

Charles D. Baker, Governor Karyn E. Polito, Lieutenant Governor Stephanie Pollack, Secretary & CEO Jonathan L. Gulliver, Highway Administrator



March 16, 2018

Leon Gaumond Jr. Town Administrator Town of Sturbridge 308 Main Street Sturbridge, MA 01566

Dear Mr. Gaumond

MassDOT has recently completed a study of the Route 20 at New Boston Road intersection in the Town of Sturbridge. This study was the result of a number of inquiries that we received requesting improvements at this location.

MassDOT has evaluated a number of roadway improvement alternatives including signalization and restricting left-turns into and out of New Boston Road from Route 20. The recommended alternative will physically prohibit left-turns out of New Boston Road. These improvements will also require additional improvements at the Route 20/Route 131 intersection to help facilitate the U-turn movements for vehicles to access Route 20 eastbound. MassDOT intends to work towards implementation of this alternative. The complete study is enclosed with this letter.

If you need any additional assistance or information regarding the implementation of these improvements, please do not hesitate to contact the MassDOT District 3 Traffic section at (508) 929-3906.

Sincerely,

Barry J. Lorion Acting District Highway Director

cc: Traffic Correspondence File M.O. File

> Ten Park Plaza, Suite 4160, Boston, MA 02116 Tel: 857-368-4636, TTY: 857-368-0655 www.mass.gov/massdot



INTEROFFICE MEMORANDUM

TO:	Barry Lorion, Acting District 3 Highway Director
FROM:	Erin Kinahan, District 3 Traffic Engineer
DATE:	March 16, 2018
SUBJECT:	Sturbridge –Route 20 at New Boston Road

District Traffic Engineering staff recently evaluated traffic and safety operations at the New Boston Road/ Route 20 intersection in Sturbridge Massachusetts. This study was a result of a number of inquiries citing the need for improvements at this location. Key findings of the report include the following:

The intersection experienced a total of 14 crashes over the four year period between January 2013-December 2016. Half of these collisions, 7 total, were angle collisions with five crashes impacting vehicles either entering or exiting New Boston Road. The intersection is not eligible for Highway Safety Improvement Program funding since it does not exceed the top 5% of crash clusters in the Central Massachusetts region.

The intersection meets the Traffic Signal Warrant 1 for signalization based on the volume of vehicles exiting New Boston Road and vehicle speeds in excess of 40 mph. However signalization of this intersection is not desirable at this time due to the potential for increased crashes related to the high speeds along the Route 20 corridor.

Four separate alternatives were evaluated as part of this evaluation including: No improvements, signalization, restricting left-turns into and out of New Boston Road, and restricting left-turns out of New Boston Road.

Restricting left-turns out of New Boston Road is our preferred alternative. Improvements at the Route 20/Route 131 intersection including construction of a U-turn bubble and improvements to the left turn lane will be needed as part of this alternative. The full evaluation is provided on the following pages.

INTRODUCTION

The Massachusetts Department of Transportation (MassDOT), Highway Division District 3 Traffic Engineering section has completed a study of traffic and safety operations at the intersection of Route 20 at New Boston Road in the Town of Sturbridge. This study was initiated as a result of several complaints citing the need for improvements due to a number of recent collisions at this intersection. The intersection of Route 20 at New Boston Road is unsignalized with vehicles exiting from New Boston Road under STOP-Control and vehicles turning left into New Boston Road expected to yield to Route 20 westbound. This report summarizes the methodology of the engineering study, describes the results of engineering study and considers several options for changing the operation of the intersection.

METHODOLOGY

Existing traffic volume data was collected by MassDOT in October/November 2017. This data included automatic traffic recorder (ATR) counts on Route 20 and New Boston Road and turning movement counts (TMC) at the intersection of Route 20 at New Boston Road. Crash data for the most recent 4 years (January 2013 – December 2016) was supplied by the MassDOT Traffic and Safety section as well as the Town of Sturbridge. The crash data was also used to calculate a crash rate for the intersection.

As part of the traffic study, an analysis was performed using the traffic volume data and crash data to determine if a traffic signal would be warranted at the intersection. The traffic volume data was used to determine if the volume-based traffic signal warrants are met. The crash data was used to determine if the crash experience traffic signal warrant is met.

Alternatives were developed that may address the safety concerns at the Route 20/New Boston Road. In the 'Alternatives' section of this report, each alternative is described and the potential benefit of each alternative is discussed. It should be noted that each alternative has been evaluated at a conceptual level only.

Existing Conditions

Roadway Geometry

Route 20 is a multi-lane median divided roadway that runs in the east-west direction and generally parallels the I-90 (MassTurnpike) in the vicinity of the study area intersection. This intersection is located approximately 1,500 feet west of the I-84/Route 20 interchange. The Route 20 eastbound approach consists of two through lanes with an additional storage lane provided for vehicles turning left onto New Boston Road. The Route 20 westbound approach consists of two through lanes for right-turning vehicles. The posted speed limit along Route 20 in the vicinity of New Boston Road is 45 mph however approximately 100 feet from New Boston Road the speed limit is reduced to 30 mph.

New Boston Road consists of one lane in each direction with additional shoulder width provided for vehicles turning right out of New Boston Road onto Route 20 westbound.

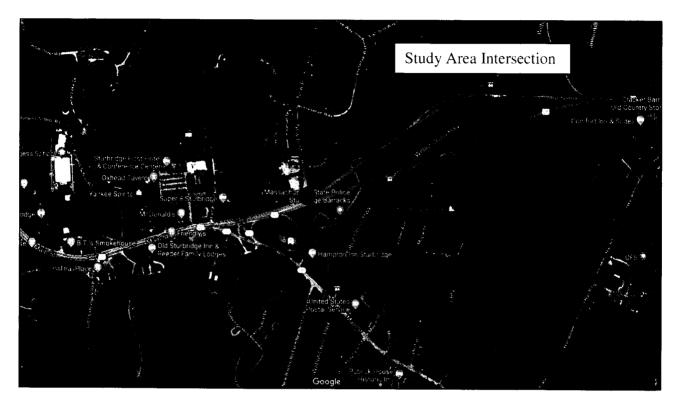


Figure 1 Study Area

Traffic Volumes

In order to determine the average daily vehicular traffic along Route 20, daily automatic traffic recorder (ATR) counts and peak hour turning movement traffic (TMC) counts were conducted between October 31, 2017 and November 4. 2017. ATRs were placed for a 48-hour period and peak hour turning movement counts were collected during the morning peak period (7:00 AM to 9:00 AM), the evening peak period (4:00 PM to 6:00 PM), and the Saturday peak period (11:00 AM to 2:00 PM). Speed data was also collected with the automatic traffic recorder counts. The results of the ATRs are presented below in Table 1.

Roadway	Direction		Total	85% Speed (I	nph)
	Eastbound	Westbound		Eastbound	Westbound
Route 20, West of New Boston Road	10,988	11,406	22,394	50	45
Route 20, East of New Boston Road	11,328	11,266	22,594	49	49
	Northbound	Southbound	Total	Northbound	Southbound
New Boston Road	2,076	2,033	4,109	32	31

Table 1 Traffic Volume Speed Summary with 85% Speed Data

The turning movement counts indicate that the morning peak hour occurs between 7:00-8:00 am and the evening peak hour occurs between 4:15-5:15 pm. The Saturday peak hour occurs between 12:15 pm and 1:15 pm. The majority of vehicles exiting New Boston Road during the morning peak hour are turning left which closely matches the same amount of vehicles turning right into New Boston Road during the evening peak hour. The number of vehicles turning right out of New Boston Road is consistent, 70-85 vehicles, among the three peak hours evaluated. During the evening peak hour 120 vehicles were observed turning left onto New Boston Road from Route 20 EB. As presented in Table 1, there is no dominant movement of vehicles along Route 20 as the EB and WB directions vary slightly.

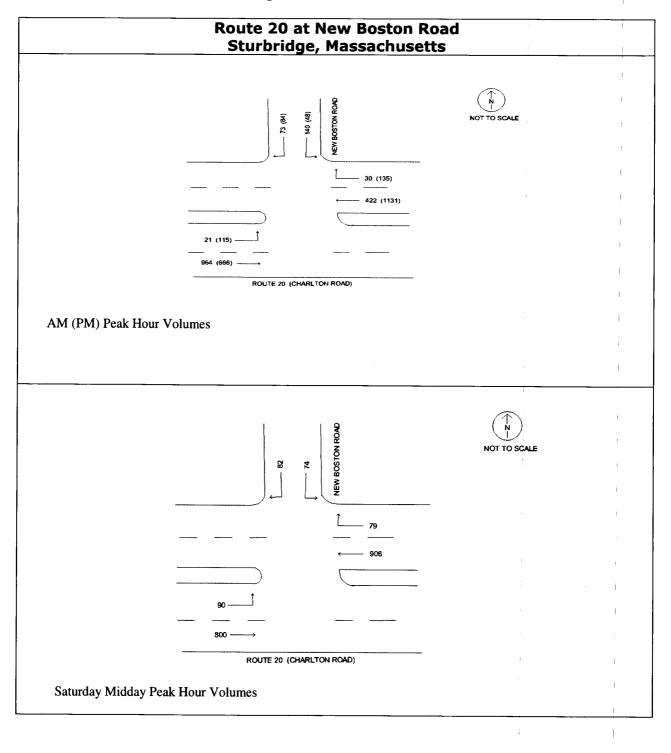


Figure 2 Peak Hour Turning Movement Counts

SAFETY ANALYSIS

In total, 14 crashes occurred at the intersection of Route 20 at New Boston Road for the 4 year period studied. A summary of the number of crashes by severity and by year is shown in Table 2.

					4 Year
	2013	2014	2015	2016	Total
Property Damage Only	4	1	3	4	12
Injury Crashes	0	0	0	2	2
Fatal Crashes	0	0	0	0	0
Total	4	1	3	6	14

Table 2: Crash Summary by Severity and by Year

Key findings of the safety analysis include: of the 14 reported crashes, 12 (86%) were categorized as property damage only, nearly half (43%) of the crashes occurred in 2016, and there were no fatal crashes reported at this intersection during the 4 year study period.

A safety analysis can also be useful to determine if a significant number of crashes occur during one specific time of day (the morning peak period, midday, the evening peak hour, or the rest of the day). A summary of the number of crashes by time of day is shown in Table 3.

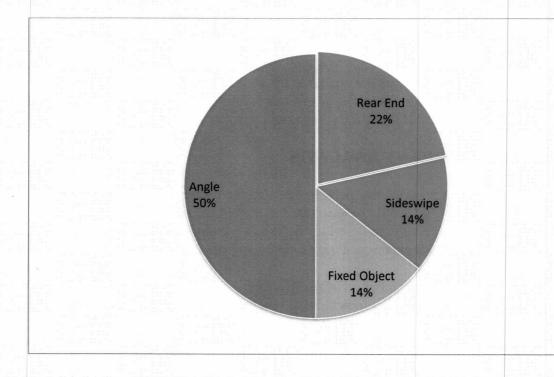
Table 3: Crash Summary by Time of Day

	2013	2014	2015	2016	4 Year Total
Morning (7:00 AM - 10:00 AM)	1	0	1	0	2
Midday (10:00 AM - 4:00 PM)	2	1	0	2	5
Evening (4:00 PM - 6:00 PM)	0	0	1	4	5
Rest of the Day (6:00 PM - 7:00 AM)	1	0	1	0	2
Total	4	1	3	6	14

Based on the information presented in Table 3, many of the crashes occurred in the midday and evening peak time frames with a total of five (36%) crashes occurring during each of these respective time frames (10:00 AM-4:00 PM and 4:00 PM-6:00 PM). However, there appears to be a trend of increasing peak hour crashes at this location with four of the six crashes recorded in 2016 occurring during this time frame (4:00 PM-6:00 PM).

Additionally, we determined the type of collision (for example: fixed object, rear-end, and broadside) for each crash that was reported. A pie chart showing the percentage of the total number of crashes that is of a particular type is shown in Figure 3.

Figure 3: Crash Summary by Type



Half of the crashes (50%) that occurred at the intersection were cross-movement collisions. These crashes are classified in Chart 1 as angle crashes. A collision diagram, showing where each crash occurred, is provided in the appendix.

The collision diagram shows four of the angle crashes were collisions involving a vehicle exiting New Boston Road colliding with a vehicle traveling on Route 20 westbound. The remaining crashes involved a vehicle entering New Boston Road from Route 20 eastbound, a vehicle making a U-turn from Route 20 eastbound to Route 20 westbound and a vehicle turning right into New Boston Road from Route 20 westbound. None of the angle crashes involving vehicle entering or exiting New Boston Road resulted in a personal injury. However, one of the personal injury accidents was a rear-end collision involving vehicles exiting New Boston Road.

A crash rate, in crashes per million entering vehicles (MEV), was calculated for the intersection. The crash rate can be an effective tool to measure the relative safety at a particular location, by comparing the crash rate at the study intersection to the average crash rate for unsignalized intersections statewide and for District 3. The crash rate for the intersection for the 4 year period studied is 0.39 crashes per MEV. This crash rate is lower than both the Statewide (0.58 crashes per MEV) and the District 3 (0.65 crashes per MEV) average crash rate for unsignalized intersections. Typically, improvements at an intersection to address safety concerns are not warranted if the crash rate is below the average crash rate.

One other factor that we use to identify intersection improvement projects is HSIP eligibility. The Highway Safety Improvement program (HSIP) is a federal program that provides a dedicated funding source for safety improvements at eligible locations. The proposed improvements must correct or improve a hazardous road location or addresses a highway safety problem. In order to qualify for this program, the "equivalent property damage only" crashes in the cluster must fall within the top 5% of all clusters in that region. Based on the crash data for the 4-year study period, the New Boston Road/Route 20 intersection does not meet the threshold for HSIP funding.

Based on our safety analysis, there does not appear to be a significant crash problem at this intersection. However the recent uptick in crashes warrants further review of this intersection in the near future, as crash data for 2017 and 2018 becomes available.

TRAFFIC SIGNAL WARRANT ANALYSIS

As part of the traffic study, an analysis was performed using the traffic volume data and crash data to determine if a traffic signal would be warranted at the intersection. The traffic volume data was used to determine if the volume-based traffic signal warrants are met. The crash data was used to determine if the crash experience traffic signal warrant is met.

In order to determine whether the installation of a traffic control signal is justified at a particular location, an engineering study must be completed that reviews traffic conditions, as well as pedestrian and physical characteristics of the intersection. The Manual o Uniform Traffic Control Devices (MUTCD) identifies nine warrants that could justify the installation of a traffic control signal as shown in Table 4.

Warrant No.	Warrant Name
1	Eight-Hour Vehicular Volume
2	Four-Hour Vehicular Volume
3	Peak Hour
4	Pedestrian Volume
5	School Crossing
6	Coordinated Signal System
7	Crash Experience
8	Roadway Network
9	Intersection Near a Grade Crossing

Table 4 – MUTCD Traffic Control Signal Warrants

The three traffic volume warrants (Warrants 1, 2 and 3) were evaluated for the study intersection. However, it should be noted that MassDOT generally expects Warrant 1, Eight-Hour Vehicular Volume, to be met for the installation of any proposed traffic signal on State Highway as outlined in the Massachusetts Amendments to the 2009 MUTCD and the Standard Municipal Traffic Code, January 2012. In order to satisfy Warrant 1, based on criteria for when the major street speed exceeds 40 miles per hour (mph) as observed on Route 20, at least one of two conditions must be met:

- Condition A, Minimum Vehicular Volume, where the total of both approaches on the major street (Route 20) must accommodate 420 vehicles per hour (vph) for 8 hours of the day, and the minor street approach (one direction only, the higher volume approach) must accommodate 105 vph for a one lane approach (New Boston Road) for the same 8 hours of the day; or
- Condition B, Interruption of Continuous Traffic, where the total of both approaches on the major street (Route 20) must accommodate 630 vph for at least 8 hours of the day, and the minor street approach (one direction only, the higher volume approach) must accommodate 53 vph for a one lane approach for the same 8 hours of the day.

The TSWA for Warrant 1, Eight-Hour Vehicular Volume for each of the three study intersections is summarized in Tables 5.

			Traffic Volume	Satis	sfied?		
Time of Day	Route 20 Eastbound	Route 20 Westbound	New Boston Rd	Condition A	Condition B		
7:00 AM	801	266	179	Yes	Yes		
8:00 AM	981	479	221	Yes	Yes		
9:00 AM	881	495	192	Yes	Yes		
10:00 AM	602	469	128	Yes	Yes		
11:00 AM	569	458	104	No	Yes		
12:00 PM	607	614	105	No	Yes		
1:00 PM	679	645	104	No	Yes		
2:00 PM	631	636	110	Yes	Yes		
3:00 PM	763	807	103	No	Yes		
4:00 PM	797	1038	116	Yes	Yes		
5:00 PM	751	1270	126	Yes	Yes		
6:00 PM	656	1217	120	Yes	Yes		
7:00 PM	465	905	102	No	Yes		

Table 5 TSWA – Warrant 1 – Eight-Hour Vehicular Volume

As can be seen in Table 5, the traffic volumes exiting New Boston Road exceed the threshold for any one hour for eight or more hours. Therefore, Warrant 1 is met.

In addition to Warrant 1, Warrant 2 (Four Hour Warrant), Warrant 3 (Peak Hour Warrant) and Warrant 7 (Crash Experience) were also analyzed. The TWSA summary for each of the three traffic volume warrants, Warrants 1, 2 3, and 7 is provided in Table 6. The TWSA for Warrant 7 is not met as there are on average only one crash per year that could be corrected with the installation of a traffic signal.

		Warrant Met?
Warrant No.	Warrant Name	Route 20 at New Boston Road
1	Eight Hour Vehicular	YES
2	Four-Hour Vehicular	YES
3	Peak Hour	YES
7	Crash Experience	NO

Table 6: Traffic Signal Warrant Analysis Summary

Our analysis found that all three of the volume-based warrants were met at the intersection including Warrant 1, the warrant that MassDOT expects to be met for the installation of a traffic signal on State Highway. Warrant 7, Crash Experience, was not met at the intersection, because our analysis found the average crash history per year is lower than the threshold. The TSWA worksheets are provided in the appendix.

ALTERNATIVES ANALYSIS

The four alternatives that were developed are listed below:

- Alternative 1 No changes to the Route 20/New Boston Road intersection
- Alternative 2 Install a traffic signal at the Route 20/New Boston Road intersection
- Alternative 3 Prohibit left turns into or out of New Boston Road
- Alternative 4 Prohibit left turns out of New Boston Road

Alternative 1 No Improvements

Alternative 1 is being considered because the intersection does not have a significant crash history. We feel that Alternative 1 is a viable alternative because the safety analysis shows that there is on average 3.5 crashes per year over the four year study period. However there appears to be a recent trend of increasing number of crashes, based on the number that occurred in 2016. Further evaluation of the crash trends at this location is warranted if no geometric improvements are implemented at this location.

Alternative 2 Signalization

Alternative 2 is being considered due to the intersection meeting the Eight-Hour vehicular warrant. Installing a traffic signal at the intersection could potentially correct up to five (36%) of the 14 crashes that occurred at the intersection during the 4 year period studied. However, the other nine crashes would not be corrected by installing a traffic signal. Also, installing a traffic signal usually increases the number of rear-end crashes that occur at an intersection. Because of the high travel speeds on Route 20 and the proximity of the intersection to the Interstate 84/Route 20 interchange, it is our expectation that installing a traffic signal at the Route 20/New Boston Road intersection could increase the number of crashes that occur at the intersection. For this reason, we feel that installing a traffic signal at the Route 20/New Boston Road intersection is not desirable.

Alternative 3 Restrict All Left-Turns

Alternative 3 would eliminate the cross-movement conflicts at the intersection potentially correcting up to five of the accidents that were recorded at this intersection. The turning vehicle movements into and out of New Boston Road would need to be accommodated elsewhere along Route 20. The left-turning vehicles exiting New Boston Road would be required to turn right and make a U-turn at the at the Route 131/Route 20 intersection. However the left-turning vehicles into New Boston Road could not be easily accommodated elsewhere with limited opportunities for vehicles to make a U-turn.

Alternative 4 Restrict Left-Turns from New Boston Road

Alternative 4 would prohibit the vehicles exiting New Boston Road but would continue to allow the movement of vehicles entering New Boston Road. These restrictions would possibly correct up to four (29%) cross-movement crashes that occurred at the intersection. This permitted movement of vehicles entering New Boston Road resulted in only one crash over the 4 year study period. The vehicles exiting New Boston Road and wanting to head eastbound on Route 20 would need to be accommodated at with a U-turn at Route 131.

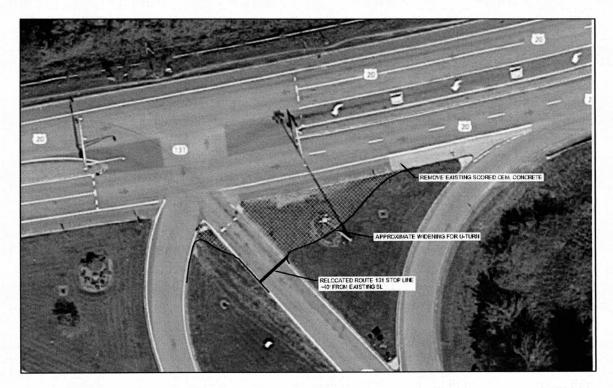
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OTHER ROADWAY IMPROVEMENTS

Both Alternative 3 and 4 will require additional roadway improvements to accommodate the leftturning vehicles out of and possibly into and New Boston Road. Alternative 3 will require improvements at the two adjacent intersections with the left-turn movements out of New Boston Road being rerouted to the Route 131/Route 20 intersection to make a U-turn. The left-turn vehicles into New Boston Road would need to travel further east as there is not a good turnaround location in close vicinity to New Boston Road. The signalized driveway to Hobbs Brook shopping plaza provides the most feasible location for these vehicles to turn around which is located approximately 1.5 miles east of New Boston Road.

Alternative 4 will require improvements at the Route 131/Route 20 intersection only. The leftturns out of New Boston Road can be accommodated at the Route 131/Route 20 intersection with some modifications to the intersection. There would be several physical modifications necessary at this intersection to accommodate U-turning vehicles including construction of a Uturn pocket along Route 20 eastbound to accommodate trucks as well as the lengthening of the existing left-turn lane to provide sufficient storage length for vehicles making this movement. Signal timing changes would also be required to accommodate the westbound U-turning traffic volume. These modifications are showing in Figure 4.

FIGURE 4 Route 131/Route 20 Improvements



Highway Safety Manual Analysis

The installation of a traffic signal will often result in an increase in total number of crashes at an intersection. In order to determine the impacts of Alternative 2 on crash rates, methodology outlined in the Highway Safety Manual (HSM) was applied to this location using the previously collected data. This analysis assumed that all of the I-84/Route 20 interchange ramps are outside of the study area for the signal installation and there would be no geometric changes. Based on the HSM and MassDOT prediction spreadsheets, the existing stop-controlled intersection calculations shows an expected crash frequency of 2.79 crashes per year with 0.51 fatal+injury (F+I) crashes and 2.28 property damage only (PDO) crashes and a predicted overall crash frequency of 1.67 crashes per year and with 0.53 F+I and 1.14 PDO. The expected crashes/year is based on the intersection past crash history and the performance of similar sites while the predicted analysis is based on the Highway Safety Manual.

This same methodology was applied for the signalized intersection scenario. The scenario considered evaluated predicted crashes if the intersection is to be signalized with protected only left turns. This would result in a predicted crash frequency of 0.97 F+I and 2.07 PDO crashes. The results of this analysis are presented in the Table 7 below.

	Expected crashes/Year	Unsignalized Intersection Predicted crashes/Year	
Total	2.79	1.67	3.04
F+I	0.51	0.53	0.97
PDO	2.28	1.14	2.07

Table 7: Highway Safety Analysis Summary

It is important to note that the proposed signal predictions will be further impacted by whether or not right turns on red are allowed. It is expected that the installation of a median that would restrict either some or all of the left-turn at the Route 20/New Boston intersection and therefore there would be an overall reduction of crashes at this location. However, predicted crashes for Alternative 3 and Alternative 4 were not evaluated as the prohibited left-turns would need to be accommodated elsewhere. Conducting a safety analysis for proposed construction of the median along Route 20 would require data for the adjacent area intersections where U-turns would occur. As the Route 131/Route 20 intersection has a protected westbound left-turn phase, the Uturn movement would occur with few conflicts.

INTESECTION OPERATIONS ANALYSIS

Traffic operations at the Route 20 and New Boston Road intersection were analyzed through intersection Level of Service (LOS) analysis. The LOS was calculated using Synchro 8.0 which is based on the traffic operations analysis methodology presented in the 2010 Highway Capacity Manual for unsignalized intersections. The unsignalized intersection level of service analysis for the existing morning, evening and Saturday midday peak hour conditions level of services analysis for intersection indicate that the minor street movement, vehicles exiting New Boston Road, are operating at failing levels-of-service. Under the evening peak hour, volumes are exceeding the capacity which makes it that the HCM methodology does not accurately predict delay for vehicles exiting New Boston Road to calculate an accurate delay for vehicles exiting New Boston Road. Also Alternative 2 was analyzed based on the signalized intersection methodology in the HCM.

If the proposed alternative of restricting left-turns out of New Boston Road were to be implemented at this location, there would be a significant improvement in traffic operations for each of the three peak hour periods. The intersection operations analysis for the exiting condition and mitigated condition are summarized in the Table 8 below.

Table 8:Unsignalized Intersection Operations Analysis Summary
New Boston Road approach

		Delay (seconds)	LOS
Unsignalized-	AM Peak Hour	53	F
Existing	PM Peak Hour	*	F
	Saturday Midday	217	F
	AM Peak Hour	12	В
Unsignalized Right-Turns Only	PM Peak Hour	22	С
	Saturday Midday	18	С

* Volume exceeds capacity

Table 9:Signalized Intersection Operations Analysis Summary
New Boston Road approach

	Morn	ing Pea	k Hour	Ever	ning Peak	Hour	Saturday Midday Peak Hour			
Approach	Delay LOS		95% Queue	Delay	LOS	95% Queue	Delay	LOS	95% Queue	
New Boston Rd	18.4	В	121'	22.6	С	89'	24.8	С	117'	
Route 20 WB	10.7	В	114'	18.3	В	388'	17.8	В	295	
Route 20 EB Left	22.7	С	27'	41.8	D	122'	35.6	D	99'	
Route 20 EB Thru	10.8	В	176'	4.5	Α	80'	6.3	A	126'	

It is expected that if the intersection were to become signalzed under Alternative 2, the overall traffic operations of the Route 20/New Boston Road would degrade as the heavier mainline movement would be required to stop at a red light. However traffic operations for vehicles exiting New Boston Road would improve.

CONCLUSION

Based on the crash reports for the 4 year study period (January 2013 – December 2016), there is not a significant crash history at the Route 20/New Boston Road intersection. The District feels that making no changes to the traffic control or configuration of the intersection, which was considered as Alternative 1 in this engineering study, is a viable alternative at this time.

Alternative 2 which propose the installation of a traffic signal at the Route 20/New Boston Road intersection would likely increase the number of crashes at this intersection and further deteriorate traffic operations along the Route 20 corridor. At this time, MassDOT does not believe that installing a traffic signal at this location is desirable or appropriate.

However given the recent increase in vehicle crashes at New Boston Road, MassDOT feels that implementation of improvements should be considered if the recent trend of crashes seen in 2016

continues over the next two years. Alternative 3 and Alternative 4 would both reduce the crashes caused by vehicles exiting out of New Boston Road. However the left-turn movements would need to be accommodated elsewhere. MassDOT favors Alternative 4 over Alternative 3 because the left-turns out of New Boston Road could be directed to the Route 20/Route 131 intersection where they would be able to make a U-turn. Alternative 4 l would require additional improvements at the adjacent Route 20/ Route 131 intersection to accommodate this movement.

MassDOT will continue to evaluate the crash history at the Route 20/New Boston Road intersection. MassDOT will also work with the Town of Sturbridge to determine the necessary improvements at the Route 20/New Boston Road and the Route 20/Route 131 intersections.

If you have any questions or require additional information, please contact Erin Kinahan, the District Traffic Engineer at (508) 929-3906 or Joe Frawley, District Traffic Operations Engineer, at (508) 929-3916.

cc: Traffic Correspondence Files EOK

Traffic Count Data

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Traffic Count Data

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Highway Capacity Analysis

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Thursday

Study Name Sturbridge - Route 20 and New Boston Road TMC Start Date Thursday, November 02, 2017 7:00 AM End Date Saturday, November 04, 2017 2:00 PM

Site Code

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7:00 AM - 9:00 AM	Cars	58	122	0	180	32	20	343	0	363	947	825	12	0	837	401	1380	E	0	0
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7:00 AM - 8:00 AM	Light Goods Vehicles	10	13	0	23	11	4	44	0	48	124	111	7	0	118	54	189	W	0	0
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4:00 PM - 6:00 PM	Cars	69	42	0	111	213	117	926	0	1043	628	586	96	1	683	996	1837	Е	0	0
One Hour Peak		82%	$\otimes \mathbb{P}_{r_1, r_2}^{M \times \ell}$	194 J	.147.	25%	87%	52%	255	2,226	3.7.1	384	33 C	100%	87%	32%	11 4 17		u [#]	
4:15 PM - 5:15 PM	Light Goods Vehicles	13	4	0	17	33	14	184	0	198	58	54	19	0	73	197	288	w	0	0
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	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	5 <u>6</u>	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	O.e.	0%	0%	(3%)	0%			
	Total	84	48	0	132	250	135	1131	0	1266	714	666	115	1	782	1216	2180			
	PHF	0.78	0.71	0	0.94	0.93	0.89	0.93	0	0.96	0.9	0.89	0.85	0.25	0.88	0.92	0.96			
	Approach %				6%.	1.1%				58%	3,99%				36%	55%				

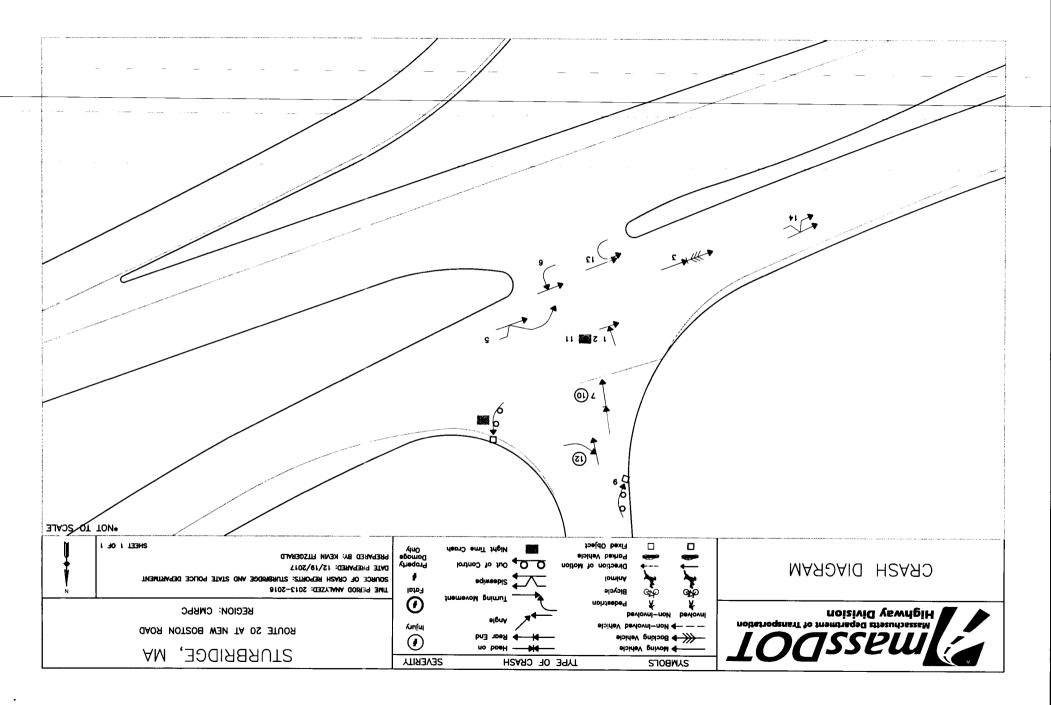
Traffic Signal Warrants Analysis

TRAFFIC SIGNAL WARRANT ANALYSIS (VOLUME BASED) Intersection: Route 20 (Charlton Road) at New Boston Road Sturbridge Major Street Direction: Eastbound - Westbound Year: 2017 Condition: Existing Operating speed on major roadway: 50 mph Required Number of approaches: 1 Approach Volumes Major Street: 2 Lane(s) on each approach 600 420 Warrant 1 EIGHT-HOUR VEHICULAR VOLUME Minimum* Minimum* Warrant 18 INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day) Minor Street: 2 Lane(s) on each approach 900 630 Major Street: 2 Lane(s) on each approach 900 630 Minor Street: 2 Lane(s) on each approach Major Street: 2 Lane(s) on each approach 120 600 420 Major Street: 2 Lane(s) on each approach 120 600 480 720 Minor Street: 1 Lane(s) on each approach 25 accuracy of regression equations Major Street: 2 Lane(s) on each approach 25 accuracy of regression equations Major Street: 2 Lane(s) on each approach 2
Major Street Direction: Eastbound - Westbound Year: 2017 Condition: Existing Operating speed on major roadway: 50 mph Required Number of approaches: 1 Approach Volumes Major Street of the ULLAR VOLUME Minimum* Minimum* Warrant 1 EIGHT-HOUR VEHICULAR VOLUME Minimum* Minimum* Warrant 1A EIGHT-HOUR VEHICULAR VOLUME Minimum* Minimum* Warrant 1A EIGHT-HOUR VEHICULAR VOLUME Minimum* Minimum* Warrant 1B INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day) Major Street : 2 Lane(s) on each approach 900 630 Warrant 1B INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day) Warrant 1A Warrant 1B Warrant 1A Warrant 1B Major Street : 2 Lane(s) on each approach 900 630 75 53 30 PERCENT SATISFACTION OF WARRANT 1A AND WARRANT 1B Warrant 1A Warrant 1B Warrant 1A Warrant 1B Major Street : 2 Lane(s) on each approach 120 60 Warrant 2 FOUR HOUR VEHICULAR VOLUME Major Street : 2 Lane(s) on each approach 16 *verify* indicated, see
Year: 2017 Condition: Existing Operating speed on major roadway: 50 mph Number of approaches: Required Approach Volumes Adjusted Warrant 1 EIGHT-HOUR VEHICULAR VOLUME Major Street: 1 Adjusted Warrant 1A EIGHT-HOUR VEHICULAR VOLUME Minor Street: 1 Lane(s) on each approach Minor Street: 600 420 Warrant 1B INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day) Major Street: 2 Lane(s) on each approach Minor Street: 900 630 Warrant 1B INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day) Major Street: 2 Lane(s) on each approach 900 630 Major Street: 2 Lane(s) on each approach Minor Street: 1 Lane(s) on each approach 900 630 30 PERCENT SATISFACTION OF WARRANT 1A AND WARRANT 1B Major Street: Warrant 1A Warrant 1A Warrant 1B Major Street: 2 Lane(s) on each approach Minor Street: 1 Lane(s) on each approach 120 60 Warrant 3 FOUR HOUR VEHICULAR VOLUME Major Street: 2 Lane(s) on each approach Minor Street: 1 Lane(s) on each approach If "verify" indicated, see Figure 4C-1 or 4C-2. Marrant 3 PEAK HOUR VOLUME Major Street: 2 Lane(s) on each approach Minor Street: 1 Lane(s) on each approach Minor Street:
Operating speed on major roadway: 50 mph Number of approaches: Required Approach Volumes Warrant 1 EIGHT-HOUR VEHICULAR VOLUME Warrant 1A Iminimum * Minimum * Warrant 1 EIGHT-HOUR VEHICULAR VOLUME Major Street : 2 Lane(s) on each approach 600 420 Minor Street : 1 Lane(s) on each approach 600 420 Minor Street : 1 Lane(s) on each approach 900 630 Minor Street : 1 Lane(s) on each approach 900 630 Minor Street : 1 Lane(s) on each approach 75 53 80 PERCENT SATISFACTION OF WARRANT 1A AND WARRANT 1B Warrant 1A Warrant 1B Warrant 1B Major Street : 2 Lane(s) on each approach 480 720 Minor Street : 1 Lane(s) on each approach 25 eacuracy of regression equations Warrant 2 FOUR HOUR VEHICULAR VOLUME Minor Street : 1 Lane(s) on each approach 15 eacuracy of regression equations Warrant 3 PEAK HOUR VOLUME Major Street : 2 Lane(s) on each approach 16 "verify" indicated, see Figure 4C-1 or 4C-2. Minor Street : 1 Lane(s) on each approach 25 = accuracy of regression equations Warrant 3
Number of approaches: 1 Approach Volumes Adjusted Warrant 1 Warrant 1A EIGHT-HOUR VEHICULAR VOLUME (8 hours of day) Major Street: Adjusted Warrant 1A MINIMUM VEHICULAR VOLUME (8 hours of day) Major Street: Adjusted Warrant 1A MINIMUM VEHICULAR VOLUME (8 hours of day) Major Street: Adjusted Warrant 1B INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day) Major Street: 2 Lane(s) on each approach 900 630 Warrant 1B INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day) Major Street: 2 Lane(s) on each approach 900 630 B0 PERCENT SATISFACTION OF WARRANT 1A AND WARRANT 1B Major Street: Warrant 1A Warrant 1A Warrant 1B Major Street: 2 Lane(s) on each approach 120 60 Warrant 2 FOUR HOUR VEHICULAR VOLUME Major Street: 1 Lane(s) on each approach If "verify" indicated, see Figure 4C-1 or 4C-2. Warrant 3 PEAK HOUR VOLUME Major Street: 2 Lane(s) on each approach 15 = accuracy of regression equations Warrant 3 PEAK HOUR VOLUME Major Street: 2 Lane(s) on each approach 16 "verify" indicated, see Figure 4C-3 or 4C-4. Major Street: 1 Lane(s) on each approach 25 = accuracy of regression equations 40/////////
Marrant 1 Warrant 1 Marrant 1 Marrant 1 Minimum EIGHT-HOUR VEHICULAR VOLUME Major Street :: Adjusted Minimum** Minimum** Marrant 1 Minimum Minimum Minimum** Major Street :: 2 Lane(s) on each approach Minor Street :: 600 420 Marrant 1B INTERRUPTION OF CONTINUOUS TRAFFIC (8 hours of day) Major Street :: 2 Lane(s) on each approach 900 630 Minor Street :: 2 Lane(s) on each approach 900 630 Minor Street :: 1 Lane(s) on each approach 900 630 30 PERCENT SATISFACTION OF WARRANT 1A AND WARRANT 1B Warrant 1A Warrant 1B Major Street :: 2 Lane(s) on each approach 480 720 Minor Street :: 1 Lane(s) on each approach 120 60 Marrant 2 FOUR HOUR VEHICULAR VOLUME Major Street :: 1 Lane(s) on each approach 16 "verify" indicated, see Figure 4C-1 or 4C-2. Marrant 3 PEAK HOUR VOLUME Major Street :: 1 Lane(s) on each approach 16 "verify" indicated, see Figure 4C-3 or 4C-4. Major Street :: 1 Lane(s) on each approach 16 "verify" indicated, see Figure 4C-3 or 4C-4. Major Street :: 1 Lane(s) on each approach 25 = accuracy of regression equations Major Street :: 1 Lane(s) on each approach
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Hour Minor Road+ Eastbound Westbound On Major Rd 1A 1B 80%(1A&1B) 2
Hour Minor Road+ Eastbound Westbound On Major Rd 1A 1B 80%(1A&1B) 2
6:00 - 7:00 AM 1/9 801 286 1087 Yes Yes Yes Yes Yes
7:00 - 8:00 AM 221 981 479 1460 Yes Yes Yes Yes Yes
7:00 - 8:00 AM 221 981 479 1460 Yes Yes Yes Yes 8:00 - 9:00 AM 192 881 495 1376 Yes Yes Yes Yes
9:00 - 10:00 AM 128 602 469 1071 Yes Yes Yes Yes
10:00 - 11:00 AM 104 569 458 1027 No Yes No Yes
11:00 - 12:00 AM 105 607 614 1221 No Yes No Yes
12:00 - 1:00 PM 104 679 645 1324 No Yes No Yes
1:00 - 2:00 PM 110 631 636 1267 Yes Yes No Yes
Warrants 1 2
Met? Yes Yes

Source: 2003 Manual on Uniform Traffic Control Devices (MUTCD)

Crash Data

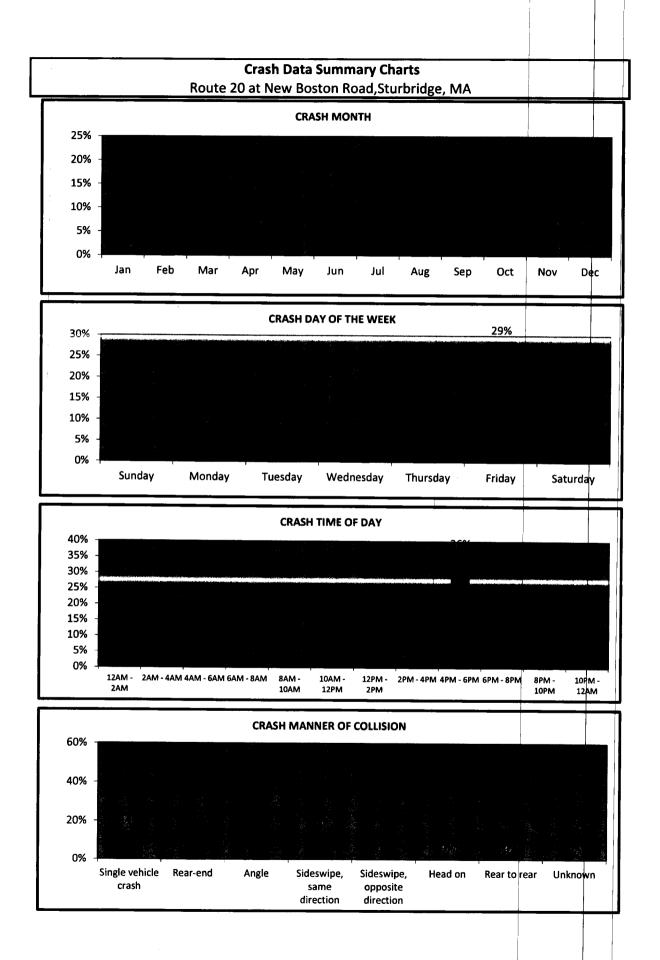
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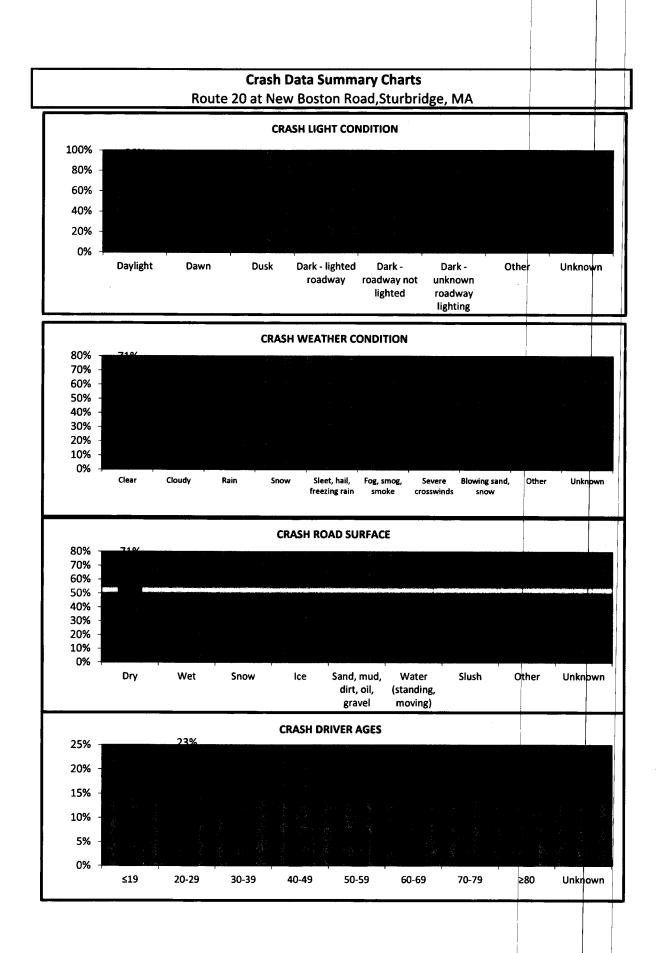
	r	7	1	· · · · · · · · · · · · · · · · · · ·							
The strempted to change lanes and struck MV2	010	76	Failure to keep in proper lane or running off road		Clear	Daylight	Sideswipe, same direction	Martis	Friday	91/10/11	Τ¢
NV1 attempted to turn left to make a U-turn and struck MV2	99	٤	Failed to yield right of way	19W	Rain	Daylight	elanA	Mastis	Yebini	91/12/01	Ε Τ
MVX was travelling at speeds greater than reasonable with brakes not functioning property. MV2 failed to turn onto New Boston road and struck MV1	81	29	fnemqiupe evitostab BritsneqO	Dry	Clear	jd y ikeQ	əlgıA	Wd ES:S			21
MV1 failed to yield for MV2	17	78	Failed to yield right of way	Dry	Clear	Daylight	algnA	Md 20:T	Yebina	91/21/80	ττ
TVM bebra 1891 SVM	52	12	No improper driving	Du	Clest	Daylight	kear-end	MA 00:01		91/21/90	01
MV1 slid on an icy road and struck a fire hydrant		6T	No improper driving	lce	Clear	Daylight	Single vehicle crash	Wd TT:S	-	91/21/30	6
ngis gots eft teeq bits bas enos Aqm05 a in Aqm25 gnilleust sew SVM BW LVM gnishts	81	TS	timil beeqs beriodfus bebeex2	39W	nieЯ	Dark - lighted Vadway	əlşııA	Mq to:8			8
MV1 rear ended MV2		02	Followed too closely	λu	Clear	Daylight	Rear-end	MA 82:8	AepsJny	S1/4/150	L
MV2 EB attempted to turn left to make a U-turn and struck MV1 WB	99	28	Failed to yield right of way	Dry	Cloudy	Daylight	Angle	Wd 80:5	Aepsen T		9
2VM sttempted to make a left turn from the right lane and struck MV2	67	89	Made an improper turn	teW	nisA	Daylight	Sideswipe, same direction	MIN ET:E	Friday	₽T/9T/S0	s
MV1 attempted to take the turn onto New Boston Road at speeds greater than reasonable, lost control and struck a large rock		56	timil baaqe baritorited speed limit	Duy	Clear	Dark - Hghted Dark - Hghted	Single vehicle crash	M9 24:6	Aepsaupa M	EI/81/60	4
WV1 WB, a tractor trailer, stopped and started to reverse on Rt 20 striking WV1 WB		50	Wrong side or wrong way	۵u	Clear	۶dgilyaO	Rear-end	X:50 PM	Aepuns	£1/\$1/70	ε
MV2 attempted to cross &t 20 and struck MV1 WB on &t 20		29	Disregarded traffic signs, signals, road Markings	ALQ.	Clear	Daylight	əlşırA	MA 72:0	Vebrute2	ET/67/90	z
MV2 attempted to cross Rt 20 and struck MV1 WB on Rt 20		75	Failed to yield right of way	Dry	Clear	Daylight	algıA	3:32 PM	Vebnow	£1/81/20	τ
		Amerikan Salah salah sa	h kan dan kan di kana dan kan dan kan dan dan dan dan dan dan dan dan dan d	ini in the second	.			- 		2447-7275-04 1971-1971-198 1971-1971-198	
Crash Data Sturbridge, MA Route 20 at New Boston Road,Sturbridge, MA 2013 - 2013 2013 - 2012											

*Courtesy Crash - A term used to describe a crash that occurs subsequent to a non-involved mainline driver who gives the right of way, contrary to the rules of the road, to another driver.

Summaries based on crash reports obtained from the Sturbridge and State Police Department.



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INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN : STUR	CBRIDE	ĩΕ		COUNT DA	TE:		1
district: <u>3</u>	UNSIGN	IALIZED :	X] SIGNA	LIZED :		
	Parte		TERSECTION				:
MAJOR STREET :	LOUR	20					
MINOR STREET(S) :	<u>Rouk</u> New	Boston	Rd	-			
INTERSECTION DIAGRAM (Label Approaches)	North		N B ,	ew oster 2d	Rt 20		
			PEAK HOUF			Tota	Peak
APPROACH :	1	2	3	4	5	Но	urly
DIRECTION :	EB	NB	SB				roach
PEAK HOURLY VOLUMES (AM/PM) :	985/781	452/1266	213/132			0	ume
"K" FACTOR :	6.79/8.97	INTERSI	ECTION ADT APPROACH		AL DAILY	24	287
TOTAL # OF CRASHES :	14	# OF YEARS :	4	CRASHES	GE # OF PER YEAR () :	3,	5
CRASH RATE CALCU	LATION :	0.39	RATE =	<u> (A * 1,0 </u>	00,000) 1365)		I
Comments :							
Project Title & Date: <u>Ba</u>	sed on C	Comts FA	UL 2017				1
	0,585 0.652	Statewic Sistrict	de 3				