silty Claye		(5.57E-06) [1.70	
	KD			Clay Loan	Ý.	(8.04E-06) [2.4	5E-06]
γ	$\gamma = \frac{KD}{V} =$		ft²/day		yey Loam	(7.11E-06) [2.1]	
				Silty Clay		(3.34E-06) [1.02E-06] (4.21E-06) [1.28E-06]	
Dimensionless width α	$a = \frac{a}{\sqrt{4\gamma t}} =$	0 0948		Clay			3E-00]
	$\sqrt{4\gamma t}$	0.0040		Reference #1			
				Material	Maximum	Specific Yield Minimum	Average
	h			Clay	0.05	0	0.02
Dimensionless length β	$\beta = \frac{b}{\sqrt{4\gamma t}} =$	2.2759		Sandy clay	0.12	0.03	0.07
	$\sqrt{4\gamma t}$			Silt Fine Sand	0.19	0.03	0.07
				Medium Sand	0.32	0.15	0.26
				Coarse Sand	0.35	0.2	0.27
Solution:				Gravelly Sand Fine gravel	0.35	0.2	0.25
				Medium gravel	0.33	0.13	0.23
From Table 1 of Hantush ((1967), attache	<u>d:</u>		Coarse gravel	0.26	0.12	0.22
Function S*(a , b) =	0.1966			Applied	Hydrology 4th	Edition, C.W. Fe	etter
Water Table + Mound	$h_m = 1$	$\int h_i^2 + \left[\frac{2j}{K}\right]$	$-\lambda t \cdot S * (a$	$\alpha,\beta)$			
	$h_m =$	5.7	feet				
Mound Height =	h _m - D =	0.7	feet	7			
		~ 11					
Reference: Hantush, M.S. 1967. "	.						

T:__2019 DOCUMENTS K\287-2118-K_JW Management_Fiske Hill East_Site Plan\Stormwater\Rev2_PE\Appendices\APP_F_RG4.2 Mounding Analysis.xls
Printed: 5/1/2023, 2:35 PM

Project: Blueberry Hill Estates Performed By: PE				Project #: 287-2119-K				
				Description: RG5.1				
	Calculated Mound Height: 1.0 feet							
Input Parameters (in	put only shaded a	areas <u>):</u>						
Recharge Period	<i>t</i> =	<u>0.2</u>	days	Basin Drains in 4	Hours			
Width of Field	W =	<u>12</u>	feet					
Length of Field	L =	39	feet					
Hydraulic Conductivity	K =	44.3	ft/day	Loamy Sand per	chart = 5.13E	-4 ft/s = 44.3 ft/d		
Specific Yield	V =	0.21	ft ³ /ft ³	Loamy Sand per			d 30% silt)	
Saturated Thickness	V = D =	<u>5</u>	feet	Loanny Gana por	0.21(
				1 500 6				
Daily Flow	Q =	<u>11,415</u>	gpd	1,526 c.f.				
				Desc	ription	k (ft/s) [m	/s]	
Calculated Paramete	ers:			Sand	1	(5.77E-04) [1.7		
				Loamy		(5.13E-04) [1.5		
1/2 width	a =	6	feet	Sandy Loam		(1.13E-04) [3.45E-05] (2.36E-05) [7.19E-06]		
1/2 length	b =	19.5	feet	Silty Loam Loamy		(2.28E-05) [6.94E-06]		
Recharge Rate	<i>j</i> =	3.26	ft/day	Sandy Cla	yey Loam	(2.07E-05) [6.3		
r toonargo r tato	J —	0.20	na day	Silty Claye	ey Loam	(5.57E-06) [1.7		
	$\gamma = \frac{KD}{V} = 1054.8$ ft		- 2	Clay Loan		(8.04E-06) [2.45E-06]		
	$\gamma = = V$	1054.8	ft²/day	Sandy Cla Silty Clay	yey Loam	(7.11E-06) [2.1 (3.34E-06) [1.0		
				Clay		(4.21E-06) [1.2		
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$	0.2066		Reference #1				
	$\sqrt{4\gamma t}$			Material	Kelere	Specific Yield		
				Material	Maximum	Minimum	Average	
Dimonsionloss longth	eb	0 6712		Clay Sandy clay	0.05	0.03	0.02	
Dimensionless length	$\beta = \frac{b}{\sqrt{4\gamma t}} =$	0.0713		Silt	0.12	0.03	0.07	
1	V			Fine Sand	0.28	0.1	0.21	
				Medium Sand Coarse Sand	0.32	0.15	0.26	
Solution:				Gravelly Sand	0.35	0.2	0.27	
				Fine gravel	0.35	0.21	0.25	
From Table 1 of Hantu	sh (1967), attache	d:		Medium gravel Coarse gravel	0.26	0.13	0.23	
Function S*(a , b) =	0.3387					Edition, C.W. Fe		
Water Table + Mound	$h_m = 1$	$\sqrt{h_i^2 + \left\lfloor \frac{2}{K} \right\rfloor}$	$\frac{j}{2}\lambda t \cdot S * ($	(α, β)				
	$h_m =$	6.0	feet					
Mound Height =	h _m - D =	1.0	feet					
Reference: Hantush, M.S. 19 Water Resources Research, 3	67. "Growth and Decay			in Response to Unif	orm Percolatio	on."		

T:__2019 DOCUMENTS K\287-2118-K_JW Management_Fiske Hill East_Site Plan\Stormwater\Rev2_PE\Appendices\APP_F_RG5.1 Mounding Analysis.xls Printed: 5/1/2023, 2:30 PM

	Project #: 287-2119-K							
Performed By: PE				Description: RG15H				
				Calculated Mound Height: 0.1 feet				
						5		
Input Parameters (in	put only shaded a	areas):						
Recharge Period	<i>t</i> =	<u>0.3</u>	days	Basin Drains in	4.5 Hours			
Width of Field	W =	<u>4</u>	feet					
Length of Field	L =	<u>60</u>	feet					
Hydraulic Conductivity	K=	44.3	ft/day	Loamy Sand pe	r chart = 5.13E	-4 ft/s = 44.3 ft/d		
Specific Yield	V =	0.21	ft ³ /ft ³	• •		70% medium sar		
Saturated Thickness	D =	<u>5</u>	feet	5			, ,	
Daily Flow	2 = Q =	<u>5</u> <u>1,645</u>	gpd	220 c.f.				
Daily 110W	Q –	1,045	gpu	220 0.1.				
Coloulated Devenueta					cription	k (ft/s) [m		
Calculated Paramete	ers:			Sand		(5.77E-04) [1.7 (5.13E-04) [1.5		
1/0 width		2	fact	Loamy Sandy Lo	am	(1.13E-04) [3.4		
1/2 width		2	feet	Silty Loa	m	(2.36E-05) [7.19E-06]		
1/2 length		30	feet	Loamy	Loamy		4E-06]	
Recharge Rate	<i>j</i> =	0.92	ft/day		ayey Loam /ey Loam	(2.07E-05) [6.3 (5.57E-06) [1.7		
	KD			Clay Loa		(8.04E-06) [2.4		
	$\gamma = \frac{KD}{V} =$	1054.8	ft²/day		ayey Loam	(7.11E-06) [2.1		
	V			Silty Clay	1	(3.34E-06) [1.0	-	
Dimensionless width	a	0.0560		Clay		(4.21E-06) [1.2	8E-06]	
	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$			Reference #1				
	•			Material	Maximum	Specific Yield Minimum	Average	
	h			Clay	0.05	0	0.02	
Dimensionless length	$\beta = \frac{b}{\sqrt{4\chi t}} =$	0.8432		Sandy clay	0.12	0.03	0.07	
	$\sqrt{4}$			Silt Fine Sand	0.19	0.03	0.07	
				Medium Sand	0.32	0.15	0.26	
Solution:				Coarse Sand	0.35	0.2	0.27	
Solution.				Gravelly Sand Fine gravel	0.35	0.2	0.25	
From Toble 4 of Honty	ab (4007) attacks	-J.		Medium gravel	0.26	0.13	0.23	
From Table 1 of Hantu		<u>a:</u>		Coarse gravel	0.26	0.12 Edition, C.W. Fe	0.22	
Function S*(a , b) =	0.1131			Applied	a nyarology 4th	Lunion, C.W. Fe	aller	
Water Table + Mound	$h_m = 1$	$h_i^2 + \left[\frac{2j}{K}\right]$	$\frac{1}{2}\lambda t\cdot S*(a$	(α, β)				
	$h_m =$	5.1	feet					
Mound Height =	h _m - D =	0.1	feet	7				
Reference: Hantush, M.S. 19 Water Resources Research, 3	-	of Groundw	ater Mounds	s in Response to Ur	niform Percolat	ion."		

T:_2019 DOCUMENTS K\287-2118-K_JW Management_Fiske Hill East_Site Plan\Stormwater\Rev1\Appendices\APP_F_RG15H Mounding Analysis.xls Printed: 3/21/2023, 3:37 PM

Project: Blueberry Hill Estates				Project	: #: 287-21	19-K	
Performed By: PE				Description: RG 42			
-			Calculate	ed Mound I	Height: 0.3	feet	
						U	
Input Parameters (in	put only shaded a	areas):					
Recharge Period	<i>t</i> =	<u>0.3</u>	days	Basin Drains in 4	.5 Hours		
Width of Field	W =	<u>8</u>	feet				
Length of Field	L =	<u>60</u>	feet				
Hydraulic Conductivity	K=	<u>44.3</u>	ft/day	Loamy Sand per	chart = 5.13E	-4 ft/s = 44.3 ft/d	
Specific Yield	V =	<u>0.21</u>	ft ³ /ft ³	Loamy Sand per	chart = 0.21 (70% medium saı	nd, 30% silt)
Saturated Thickness	D =	5	feet				
Daily Flow	 Q =		gpd	530 c.f.			
Dully Flow	u –	<u>0,000</u>	gpu				
Calculated Paramete	are :				ription	k (ft/s) [m	
Calculated Parallet	<u>#15.</u>			Sand Loamy		(5.77E-04) [1.7 (5.13E-04) [1.5	
1/2 width	2-	4	feet	Sandy Loa	am	(1.13E-04) [3.4	
		4 30		Silty Loan	n	(2.36E-05) [7.1	
1/2 length				Loamy Sandy Cla	vouloam	(2.28E-05) [6.9 (2.07E-05) [6.3	
Recharge Rate	<i>j</i> =	1.10	ft/day	Silty Clay	yey Loam ey Loam	(5.57E-06) [1.7	
	KD			Clay Loan		(8.04E-06) [2.4	-
	$\gamma = \frac{KD}{V} =$	1054.8	ft²/day		yey Loam	(7.11E-06) [2.1	
				Silty Clay Clay		(3.34E-06) [1.0 (4.21E-06) [1.2	-
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$	0.1124			Refere		
	$\sqrt{4\gamma t}$			Material	Referen	Specific Yield	
					Maximum	Minimum	Average
Dimensionless length	$\beta = \frac{b}{\sqrt{4\pi}} =$	0.8432		Clay Sandy clay	0.05	0.03	0.02
	$\sqrt{4\gamma t}$	0.0.01		Silt	0.19	0.03	0.07
				Fine Sand Medium Sand	0.28	0.1	0.21
O a lasti a sa				Coarse Sand	0.35	0.2	0.27
<u>Solution:</u>				Gravelly Sand Fine gravel	0.35	0.2	0.25
From Table 4 of the f	ab (4007) - ++	l.		Medium gravel	0.26	0.13	0.23
From Table 1 of Hantu		<u>ea:</u>		Coarse gravel	0.26	0.12 Edition, C.W. Fe	0.22
Function S*(a , b) =	0.2141			Прриса	riyarology 4ar	Luidon, O.W. I	
Water Table + Mound	$h_m = L$	$\sqrt{h_i^2 + \left[\frac{2j}{K}\right]}$	$-\lambda t \cdot S * (a)$	(α,β)			
	$h_m =$	5.3	feet				
Mound Height =	h _m - D =	0.3	feet	7			
Reference: Hantush, M.S. 19	67. "Growth and Decay	y of Groundw	ater Mounds	in Response to Un	iform Percolati	on."	
Water Resources Research, 3	3, pp. 227-234.						

Groundwater Mounding Analysis - Hantush Method

T:__2019 DOCUMENTS K\287-2118-K_JW Management_Fiske Hill East_Site Plan\Stormwater\Rev1\Appendices\APP_F_RG42 Mounding Analysis.xls Printed: 3/21/2023, 3:37 PM

	Project: Blueberry Hill Estates				Project #: 287-2119-K			
Performed By: PE				Description: RG69				
				Calculate	ed Mound I	Height: 0.	1 feet	
Input Parameters (inp	out only shaded a	areas):						
Recharge Period	<i>t</i> =	<u>0.2</u>	days	Basin Drains in 4	.5 Hours			
Width of Field	W =	<u>6</u>	feet					
Length of Field	L =	<u>35</u>	feet					
Hydraulic Conductivity	K =	<u>44.3</u>	ft/day	Loamy Sand per	chart = 5.13E	-4 ft/s = 44.3 ft/	d	
Specific Yield	V =	0.21	ft ³ /ft ³	Loamy Sand per	chart = 0.21 (70% medium sa	and, 30% silt	
Saturated Thickness	D =	5	feet	, .	,			
Daily Flow	 Q =		gpd	184 c.f.				
Daily How	Q –	<u>1,070</u>	gpu					
Coloulated Devenate					ription	k (ft/s) [I		
Calculated Paramete	<u>irs:</u>			Sand Loamy		(5.77E-04) [1. (5.13E-04) [1.		
1/2 width	a =	3	feet	Sandy Loa	am	(1.13E-04) [3.		
		17.5	feet	Silty Loan	n	(2.36E-05) [7.		
1/2 length				Loamy Sandy Cla	yey Loam	(2.28E-05) [6. (2.07E-05) [6.		
Recharge Rate	<i>j</i> =	0.88	ft/day	Silty Clay		(5.57E-06) [1.		
	KD			Clay Loan		(8.04E-06) [2.		
	$\gamma = \frac{KD}{V} =$	1054.8	ft²/day	Sandy Cla Silty Clay	yey Loam	(7.11E-06) [2. (3.34E-06) [1.		
	a			Clay		(4.21E-06) [1.	-	
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$	0.1033			Refere	nce #1		
	$\sqrt{-\mu}$			Material		Specific Yield	1 -	
	7			Clay	Maximum 0.05	Minimum 0	Average 0.02	
Dimensionless length	$\beta = \frac{b}{\sqrt{4\chi}} =$	0.6024		Sandy clay	0.12	0.03	0.07	
	$\sqrt{4\gamma t}$			Silt Fine Sand	0.19	0.03	0.07	
				Medium Sand	0.32	0.15	0.26	
Solution:				Coarse Sand Gravelly Sand	0.35	0.2	0.27	
<u>oolutionn</u>				Fine gravel	0.35	0.21	0.25	
From Table 1 of Hantus	sh (1967). attache	d:		Medium gravel Coarse gravel	0.26	0.13	0.23	
Function S*(a , b) =	0.1816					Edition, C.W. I		
	0.1010							
Water Table + Mound	$h_m = 1$	$h_i^2 + \left[\frac{2j}{2}\right]$	$-\lambda t \cdot S * (a)$	$\alpha,\beta)$				
	<i>""</i>	$ \lfloor K $		· ´]				
	$h_m =$	5.1	feet					

Water Resources Research, 3, pp. 227-234.

	Project: Blueberry Hill Estates				Project #: 287-2119-K			
Performed By: PE				Description: RG70				
				Calculate	d Mound	Height: 0.1	feet	
Input Parameters (in	put only shaded a	ireas):						
Recharge Period	<i>t</i> =	<u>0.2</u>	days	Basin Drains in 4	.5 Hours			
Width of Field	W =	<u>6</u>	feet					
Length of Field	L =	<u>35</u>	feet					
Hydraulic Conductivity	K=	44.3	ft/day	Loamy Sand per	chart = 5.13E	-4 ft/s = 44.3 ft/d		
Specific Yield	V=	0.21	ft ³ /ft ³	Loamy Sand per	chart = 0.21 (70% medium sa	nd, 30% silt)	
Saturated Thickness	D =	<u>5</u>	feet	, ,	- (, ,	
Daily Flow	Q =	<u> </u>	gpd	184 c.f.				
Daily 110W	Q –	<u>1,070</u>	gpu					
Coloulated Baramata					ription	k (ft/s) [n		
Calculated Paramete	<u>ers.</u>			Sand Loamy		(5.77E-04) [1.7 (5.13E-04) [1.5		
1/2 width	a =	3	feet	Sandy Loa	m	(1.13E-04) [3.4		
1/2 length		17.5	feet	Silty Loan	ı	(2.36E-05) [7.1		
•				Loamy Sandy Cla	vevloam	(2.28E-05) [6.9 (2.07E-05) [6.3		
Recharge Rate	<i>j</i> =	0.88	ft/day	Silty Claye		(5.57E-06) [1.7		
	KD		0	Clay Loan		(8.04E-06) [2.4		
	$\gamma = \frac{KD}{V} =$	1054.8	ft²/day	Sandy Cla Silty Clay	yey Loam	(7.11E-06) [2.1 (3.34E-06) [1.0		
	a			Clay		(4.21E-06) [1.2	-	
Dimensionless width	$\alpha = \frac{a}{\sqrt{4\gamma t}} =$	0.1033			Refere	nce #1		
	$\sqrt{-\mu}$			Material		Specific Yield	-	
	7			Clay	Maximum 0.05	Minimum 0	Average 0.02	
Dimensionless length	$\beta = \frac{b}{\sqrt{4\chi}} =$	0.6024		Sandy clay	0.12	0.03	0.07	
	$\sqrt{4\gamma t}$			Silt Fine Sand	0.19	0.03	0.07	
				Medium Sand	0.32	0.15	0.26	
Solution:				Coarse Sand Gravelly Sand	0.35	0.2	0.27	
				Fine gravel	0.35	0.21	0.25	
From Table 1 of Hantus	sh (1967), attache	d:		Medium gravel Coarse gravel	0.26	0.13	0.23	
Function S*(a , b) =	0.1816	<u>u.</u>				Edition, C.W. F		
	0.1010							
Water Table + Mound	$h_m = 1$	$h_i^2 + \left[\frac{2j}{K}\right]$	$-\lambda t \cdot S * (a$	(α,β)				
	$h_m =$	5.1	feet					

Water Resources Research, 3, pp. 227-234.

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu

2. Select BMP from Drop Down Menu

3. After BMP is selected, TSS Removal and other Columns are automatically completed.

	Location:	Treatment A, B, G]	
	В	С	D	Е	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
heet	Rain Garden	0.90	1.00	0.90	0.10
TSS Removal Calculation Worksheet		0.00	0.10	0.00	0.10
		0.00	0.10	0.00	0.10
		0.00	0.10	0.00	0.10
Calo		0.00	0.10	0.00	0.10
	Total TSS Removal =			90%	Separate Form Needs to be Completed for Each Outlet or BMP Train
	Project: Prepared By: Date:			*Equals remaining load from which enters the BMP	n previous BMP (E)

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu

2. Select BMP from Drop Down Menu

3. After BMP is selected, TSS Removal and other Columns are automatically completed.

	Location:	Treatment C]	
	В	С	D	Е	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
heet	Sediment Forebay	0.25	1.00	0.25	0.75
Removal on Worksheet	Rain Garden	0.90	0.75	0.68	0.08
Rem Ion V		0.00	0.08	0.00	0.08
TSS Re Calculation		0.00	0.08	0.00	0.08
Cal		0.00	0.08	0.00	0.08
	.		93%	Separate Form Needs to be Completed for Each Outlet or BMP Train	
	Project: Prepared By: Date:			*Equals remaining load fron which enters the BMP	n previous BMP (E)

V

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu

2. Select BMP from Drop Down Menu

3. After BMP is selected, TSS Removal and other Columns are automatically completed.

	Location:	Treatment D, E, F]	
	В	С	D	Е	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
heet	Rain Garden	0.90	1.00	0.90	0.10
moval Worksheet	Rain Garden	0.90	0.10	0.09	0.01
		0.00	0.01	0.00	0.01
TSS Re Calculation		0.00	0.01	0.00	0.01
Cal		0.00	0.01	0.00	0.01
		Total T	99%	Separate Form Needs to be Completed for Each Outlet or BMP Train	
	Project: Prepared By: Date:			*Equals remaining load fror which enters the BMP	n previous BMP (E)

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu

2. Select BMP from Drop Down Menu

3. After BMP is selected, TSS Removal and other Columns are automatically completed.

	Location:	Treatment H]	
	В	С	D	Е	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
heet	Rain Garden	0.90	1.00	0.90	0.10
Removal on Worksheet	Infiltration Basin	0.80	0.10	0.08	0.02
		0.00	0.02	0.00	0.02
TSS Re Calculation		0.00	0.02	0.00	0.02
Cal		0.00	0.02	0.00	0.02
	Dreiget	Total T	98%	Separate Form Needs to be Completed for Each Outlet or BMP Train	
	Project: Prepared By: Date:	PCE		*Equals remaining load from which enters the BMP	n previous BMP (E)

Version 1, Automated: Mar. 4, 2008

Mass. Dept. of Environmental Protection

Non-automated TSS Calculation Sheet must be used if Proprietary BMP Proposed 1. From MassDEP Stormwater Handbook Vol. 1

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu

2. Select BMP from Drop Down Menu

3. After BMP is selected, TSS Removal and other Columns are automatically completed.

	Location:	Treatment I]	
	В	С	D	Е	F
	BMP ¹	TSS Removal Rate ¹	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
heet	Deep Sump and Hooded Catch Basin	0.25	1.00	0.25	0.75
moval Worksheet	Infiltration Basin	0.80	0.75	0.60	0.15
		0.00	0.15	0.00	0.15
TSS Re Calculation		0.00	0.15	0.00	0.15
Cal		0.00	0.15	0.00	0.15
	- · · /		85%	Separate Form Needs to be Completed for Each Outlet or BMP Train	
	Project: Prepared By: Date:			*Equals remaining load fror which enters the BMP	n previous BMP (E)

V

McClure Engineering, Inc. March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023

APPENDIX G

CONSTRUCTION PERIOD STORMWATER POLLUTION PREVENTION PLAN AND DRAFT WEEKLY CONSTRUCTION PERIOD INSPECTION REPORT

Weekly Stormwater Construction Site Inspection Report Lot 3 Berry Farms Road, Sturbridge, MA 01566

General Information								
Proj	ject Name	55+ MANUFACTU	RED HOUSING	COMMUNITY				
Mas	sDEP File Number:							
Date	e of Inspection		S	tart/End Time				
	ector's Name(s) & tact Information							
	e of Inspection: egular	m event 🗖 Durin	ng storm event	Dest-storm e	vent			
			Weather Inform	nation				
If ye	Has there been a storm event since the last inspection? □Yes□NoIf yes, provide: Storm Start Date & Time:Storm Duration (hrs):Approximate Amount of Precipitation (in):							
	ther at time of this instant lear Cloudy ther:	Rain 🛛 Sleet 🗆	Fog 🗖 Snowi emperature:	ng 🛛 High Win	ds			
	e any discharges occur es, describe:	rred since the last ins	pection? □Yes	□No				
	there any discharges a es, describe:	at the time of inspecti	on? 🛛 Yes 🖾 No)				
	Site – Specific BMPs	BMP Installed?	BMP Maintenance Required?	Corrective Acti	on Needed and Notes			
1	Erosion Control	□Yes □No	□Yes □No					
2	Barrier Catch Basin Inlet Protection	Yes No	Yes No					
3	Temporary Soil Stabilization	Yes No	□Yes □No					
4	Stormwater System	Yes No	□Yes □No					

CERTIFICATION STATEMENT

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Print name and title: _____

Signature: _____ Date:_____

Overall Site Issues

Below are some general site issues that should be assessed during inspections. Customize this list as needed for conditions at your site.

	BMP/activity	Implemented?	Maintenance Required?	Corrective Action Needed and Notes
1	Slopes and disturbed areas not actively being worked properly stabilized?	□Yes □No	□Yes □No	
2	Natural Resource areas (e.g., streams, wetlands, mature trees, etc.) protected with barriers or similar BMPs?	□Yes □No	□Yes □No	
3	Perimeter Controls and sediment barriers adequately installed (keyed into substrate) and maintained?	□Yes □No	□Yes □No	
4	Discharge Points and receiving waters free of any sediment deposits?	□Yes □No	QYes QNo	
5	Storm Drain Inlets properly protected?	□Yes □No	□Yes □No	
6	Construction exit preventing sediment from being tracked into the street?	□Yes □No	QYes QNo	
7	Trash / Litter from work areas collected and placed in covered dumpsters?	□Yes □No	□Yes □No	
8	Washout Facilities (e.g., paint, stucco, concrete) available, clearly marked, and maintained?	□Yes □No	□Yes □No	
9	Vehicle and Equipment Fueling, cleaning, and maintenance areas free of spills, leaks, or any other deleterious material?	□Yes □No	□Yes □No	
10	Materials that are potential stormwater contaminants stored inside or under cover?	□Yes □No	□Yes □No	
11	Non-stormwater discharges (wash water, dewatering) properly controlled?	□Yes □No	□Yes □No	

McClure Engineering, Inc. March 31, 2022 Rev. November 9, 2022 Rev. April 28, 2023

APPENDIX H

STORMWATER MANAGEMENT SYSTEM LONG-TERM OPERATION & MAINTENANCE (O & M) PLAN

"55+ Manufactured Housing Community", Lot 3 Berry Farms Road, Sturbridge, MA – Stormwater Management Report

STORMWATER MANAGEMENT SYSTEM

Long Term Operations and Maintenance Plan

"Blueberry Hill Estates" Lot 3 Berry Farms Road Sturbridge, MA 01566

> **Prepared For:** Justin Stelmok 557 Southwest Cutoff Worcester, MA 01607

March 31, 2022 Rev. November 10, 2022



119 Worcester Road - Charlton, Massachusetts 01507 - T: 508.248.2005

TABLE OF CONTENTS

Responsible Party1
Site Description2
Structural Storm Water BMP Maintenance4
Infiltration Basin Deep Sump Hooded Catch Basin Sediment Forebay Rain Garden Rain Guardian Pipe Outfall/Rip Rap Apron/Level Spreader Interceptor Trench Trench Drain Non-Structural Storm Water Controls
Invasive Species Hay Bales Silt Fence Mulching Temporary & Permanent Seeding Landscape & Parking Maintenance Fertilizer, Herbicide, and Pesticide Storage Waste Storage & Trash Removal Hazardous Waste or Oil Spill Reporting Procedure
Snow Management Plan8
Inspections / Recordkeeping / Training9
Public Safety Features9
Operation & Maintenance Budget Estimate9

TABLES

Table 1	Inspection & Maintenance Schedule	
---------	-----------------------------------	--

ATTACHMENTS

Attachment #1	Illicit Discharger Compliance Statement
Attachment #2	Inspection Log & Maintenance Plan

Long-Term Operation & Maintenance Plan Site Stormwater Management System Lot 3 Berry Farms Road, Sturbridge, MA

Property Owner/Responsible Party:	Justin Stelmok 557 Southwest Cutoff Worcester, MA 01607 Phone: (508) 832-5324 Office Phone: (508) 868-3996 Cell
Storm Water Management System Owner:	(same as above)
Site subject to Wetlands Protection Act:	Yes

The Responsible Party Shall:

- Prepare an "Operation and Maintenance (O & M) Compliance Statement" (Attachment #1)
- Implement the routine and non-routine operation, maintenance, and inspection tasks in accordance with the procedures specified in this document to ensure that all storm water management systems function as designed.
- Maintain a log of all operation and maintenance (O & M) activities. Keep records for the last three (3) years, including inspections, repairs, replacement and disposal (for disposal, the log shall indicate the type of material and disposal location).
- Make this log available to **Town of Sturbridge** official representatives upon request;
- Allow **Town of Sturbridge** official representatives to inspect each storm water system "best management practice" (BMP) to determine whether the responsible party is implementing the operation and maintenance plan;
- Agree to notify in writing all future property owners of the presence of the storm water management system and the requirement for proper operation and maintenance.

Responsible Party shall maintain a contract with the following companies:

Landscaping and Pavement Maintenance:

Snow Removal and Plowing:

Storm Water System Maintenance:

Long-Term Operation & Maintenance Plan Blueberry Hill Estates Lot 3 Berry Farms Road, Sturbridge, MA

Site Description:

The Subject Site consists of approximately 41.5 acres. The property lies on the northern side of Main Street and along the Southbridge Town Line. The property is shown as Lot 3 of the Berry Farms Road Definitive Subdivision. The site is located within the Town of Sturbridge Rural Residential zoning district. The existing site consists of mostly wooded area, as well as wetlands. The site has previously been logged and some existing logging trails still exist throughout the property. The site topography slopes generally in a westerly direction towards a valley containing wetlands. The site is surrounded by wetlands on the western boundary, as well as (3) vernal pools as determined by LEC Environmental.

The site is located within an area of minimal flood hazard (Zone X) per Flood Insurance Rate Map (FIRM) Worcester County Massachusetts (All Jurisdictions), Map Number 25027C0933E, effective on 07/04/2011 (see Appendix C).

The proposed site layout is for the construction of a 55+ Manufactured Housing Community. The community is proposed with (4) 20' wide private roads, (3) cul-de-sacs, (1) emergency access drive through the Town of Southbridge, a common clubhouse and active open space area, and (71) total units. The community will be serviced by municipal water and sewer through Berry Farms Road. The stormwater management system for the site consists of country style drainage, including swales and rain gardens with minimal structures for conveyance. Rain gardens will be placed between all units, and will act as a stormwater structure, but also on-site landscaping and yard separation/ privacy barrier. Other than a single deep sump and hooded catch basin in the parking lot for the club house, all stormwater will be conveyed on the surface to rain gardens. These rain gardens will provide for peak flow attenuation, water quality treatment, and groundwater recharge. A total of (77) rain gardens are proposed, with the majority being smaller rain gardens positioned between units which will detain and treat runoff from the units, roads, and driveway. A few larger secondary rain gardens are also proposed. A single large infiltration basin is proposed within an existing natural depression. Interception trenches are proposed behind the units on Roads A and D to convey clean runoff from the undeveloped portions of the property towards the existing discharge points of the property.

The "Special Permit and Site Plan, Blueberry Hill Estates, 55+ Manufactured Housing Community, Lot 3 Berry Farms Road, Sturbridge, MA" Plan Set prepared by McClure Engineering, Inc., dated 4/1/22, revised 11/10/22 provides details of the complete stormwater management system design.

Operation and Maintenance (O&M) Plan

The purpose of this Storm Water Management System Operation and Maintenance Plan is to prevent erosion, sedimentation, pollution or other deterioration of the storm water management system and resource areas located on and adjacent to the property located at Lot 3 Berry Farms Road, Sturbridge, MA. The storm water management system shall be maintained properly to assure its continued performance. Inspection and maintenance for the system should be in compliance with Table 1.

TABLE 1

STORMWATER SYSTEM INSPECTION AND MAINTENANCE SCHEDULE				
Lot 3	"Blueberry Hill Estates" Lot 3 Berry Farms Road, Sturbridge, MA			
Best Management Practice (BMP)	Inspection Frequency	Maintenance Frequency		
	STRUCTURAL BMPs			
Infiltration Basin	After every major storm during first 3 months of operation and twice a year thereafter and when there are discharges through the high outlet orifice.	Bi-Annual Min (Early Spring & Late Fall) and/or As Needed		
Deep Sump Hooded Catch Basin	Quarterly	Quarterly and/or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the basin to the lowest pipe invert in the basin.		
Sediment Forebay	Monthly	Quarterly and/or As Needed		
Interceptor Trench	Quarterly	As Needed		
Rain Garden	Monthly	As Needed		
Rain Guardian	Quarterly	As Needed		
Pipe Outfall/ Rip Rap Apron/ Level Spreader	After heavy rains and Bi-Annually Min (Early Spring & Late Fall)	Bi-Annual Min (Early Spring & Late Fall) and/or As Needed		
Trench Drain	After heavy rains and Bi-Annually Min (Early Spring & Late Fall)	Bi-Annual Min (Early Spring & Late Fall) and/or As Needed		

NON-STRUCTURAL STORMWATER CONTROLS			
Invasive Species	Quarterly As Needed		
Retaining Walls	Quarterly	As Needed	
Wildlife Crossing Structure	Quarterly	As Needed	
Landscaping	Bi-Annual (Early Spring & Late Fall)	Seasonally As Needed	
Parking Area Sweeping	Bi-Annual (Early Spring & Late Fall)	Bi-Annual (2-Times / Year) (Apr/May and Oct/Nov.)	
Snow Removal	Seasonally As Needed	In Accordance with M.G.L. Title XIV. Public Ways and Works; Chapter 85	
Site Inspections	Bi-Annual (Early Spring & Late Fall)	Keep Records on File at Site for Three (3) Years	

Responsible Party shall be responsible for the system and all Operation and Maintenance procedures, including those outlined in the following sections.

STRUCTURAL STORM WATER BMP MAINTENANCE:

Infiltration Basin:

Infiltration basins are prone to clogging and failure so it is imperative to develop and implement aggressive maintenance plans and schedules. Installing the required pretreatment BMPs will significantly reduce maintenance requirements for the basin. Perform inspections and preventive maintenance at least twice a year, and after every time drainage discharges through the high outlet orifice. Inspect the pretreatment BMPs in accordance with the minimal requirements specified for those practices and after every major storm event. A major storm event is defined as a storm that is equal to or greater than the 2-year, 24-hour storm (generally 2.9 to 3.6 inches in a 24-hour period, depending in geographic location in Massachusetts). Once the basin is in use, inspect it after every major storm for the first few months to ensure it is stabilized and functioning properly and if necessary take corrective action. Note how long water remains standing in the basin after a storm; standing water within the basin 48 to 72 hours after a storm indicates that the infiltration capacity may have been overestimated. If the ponding is due to clogging, immediately address the reasons for the clogging (such as upland sediment erosion, excessive compaction of soils, or low spots). Thereafter, inspect the infiltration basin at least twice per year. Important items to check during the inspection include: signs of differential settlement, cracking, erosion, leakage in the embankments, tree growth on the embankments, condition of riprap, sediment accumulation, and the health of the turf. At least twice a year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings and accumulated organic matter to prevent an impervious organic mat from forming. Remove trash and debris at the same time. Use deep tilling to break up clogged surfaces, and revegetate immediately. Remove sediment from the basin as necessary, but wait until the floor of the basin is thoroughly dry. Use light equipment to remove the top layer so as to not compact the underlying soil. Deeply till the remaining soil, and revegetate as soon as possible. Inspect and clean pretreatment devices associated with basins at least twice a year, and ideally every other month.

Deep Sump Hooded Catch Basin:

Regular maintenance is essential. Deep sump catch basins remain effective at removing pollutants only if they are cleaned out frequently. Inspect or clean deep sump basins at least four times per year and at the end of the foliage and snow removal seasons. Sediments must also be removed four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin. Clamshell buckets are typically used to remove sediment in Massachusetts. However, vacuum trucks are preferable, because they remove more trapped sediment and supernatant than clamshells. Vacuuming is also a speedier process and is less likely to snap the cast iron hood within the deep sump catch basin. Although catch basin debris often contains concentrations of oil and hazardous materials such as petroleum hydrocarbons and metals, MassDEP classifies them as solid waste. Unless there is evidence that they have been contaminated by a spill or other means, MassDEP does not routinely require catch basin cleanings to be tested before disposal. Contaminated catch basin cleanings must be evaluated in accordance with the Hazardous Waste Regulations, 310 CMR 30.000, and handled as hazardous waste. In the absence of evidence of contamination, catch basin cleanings may be taken to a landfill or other facility permitted by MassDEP to accept solid waste, without any prior approval by MassDEP. However, some landfills require catch basin cleanings to be tested before they are accepted.

Sediment Forebay:

Sediment forebays should be readily accessible for maintenance and sediment removal. Inspect sediment forebays after each significant rainfall. Remove and properly dispose of sediment at least 2 times per year or when sediment deposits total approximately 12". The effectiveness of a sediment forebay is based less on its size than on regular sediment removal. Place waste material in designated disposal areas. Smooth site to blend with surrounding area and stabilize. Clean or replace gravel when sediment pool does not drain properly. Stabilize the floor and sidewalls of the sediment forebay before making it operational, otherwise the practice will discharge excess amounts of suspended sediments. After removing the sediment, replace any vegetation damaged during the clean-out by reseeding. When reseeding, incorporate practices such as hydroseeding with a tackifier, blanket, or similar practice to ensure that no scour occurs in the forebay, while the seeds germinate and develop roots. Check embankment, emergency spillway, and outlet for erosion damage. Check embankment for: settlement, seepage, or slumping along the toe or around pipe. Look for signs of seepage or erosion. Repair immediately. Remove trash and other debris from principal spillway, emergency spillway, and pool area.

Rain Garden/ Bioretention:

Bioretention areas require careful attention while plants are being established and seasonal landscaping maintenance thereafter. Inspect pretreatment devices and bioretention cells regularly for sediment build-up, structural damage, and standing water. Inspect soil and repair eroded areas monthly. Re-mulch void areas as needed. Remove litter and debris monthly. Treat diseased vegetation as needed. Remove and replace dead vegetation twice per year (spring and fall). Proper selection of plant species and support during establishment of vegetation should minimize—if not eliminate—the need for fertilizers and pesticides. Remove invasive species as needed to prevent these species from spreading into the bioretention area. Replace mulch every two years, in the early spring. Upon failure, excavate bioretention area, scarify bottom and sides, replace filter fabric and soil, replant, and mulch. Because the soil medium filters contaminants from runoff, the cation exchange capacity of the soil media will eventually be exhausted. When

the cation exchange capacity of the soil media decreases, change the soil media to prevent contaminants from migrating to the groundwater, or from being discharged via an underdrain outlet. Using small shrubs and plants instead of larger trees will make it easier to replace the media with clean material when needed. Plant maintenance is critical. Concentrated salts in roadway runoff may kill plants, necessitating removal of dead vegetation each spring and replanting. Never store snow in bioretention areas.

Pipe Outfall/Rip Rap Apron/Level Spreader:

Inspect riprap outlet structures after heavy rains for erosion at sides and ends of apron and for stone displacement. Rock may need to be added if sediment builds up in the pore spaces of the outlet pad. Make repairs immediately using appropriate stone sizes. Do not place stones above finished grade. If erosion is occurring down gradient of the outfall, the down gradient vegetation is not stable and the area should be stabilized, the rip rap apron is not long or wide enough and needs to be increased, or the riprap stones are too small or not graded well. If movement of stone is occurring: riprap stones may be too small or not graded well, or the appropriate filter fabric may not be installed under riprap. If erosion occurs around apron and scour holes appear at outlet, foundation may not be excavated wide or deep enough. If erosion of the foundation is occurring, the appropriate filter fabric may not be installed under riprap.

Level spreaders should be inspected periodically and after every major storm. Any detrimental sediment accumulation should be removed. If rilling has taken place on the lip, the damage should be repaired and re-vegetated. Vegetation should be mowed occasionally to control weeds and encroachment of woody vegetation. Clippings should be removed and disposed of outside the spreader and away from the outlet area. Fertilization should be done as necessary to keep the vegetation healthy and dense. The spreader should be inspected after every runoff event to ensure that it is functioning correctly.

Interceptor Trench

Interceptor trenches are prone to failure due to clogging, it is imperative that they be aggressively maintained on a regular schedule. Using pretreatment BMPs will significantly reduce the maintenance requirements for the trench itself. Removing accumulated sediment from a deep sump catch basin or a vegetated filter strip is considerably less difficult and less costly than rehabilitating a trench. Perform preventive maintenance at least twice a year. Inspect and clean pretreatment BMPs every six months and after every major storm event (2 year return frequency). Check inlet and outlet pipes to determine if they are clogged. Remove accumulated sediment, trash, debris, leaves and grass clippings from mowing. Remove tree seedlings, before they become firmly established. Inspect the trench after the first several rainfall events, after all major storms, and on regularly scheduled dates every six months. If the top of the trench is grassed, it must be mowed on a seasonal basis. Grass height must be maintained to be no more than four inches. Routinely remove grass clippings leaves and accumulated sediment from the surface of the trench. Inspect the trench 24 hours or several days after a rain event, to look for ponded water. If there is ponded water at the surface of the trench, it is likely that the trench surface is clogged. To address surface clogging, remove and replace the topsoil or first layer of stone aggregate and the filter fabric. If water is ponded inside the trench, it may indicate that the bottom of the trench has failed. To rehabilitate a failed trench, all accumulated sediment must be stripped from the bottom, the bottom of the trench must be scarified and tilled to induce

infiltration, and all of the stone aggregate and filter fabric or media must be removed and replaced.

Rain Guardian

Rain Guardian pretreatment chambers simplify bioretention maintenance by collecting sand, leaves, grass clippings, and other debris in an easy to clean, confined location. Regularly maintaining the Rain Guardian sustains its functionality by maximizing storage and filtration capacities. Maintenance frequency is variable and depends on many factors such as rainfall frequency, drainage area size and land use type, and season of the year. Following rain events, inspect the pretreatment chamber for debris on the top grate, within the chamber, and on the vertical, drop-in filter wall. The maintenance steps described below should be completed if areas of the top grate are clogged, the chamber is >75% full, or the vertical filter wall is clogged. Maintenance should be completed when stormwater has completely drained from the bioretention practice. The filter wall allows the chamber to dry between rain events, which further simplifies maintenance by ensuring removed debris is largely dry. Ensure all debris collected during cleaning of the chamber is completely removed from the site and properly disposed of according to local environmental rules. Once cleaning is complete, reinstall the filter wall with filter fabric facing the inside of the chamber and replace the top grate.

Trench Drain

Maintenance frequency is variable and depends on many factors such as rainfall frequency, drainage area size and land use type, and season of the year. Perform preventive maintenance at least twice a year, inspect and clean the trench every six months. Following rain events, inspect the trench for debris on the top grate, within the chamber, and at the outlet. Check inlet and outlet pipes to determine if they are clogged. Remove accumulated sediment, trash, debris, leaves, grass clippings, etc. by hand, or with a pressure washer. Ensure all debris collected during cleaning of the chamber is completely removed from the site and properly disposed of according to local environmental rules. Once cleaning is complete, replace the top grate.

NON - STRUCTURAL STORM WATER MANAGEMENT CONTROLS / GOOD HOUSEKEEPING PRACTICES:

Invasive Species:

Basins, rain gardens, landscape areas, and common areas will be monitored for the presence of invasive species throughout the year. These areas will be kept free of invasive species utilizing best management practices for removal and disposal.

Retaining Walls:

Inspect retaining walls for leaning, undermining, and failure. Remove vegetation from retaining walls as necessary.

Wildlife Crossing Structure:

Inspect wildlife crossing structure opening and ensure they are not blocked or clogged with debris. Inspect natural light providing grates to ensure they are not clogged with debris. Clean the structure openings and grates as necessary. Ensure the natural substrate on the bottom of the

structure is maintained and is not eroding or rutting. If erosion is observed, a paver style system may be necessary to lock natural soils within structure and stabilize surface.

Hay bales:

Inspect straw/hay bales before a forecasted storm event, immediately after each runoff producing rainfall and at least daily during prolonged rainfall. Ensure there are not gaps between bales or evidence of undermining. Close attention should be paid to the repair of damaged bales, undercutting beneath bales, and flow around the ends of the bales. Necessary repairs to barriers or replacement of bales should be accomplished promptly. Replace rotted or sediment covered bales as necessary. Sediment deposits should be checked after each runoff-producing rainfall. They must be removed when the level of deposition reaches approximately one-half the height of the barrier. Any sediment deposits remaining in place after the straw bale barrier is no longer required should be dressed to conform to the existing grade, prepared and seeded.

Silt Fence:

A sediment fence requires a great deal of maintenance. Silt fences should be inspected immediately after each rainfall and at least daily during prolonged rainfall. Remove accumulated sediment when it reaches one half the height of the sediment fence. Remove sediment deposits promptly to provide adequate storage volume for the next rain and to reduce pressure on fence. Take care to avoid undermining fence during cleanout. Sagging, frayed, torn, or otherwise damaged fabric should be repaired or replaced. Repair end runs and undercutting. Inspect reinforcement and staking materials for structural integrity, and replace when necessary. Sediment deposits remaining after the fabric has been removed should be graded to conform to the existing topography and vegetated.

Mulching:

Mulching shall be used in areas which cannot be seeded because of the season, or are otherwise unfavorable for plant growth (traffic and parking areas). When properly applied, mulch offers a fast, effective means of controlling erosion and dust. Soil surfaces should be roughened prior to mulching. Run track-mounted machinery up and down the slope in order to leave horizontal depressions in the soil running parallel to the slope. Roughened soil surfaces should be mulched and/or seeded as soon as possible. Ensure there is a continuous, uniform, even coverage. Ensure mulch layer is not so thick that it suppresses desired seed germination and plant growth. Ensure rilling or gullying does not occur beneath "binded" mulch. Replace or repair mulch if washed or blown away. On steep slopes and critical areas such as waterways, use netting or anchoring with mulch to hold it in place. Inspect after rainstorms to check for movement of mulch or erosion. If washout, breakage, or erosion occurs, repair surface, reseed, remulch, and install new netting. Straw or grass mulches that blow or wash away should be repaired promptly. Blanket mulch that is displaced by flowing water should be repaired as soon as possible. Continue inspections until vegetation is well established.

Temporary & Permanent Seeding

Well-established vegetation is widely considered the most effective form of erosion control. The presence of temporary or permanent cover will provide stabilization and erosion protection to disturbed areas. Temporary seed mixes contain annual vegetation that grows quickly and helps stabilize an area until permanent vegetation can be established. Proper soil bed preparation, seeding method and soil moisture are critical for successful seed application. Before planting,

scarify/roughen the soil surface and install appropriate surface drainage measures to prevent erosion and scouring. Seed with an approved conservation cover mix during the specified growing season, using native plant species. Seeding operations should be performed within one of the following periods: April 1 - May 31, August 1 - September 10, November 1 - December 15 as a dormant seeding (seeding rates shall be increased by 50% for dormant seeding). As needed, provide water, fertilizer, lime, and mulch to the seedbed. If it is unlikely that growth will occur due to cold weather, apply mulch for temporary stabilization. Inspect within 6 weeks of planting to see if stands are adequate. Check for damage after heavy rains. Stands should be uniform and dense. Fertilize, reseed, and mulch damaged and sparse areas immediately. Tack or tie down mulch as necessary. Seeds should be supplied with adequate moisture. Furnish water as needed, especially in abnormally hot or dry weather or on adverse sites. Water application rates should be controlled to prevent runoff. Inspect seeded areas for failure and make appropriate repairs and re-seed and re-plant as necessary. Inspect for bare spots, rilling, or gullying and correct as necessary. If stand has less than 40% cover, re-evaluate selection of seeding materials and quantities of fertilizer. Re-establish the stand following seedbed preparation and seeding recommendations. If the season prevents resowing, mulch or jute netting is an effective temporary cover. Lack of water may also be an issue. Conduct a follow up survey after one year and re-seed failed areas. Temporarily stabilized areas will require permanent stabilization when the area has been completed as designed or when the growing season begins.

Landscape & Parking Area Maintenance

Landscape areas shall be maintained in a neat and orderly fashion. Landscape maintenance debris shall not be deposited on adjacent properties and properly disposed of off-site as necessary to maintain a clean and orderly appearance. Parking Areas shall be inspected often and after significant rainfall events. Inspect for signs of erosion, rilling, gullying. Regrade and repair parking areas as necessary. If areas are needing constant maintenance apply mulch/wood chips to help prevent further erosion. Areas not used for parking or traffic should be seeded for stabilization. All parking areas should be stabilized prior to off season shutdown, preferably with a mulch application.

Fertilizer, Herbicide, and Pesticide Storage

Storage of all fertilizers, herbicides, and pesticides will be indoors. Use of all fertilizers, herbicides, and pesticides shall be in a manner consistent with the products intended use.

Waste Storage & Trash Removal

All waste products are to be stored indoors, under cover, or within a covered dumpster. Inspect on-site area for litter and trash on a weekly basis. Any accumulated trash, litter, and discarded materials in this area will be removed and will be disposed of at a suitable location on a weekly basis. The loading and dumpster areas throughout the site will be inspected on a daily basis for cardboard and/or paper products and will be inspected on a weekly basis for any accumulated trash, litter, and discarded material. Dumpster to be kept closed when not in use. Gates to the dumpster enclosure areas are proposed to be locked when not in use.

Hazardous Waste or Oil Spill Response Procedure

<u>Initial Notification</u>: In the event of a spill of hazardous waste or oil the facility manager or supervisor will be notified immediately by telephone.

<u>Assessment – Initial Containment:</u> The supervisor or manager will assess the incident and initiate control measures. The supervisor will first contact the Town of Sturbridge Fire Department and then notify the Town of Sturbridge. The Fire Department is ultimately responsible for matters of public health and safety and should be notified immediately.

Fire Department Telephone:	911 (Emergency) 508-347-2525 (Non-Emergency/Dispatch)
Police Department Telephone:	911 (Emergency) 508-347-2525 (Non-Emergency/Dispatch)

<u>Further Notification:</u> Based on the assessment by the Fire Chief, additional notification to a clean up contractor may be made. The Massachusetts Department of Environmental Protection and the EPA may be notified depending upon the nature and severity of the spill. The Fire Chief will be responsible for determining the level of clean up and notification required.

SNOW MANAGEMENT PLAN:

Snow plowing, blowing, and shoveling will be done to allow safe passage of interior roadways, emergency access roadway, as well as access to home sites and the common areas. No salt shall be used to treat unpaved areas during snow and ice conditions. Snow from lighter storms will be plowed to the perimeter of the roadways, sidewalks, driveways, and parking lots and allowed to melt to on-site ran gardens or infiltrations basins. Snow will be temporarily stock piled behind the clubhouse in the event that snow storage along roadways and driveways becomes an issue. If site snow storage interferes with site operations (i.e. blocking of travel aisles, sight distance, or parking) the snow pile will be either removed or reduced legally in a legal manner by the snow plow vendor within 24 hours. Snow shall not be stored directed in or on rain gardens.

Winter Road Salt & Sand Use Restrictions

Salt and sand for winter de-icing will only be stored indoors or under cover. Use of road salt and sand will only be used on a limited basis during the winter months to insure safe passage of roadways, driveways, pedestrian walkways, and parking areas. A reduced salt area shall be enforced along the roadways in close vicinity to vernal pools.

INSPECTIONS / RECORDKEEPING / TRAINING:

Routine Inspections

Routine inspections and maintenance to be conducted with the frequency described in this Operation and Maintenance Plan. An example inspection form is provided in **Attachment #2**.

Recordkeeping

Records of all drainage system inspections and maintenance shall be kept on file for a period of at least three (3) years and provided to the Town of Sturbridge upon request.

PUBLIC SAFETY FEATURES:

All cast iron storm water structure grates and covers shall be kept in good condition and kept closed at all times. Any damaged or broken structures will be replaced immediately upon discovery;

OPERATION AND MAINTENANCE BUDGET ESTIMATE:

The responsible party agrees to maintain an adequate annual budget to provide for the routine maintenance activities detailed in this document including but not limited to:

- Infiltration Basin Maintenance
- Rain Garden Maintenance
- Interceptor Trench Maintenance
- Rain Guardian Maintenance
- Deep Sump Hooded Catch Basin Maintenance
- Sediment Forebay Maintenance
- Pipe Outfall/ Rip Rap Apron/ Level Spreader Maintenance
- Landscape Maintenance
- Trash Removal
- Snow Plowing & Removal

Attachment #1

Operation & Maintenance (O & M) Compliance Statement

Illicit Discharge Compliance Statement Site Storm water Management System Blueberry Hill Estates Lot 3 Berry Farms Road, Sturbridge, MA

Property Owner/Responsible Party:	Justin Stelmok 557 Southwest Cutoff Worcester, MA 01607 Phone: (508) 832-5324 Office Phone: (508) 868-3996 Cell
Storm water Management System Owner:	(same as above)
Site subject to Wetlands Protection Act:	Yes

The above listed Responsible Party is responsible for implementation of this "Long-Term Operation and Maintenance Plan" and certifies that:

- The site has been inspected for erosion and appropriate steps have been taken to permanently stabilize any eroded areas.
- All aspects of storm water BMPs have been inspected for damage, wear and malfunction, and appropriate steps have been taken to repair or replace the system or portions of the system so that the storm water at the site may be managed in accordance with the Stormwater Management Standards, revise date January 2, 2008.
- There is no record or knowledge of existing illicit discharges to the on-site stormwater management system.
- All "future property owners" must be notified of their continuing legal responsibility to operate and maintain the existing stormwater management system structures.
- The "Long-Term Operation and Maintenance Plan" for the storm water BMPs is being implemented.

Signature of Responsible Party:

Justin Stelmok

Date

Attachment #2

Inspection & Maintenance Reports

Long-Term Operation and Maintenance Plan Storm Water Management System

Lot 3 Berry Farms Road, Sturbridge, MA

INSPECTION AND MAINTENANCE REPORT FORM

<u>Note:</u> This Log should be copied prior to use. Note Additional Comments on back of Form.

Inspector's Name:	Date:	Time:	am/pm
Inspector's Qualifications:			
Days Since Last Rainfall:		Amount of Last Rainfall	: inches

Item/Condition to be Checked	Maintenance Required				Corrective Action & Date
	No	Yes			
Infiltration Basin					
Deep Sump Hooded Catch Basin					
Sediment Forebay					
Rain Guardian					
Rain Garden					
Interceptor Trench					
Pipe Outfall/ Rip Rap Apron/ Level Spreader					
Trench Drain					
Landscaping / Trash Removal					
Invasive Species					
Snow Removal (seasonal)					
Retaining Walls					
Wildlife Crossing Structure					